

On the Nature of Symmetrical Contacts of a Beam Impacting a Rigid Wall

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The impact of a compliant structure against a rigid obstacle is governed by multiple time and spatial scales, ranging from rapid local deformations and stress wave propagation to slower global motions. This interaction presents significant challenges for numerical methods, particularly in resolving the immediate postimpact response and contact dynamics. In this study, we focus on the impact of a beam on a rigid obstacle as a minimal yet representative example of such multiscale phenomena. A local asymptotic solution is derived for the short-time response. At leading order, the analysis reveals two distinct contact patterns governed by the ratio of the impact velocity to the beam curvature at the contact point: a single concentrated force whose intensity decays with the inverse square root of time or a pair of traveling forces that move away from each other at a rate inversely proportional to the square root of time. Notably, in the latter case, the beam segment between the traveling forces remains stationary, posing challenges for numerical methods based on penetration conditions. These findings highlight the utility of asymptotic methods in capturing the singular and bifurcated nature of contact forces in multiscale impact problems. The solution can be used to improve or provide a benchmark for assessing numerical approaches in the impact analysis of compliant beam structures with rigid walls. [DOI: 10.1115/1.4070020]

Keywords: discrete contact, continuous contact, self-similar solution, Volterra equation of the first kind, free boundary problem, dynamics, impact, wave propagation

1 Introduction

The impact of a compliant structure against a rigid obstacle is a complex phenomenon characterized by multiple time and spatial scales [1]. On short time scales, rapid local deformations and stress wave propagation dominate, while on longer time scales, slower global motions become more prominent. Similarly, spatial scales range from short localized contact regions, where stress gradients are steep, to the larger structural dimensions influenced by boundary conditions and governing multimodal responses. These scales interact in ways that present challenges for numerical modeling, particularly in accurately capturing the immediate response after impact and resolving the details near the contact point.

The study of contact dynamics in flexible beam systems has evolved significantly, starting with foundational analytical models and progressing to advanced numerical and experimental approaches. Early works [2,3] provided pioneering analytical solutions for beam impacts, focusing on infinite and elastic beams under various loading and contact conditions. These studies established critical insights into energy transfer and stress distribution during localized impact events. Theoretical solutions of the pointwise impact on Bernoulli, Rayleigh, or Timoshenko beam are now available in textbooks [4]. Zhou and Schonberg [5] critiqued

global-local methods for impact analysis, among which the use of assumptions of Hertzian contact, and developed analytical solutions for a spherical impactor on a flexible beam as a solution of a Volterra equation.

The introduction of numerical methods marked a significant advancement. Initial simulations utilized finite element methods to discretize beams and model interactions with rigid walls, relying on penalty methods or simplified contact formulations. Bauchau [6] was instrumental in improving these techniques, addressing the challenges of transient interactions and introducing robust computational frameworks for dynamic contact scenarios. Nonsmooth time integration schemes tailored for flexible multibody systems under unilateral constraints were also advanced [7]. Simultaneously, the study of friction and more complex contact scenarios gained prominence. Durville [8] modeled contact-friction behavior in elastic beam assemblies, and Bertails-Descoubes et al. [9] introduced nonsmooth Newton solvers to capture Coulomb friction with high accuracy. These works bridged computational mechanics with physical phenomena, enhancing model reliability. Unlike in previous dissipative schemes [10], incorporating energy preservation and stability into numerical schemes became a focal point with works like Lens and Cardona [11], who developed nonlinear beam elements within energy-preserving frameworks.

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141 The article begins in Sec. 2 with the formulation of the beam
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153 2 Problem Formulation

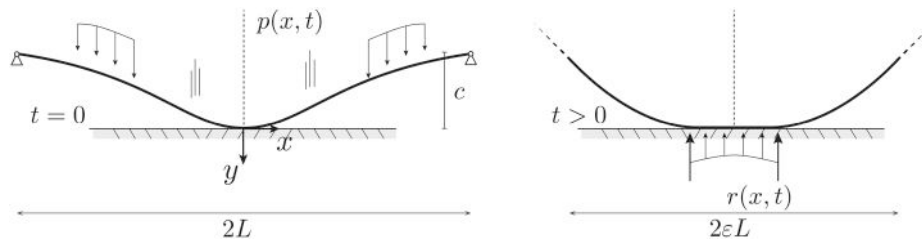
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$$158 \mu \partial_t^2 y(x, t) + EI \partial_x^4 y(x, t) = p(x, t) - r(x, t) \quad (1)$$

159 where $y(x, t)$ is the transverse position at abscissa x and time t , mea-
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 161 to be supplied with appropriate initial and boundary conditions and
 162 is subject to the complementarity condition:

$$163 0 \leq r(x, t) \perp y(x, t) \leq 0 \quad (2)$$

164 This condition translates that the unilateral contact is such that
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 166 barrier located on the axis $y = 0$ and that an a priori unknown (pos-
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169 Fig. 1 Schematic of the considered problem. Left: At a large scale, a vibrating beam with its
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 171 vicinity of the contact, where a possibly distributed reaction pattern takes place at short
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226 3 Slow and Fast Dynamics

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235 **3.1 Slow Dynamics.** The homogenous form of the governing
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 238 with the span L of the beam, considered as the macroscopic
 239 scale, the characteristic timescale is $L^2 \sqrt{\mu/EI}$.

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7 **Abstract**

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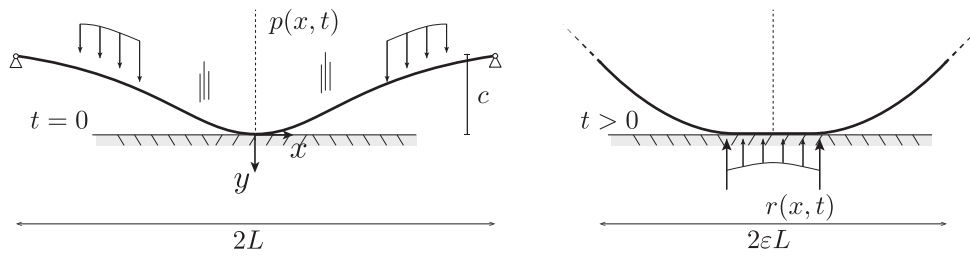


Fig. 1: Schematic of the considered problem. Left: at large scale, a vibrating beam with its own boundary conditions symmetrically impacting a rigid wall. Right: close-up view in the vicinity of the contact, where a possibly distributed reaction pattern takes place at short time post impact.

113 3.1 Slow dynamics

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 115 which has dimensions of a diffusion coefficient (L^2T^{-1}). Together with the span L of the beam, considered
 116 as the macroscopic characteristic length L^* , it defines a slow timescale

$$T^* = \sqrt{\frac{\mu L^{*4}}{EI}} \quad (4)$$

117 which is also related to the fundamental period of beam oscillations. The transverse displacement of
 118 the beam is scaled by a characteristic transverse displacement defined by $y^* = p^* L^{*4}/EI$ where p^* is a
 119 characteristic load per unit length, representative of $p(x, t)$. The solution of (3) is the slow response before
 120 contact takes place. It is written in the dimensionless form

$$y_0(x, t) = y^* \Upsilon \left(\frac{x}{L^*}, \frac{t}{T^*} \right) \quad (5)$$

121 where $\Upsilon(\cdot, \cdot)$ is a function of order 1.

122 3.2 Fast dynamics

123 After the impact (for $t \geq 0$), the governing equation (1) must be solved alongside the complementarity
 124 condition (2). Due to the piecewise linear nature of the problem, the solution is expressed as the sum of
 125 a slow response, $y_0(x, t)$, and a fast response, $z(x, t)$. Substituting $y(x, t) = y_0(x, t) - z(x, t)$ into (1) and
 126 subtracting (3), it is found that the fast response $z(x, t)$ satisfies

$$\mu \partial_t^2 z + EI \partial_x^4 z = r. \quad (6)$$

127 The fast dynamics take place on a timescale t^* assumed to be short compared to the macroscopic timescale,
 128 i.e. $t^* = \varepsilon T^*$ where $\varepsilon \ll 1$ is a small parameter. The corresponding lengthscale is naturally chosen as
 129 $\ell^* = \sqrt{\varepsilon} L^*$ because of the differentiation orders with respect to time and space in the governing equation,
 130 respectively. Introducing the dimensionless time τ and coordinate ξ

$$\tau = t/t^* \quad \text{and} \quad \xi = x/\ell^*, \quad (7)$$

131 the governing equation (6) becomes

$$\left(\frac{T^*}{t^*} \right)^2 \partial_\tau^2 \frac{z}{y^*} + \left(\frac{L^*}{\ell^*} \right)^4 \partial_\xi^4 \frac{z}{y^*} = \frac{r}{p^*} \quad (8)$$

132 or

$$\partial_\tau^2 \eta + \partial_\xi^4 \eta = \tilde{r} \quad (9)$$

133 where

$$\eta(\xi, \tau) = \frac{z(\xi \ell^*, \tau t^*)}{\varepsilon y^*} \quad \text{and} \quad \tilde{r}(\xi, \tau) = \frac{r(\xi \ell^*, \tau t^*)}{\varepsilon^{-1} p^*} \quad (10)$$

134 are the dimensionless displacement and reacting pressure on the wall. In (10), the choice to scale the
 135 transverse displacement by εy^* is justified by the distinguished limit obtained later. This indicates that
 136 the fast dynamics solution involves small displacements, of order εy^* , together with large reacting pressures,
 137 of order $\varepsilon^{-1} p^*$.

138 3.3 Early matching of solutions

139 In classical perturbation methods, asymptotic solutions are typically developed separately for the far and
 140 near fields and then matched in a subsequent step. However, for the present problem, deriving the near-
 141 field solution at the fast time scale is difficult. To simplify the process and avoid deriving the general
 142 solution explicitly, we directly anticipate the matching of the fast and slow solutions.

143 To achieve this, the slow dynamics solution (5) is reformulated using stretched coordinates as

$$\begin{aligned} y_0 &= y^* \Upsilon(\xi \sqrt{\varepsilon}, \tau \varepsilon) \\ &= y^* \left[\Upsilon(0, 0) + \sqrt{\varepsilon} \xi \Upsilon'(0, 0) + \varepsilon \left(\frac{\xi^2}{2} \Upsilon''(0, 0) + \tau \dot{\Upsilon}(0, 0) \right) \right. \\ &\quad + \varepsilon \sqrt{\varepsilon} \left(\frac{\xi^3}{6} \Upsilon'''(0, 0) + \xi \tau \dot{\Upsilon}'(0, 0) \right) \\ &\quad \left. + \varepsilon^2 \left(\frac{\xi^4}{24} \Upsilon''''(0, 0) + \frac{\xi^2 \tau}{2} \dot{\Upsilon}''(0, 0) + \frac{\tau^2}{2} \ddot{\Upsilon}(0, 0) \right) + \mathcal{O}(\varepsilon^{5/2}) \right] \end{aligned} \quad (11)$$

144 where the prime ' symbol denotes differentiation with respect to the first argument and the dot symbol
 145 $\dot{\cdot}$ denotes differentiation with respect to the second argument. In this expression, $\Upsilon(0, 0) = \Upsilon'(0, 0) = 0$
 146 and $\Upsilon''(0, 0) \leq 0$ and $\dot{\Upsilon}(0, 0) > 0$ because of the occurrence of the contact at $t = 0$ precisely. If one of
 147 these conditions was not met, the contact would not take place at time $t = 0$, or would have taken place
 148 before. We also assume that the contact is symmetrical, discarding therefore odd powers in ξ . This also
 149 means that *rolling contacts* are not studied in this paper.

150 Equation (11) therefore simplifies into

$$\frac{y_0}{\varepsilon y^*} = \frac{\xi^2}{2} \Upsilon''(0, 0) + \tau \dot{\Upsilon}(0, 0) + \varepsilon \left(\frac{\xi^4}{24} \Upsilon''''(0, 0) + \frac{\xi^2 \tau}{2} \dot{\Upsilon}''(0, 0) + \frac{\tau^2}{2} \ddot{\Upsilon}(0, 0) \right) + \mathcal{O}(\varepsilon^2). \quad (12)$$

151 At leading order (in ε , this is actually why we have chosen εy^* to scale z), the stretched macroscopic
 152 solution is therefore governed by the dimensionless curvature of the beam at contact

$$\kappa := -\Upsilon''(0, 0) = -\frac{\partial_x^2 y_0(0, 0) L^{*2}}{y^*}, \quad (13)$$

153 (notice a minus sign is introduced in this definition so that $\kappa \geq 0$), and the dimensionless impact velocity

$$\nu := \dot{\Upsilon}(0, 0) = \frac{\partial_t y_0(0, 0) T^*}{y^*}. \quad (14)$$

154 The second order solution involves a higher curvature $\partial_x^4 y_0$, the rate of change of the curvature $\partial_x^2 \partial_t y_0$ and
155 the acceleration $\partial_t^2 y_0$ at impact point. In a dimensionless form, they read

$$\psi := \Upsilon''''(0, 0) = \frac{\partial_x^4 y_0(0, 0) L^{*4}}{y^*}, \quad \alpha := \ddot{\Upsilon}(0, 0) = \frac{\partial_t^2 y_0(0, 0) T^{*2}}{y^*}, \quad (15)$$

$$\chi := \dot{\Upsilon}''(0, 0) = \frac{\partial_x^2 \partial_t y_0(0, 0) L^{*2} T^*}{y^*}. \quad (16)$$

156 The velocity ν is strictly positive, $\nu > 0$, while the acceleration α can be either positive (the beam
157 accelerates toward the wall) or negative.

158 In the following, we will focus on the leading order solution of the problem. This early matching of the
159 inner and outer solutions shows that quantities such as the beam acceleration at the time and location of
160 impact concern higher order corrections to the leading order solution.

161 3.4 Matching and complementarity conditions

162 Equation (9) governing the fast dynamics solution taking place near the contact point cannot be sup-
163 plemented with the boundary conditions of the macroscopic problem. Instead, matching conditions need
164 to be invoked. They need to translate that the total solution $y(x, t) = y_0(x, t) - z(x, t) = y_0(x, t) -$
165 $\varepsilon y^* \eta(x/\ell^*, t/t^*)$ matches the slow dynamics solution $y_0(x, t)$ in the far-field, i.e.

$$\lim_{\xi \rightarrow \pm\infty} \eta(\xi, \tau) = 0 \quad ; \quad \lim_{\xi \rightarrow \pm\infty} \partial_\xi \eta(\xi, \tau) = 0. \quad (17)$$

166 Furthermore, on the contact domain \mathcal{D} , the complementarity equation requires that $y_0(x, t) - z(x, t) = 0$,
167 that is $\Upsilon(\sqrt{\varepsilon}\xi, \varepsilon\tau) - \varepsilon\eta(\xi, \tau) = 0$. Introducing the expansion (12), this condition also reads, in a strict
168 sense,

$$\eta(\xi, \tau) = \nu\tau - \kappa \frac{\xi^2}{2} + \varepsilon \left(\psi \frac{\xi^4}{24} + \chi \frac{\xi^2 \tau}{2} + \alpha \frac{\tau^2}{2} \right) + \mathcal{O}(\varepsilon^2) \quad (18)$$

169 which is readily interpreted as a set of complementarity conditions, each one corresponding to a different
170 scale of the problem.

171 From a different perspective, a discrete contact transforms into a double or continuous contact as soon
172 as the curvature at contact reaches that of the wall [13, 18]. This condition is formally written by equating
173 the total curvature $\partial_x^2 y(x, t) = \partial_x^2 y_0(x, t) - \partial_x^2 z(x, t)$ to zero, i.e., with dimensionless quantities

$$\Upsilon''(\xi\sqrt{\varepsilon}, \tau\varepsilon) - \partial_\xi^2 \eta(\xi, \tau) = 0, \quad (19)$$

174 since the wall is considered to be straight. Substituting for $\Upsilon''(\xi\sqrt{\varepsilon}, \tau\varepsilon)$ the same development as in (12),
175 we have

$$\partial_\xi^2 \eta(\xi, \tau) = -\kappa + \varepsilon \left(\frac{\xi^2}{2} \psi + \chi \tau \right) + \mathcal{O}(\varepsilon^2) \quad (20)$$

176 which expresses, in a weaker sense, the complementarity condition. Both (18) and (20) are used in the
177 sequel, whenever convenient or appropriate.

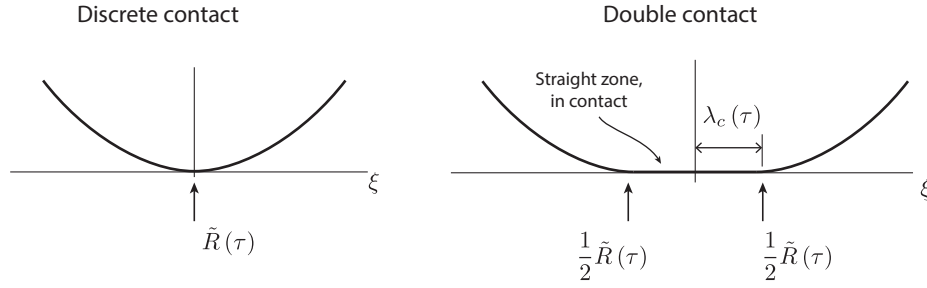


Fig. 2: Two types of contact pattern existing at leading order: discrete contact (point-like contact) and double contact (two discrete forces with a straight zone in contact in-between).

178 3.5 Interpretation of the small parameter ε

179 The above considerations show that the problem is formulated at leading order with four parameters,
 180 namely EI and μ , together with the curvature $\kappa y^*/L^{*2}$ and velocity $v_o := \nu y^*/T^*$ at impact. They are
 181 expressed in terms of three independent units (M, L, T), defining a short timescale associated with the
 182 fast dynamics problem

$$t^* = \frac{1}{v_o^2} \sqrt{\frac{EI}{\mu}}. \quad (21)$$

183 The small parameter introduced before is therefore re-evaluated as

$$\varepsilon = \frac{t^*}{T^*} = \left(\frac{L^*}{v_o T^*} \right)^2 = \left(\sqrt{\frac{EI}{\mu}} \frac{1}{v_o L^*} \right)^2. \quad (22)$$

184 The validity of the solutions developed in this paper, under consideration of separate timescales, requires
 185 this parameter to be small ($\varepsilon \ll 1$), which means, a high-velocity impact (v_o) or a long (L^*), heavy (μ) or
 186 very flexible beam (EI), relatively speaking.

187 4 Fundamental solutions of the problem

188 At leading order, the problem consists therefore in the analysis of an infinitely long beam with constant
 189 velocity and curvature impacting a rigid wall. In this scenario, several contact patterns may develop after
 190 impact: at leading order, either a discrete contact, either two symmetrical discrete forces moving away
 191 from each other, see Figure 2. The detailed expressions of the reactions and position of reaction forces
 192 are developed in Sections 5.2 and 5.3. They are based on the fundamental general solutions established in
 193 this Section, which are obtained from the impulse response function.

194 4.1 Impulse response function

195 The impulse response (or kernel) function $\mathcal{K}(\xi, \tau)$ associated with the vibrations of an infinite beam
 196 satisfies the unit impulsive force problem,

$$\partial_\tau^2 \mathcal{K} + \partial_\xi^4 \mathcal{K} = \delta(\tau) \delta(\xi) \quad (23)$$

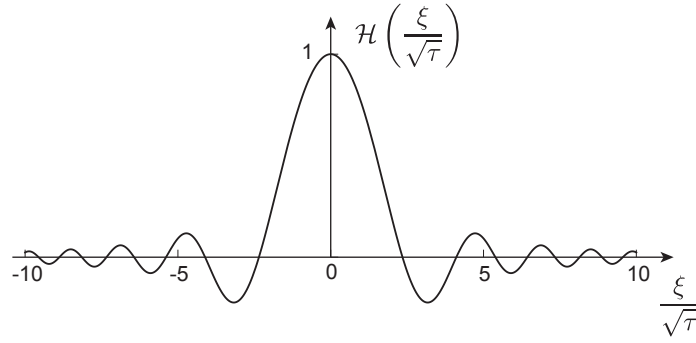


Fig. 3: Self-similar response of an infinite beam under impulsive loading.

197 for $\xi \in [-\infty; +\infty]$ and with initial rest conditions. The self-similar solution of this well-known problem
198 [2, 4] is

$$\mathcal{K}(\xi, \tau) = \sqrt{\frac{\tau}{2\pi}} \mathcal{H}\left(\frac{\xi}{\sqrt{\tau}}\right) \quad (24)$$

199 with

$$\mathcal{H}(\beta) = \sqrt{\frac{\pi}{2}} \beta \left[\mathcal{S}\left(\frac{\beta}{\sqrt{2\pi}}\right) - \mathcal{C}\left(\frac{\beta}{\sqrt{2\pi}}\right) \right] + \sqrt{2} \sin\left(\frac{\pi + \beta^2}{4}\right) \quad (25)$$

200 and where $\mathcal{S}(\cdot)$ and $\mathcal{C}(\cdot)$ stand for Fresnel integrals of the first kind [19]. This function is represented in
201 Fig. 3. The second derivative of $\mathcal{H}(\beta)$, required later, reads

$$\mathcal{H}''(\beta) = \frac{1}{2} \sin\left(\frac{\beta^2}{4}\right) - \frac{1}{2} \cos\left(\frac{\beta^2}{4}\right). \quad (26)$$

202 4.2 Pattern 1: Discrete contact at $\xi = 0$

203 In the case of a discrete contact between the beam and the rigid wall, see Fig. 2, the reaction is written
204 as a Dirac function, $\tilde{r}(\xi, \tau) = \tilde{R}(\tau) \delta(\xi)$, and the generic governing equation reads

$$\partial_\tau^2 \eta + \partial_\xi^4 \eta = \tilde{R}(\tau) \delta(\xi) \quad (27)$$

205 where $\tilde{R}(\tau)$ will be determined from the complementarity condition. The solution of this equation is
206 written as a convolution of the point reaction and the impulse response function

$$\eta(\xi, \tau) = \int_0^\tau \tilde{R}(\bar{\tau}) \mathcal{K}(\xi, \tau - \bar{\tau}) d\bar{\tau} = \frac{1}{\sqrt{2\pi}} \int_0^\tau \tilde{R}(\bar{\tau}) \sqrt{\tau - \bar{\tau}} \mathcal{H}\left(\frac{\xi}{\sqrt{\tau - \bar{\tau}}}\right) d\bar{\tau}. \quad (28)$$

207 The dimensionless curvature is obtained by differentiation of (28),

$$\partial_\xi^2 \eta(\xi, \tau) = \frac{1}{2\sqrt{2\pi}} \int_0^\tau \frac{\tilde{R}(\bar{\tau})}{\sqrt{\tau - \bar{\tau}}} \left[\sin\left(\frac{\xi^2}{4(\tau - \bar{\tau})}\right) - \cos\left(\frac{\xi^2}{4(\tau - \bar{\tau})}\right) \right] d\bar{\tau}. \quad (29)$$

208 In particular, the dimensionless curvature at the contact point is given by

$$\partial_{\xi}^2 \eta(0, \tau) = \frac{-1}{2\sqrt{2\pi}} \int_0^{\tau} \frac{\tilde{R}(\bar{\tau})}{\sqrt{\tau - \bar{\tau}}} d\bar{\tau}. \quad (30)$$

209 4.3 Pattern 2: Double contact

210 In another contact pattern, the contact between the beam and the wall consists in two symmetric con-
211 centrated forces, located at $\xi = \pm\lambda_c(\tau)$ and moving away from each other, see Fig. 2. In this case, the
212 reaction is written as a sum of two Dirac functions and the governing equation reads

$$\partial_{\tau}^2 \eta + \partial_{\xi}^4 \eta = \frac{1}{2} \tilde{R}(\tau) \delta(\xi - \lambda_c(\tau)) + \frac{1}{2} \tilde{R}(\tau) \delta(\xi + \lambda_c(\tau)) \quad (31)$$

213 with $\tilde{R}(\tau)$ the total reaction on the wall. In this case, the response reads

$$\eta(\xi, \tau) = \frac{1}{2} \int_0^{\tau} \tilde{R}(\bar{\tau}) [\mathcal{K}(\xi - \lambda_c(\bar{\tau}), \tau - \bar{\tau}) + \mathcal{K}(\xi + \lambda_c(\bar{\tau}), \tau - \bar{\tau})] d\bar{\tau}. \quad (32)$$

214 The dimensionless curvature of the beam is obtained after two differentiations with respect to ξ . In
215 particular, for $\xi = 0$,

$$\partial_{\xi}^2 \eta(0, \tau) = \frac{1}{\sqrt{2\pi}} \int_0^{\tau} \frac{\tilde{R}(\bar{\tau})}{\sqrt{\tau - \bar{\tau}}} \mathcal{H}''\left(\frac{\lambda_c(\bar{\tau})}{\sqrt{\tau - \bar{\tau}}}\right) d\bar{\tau}. \quad (33)$$

216 5 Perturbation analysis solution

217 5.1 General equations

218 The problem at hand is governed by the linear partial differential equation (9), supplemented with the
219 complementarity condition (18). Perturbation approaches are not well documented for linear complemen-
220 tarity problems. However, as seen next, it seems that standard methods in perturbation methods could
221 provide an interesting approach to the solution of this class of problems. Considering the usual ansatz

$$\eta = \eta_0 + \varepsilon \eta_1 + \varepsilon^2 \eta_2 + \dots, \quad (34)$$

222 substituting this ansatz into the governing equation and balancing the similar powers of ε yields

$$\text{ord}(\varepsilon^0) : \quad \partial_{\tau}^2 \eta_0 + \partial_{\xi}^4 \eta_0 = \tilde{r}_0 \quad (35)$$

$$\text{ord}(\varepsilon^1) : \quad \partial_{\tau}^2 \eta_1 + \partial_{\xi}^4 \eta_1 = \tilde{r}_1 \quad (36)$$

⋮

223 where the expansion

$$\tilde{r} = \tilde{r}_0 + \varepsilon \tilde{r}_1 + \varepsilon^2 \tilde{r}_2 + \dots \quad (37)$$

224 is naturally considered for the unknown reaction on the wall. This a priori unknown reaction satisfies the
225 complementarity condition (18) which is also considered at the different orders:

$$\text{ord}(\varepsilon^0) : \quad \eta_0(\xi, \tau) = \nu\tau - \kappa \frac{\xi^2}{2} \quad (38)$$

$$\text{ord}(\varepsilon^1) : \quad \eta_1(\xi, \tau) = \psi \frac{\xi^4}{24} + \chi \frac{\xi^2 \tau}{2} + \alpha \frac{\tau^2}{2} \quad (39)$$

⋮

226 At leading order, the governing equation (35) needs to be solved together with the complementarity
 227 equation (38) which should be satisfied for $\xi \in \mathcal{D}(\tau)$. Two different scenarii are possible: the impact results
 228 either in a discrete contact $\mathcal{D} = \{0\}$, either a double contact at the edges of domain $\mathcal{D} = [-\lambda_c(\tau), \lambda_c(\tau)]$.

229 5.2 Impact initiated with a discrete contact

230 Let us first consider the case where a discrete contact is initiated at time $t = 0$. A reaction force $\tilde{R}_0(\tau)$
 231 develops and the fast dynamics solution is given by (28). In particular, at the contact point ($\xi = 0$),

$$\eta_0(0, \tau) = \int_0^\tau \tilde{R}_0(\bar{\tau}) \sqrt{\frac{\tau - \bar{\tau}}{2\pi}} d\bar{\tau}. \quad (40)$$

232 Substitution into the complementary condition (38) yields

$$\int_0^\tau \tilde{R}_0(\bar{\tau}) \sqrt{\frac{\tau - \bar{\tau}}{2\pi}} d\bar{\tau} = \nu\tau. \quad (41)$$

233 This Volterra equation of the first kind for $\tilde{R}_0(\bar{\tau})$ admits a solution in the form

$$\tilde{R}_0(\tau) = 2\sqrt{\frac{2}{\pi}} \frac{r_0}{\sqrt{\tau}}. \quad (42)$$

234 Indeed, by substituting this ansatz into the equation above, the integral is evaluated to $r_0\tau$, which shows
 235 that the complementarity condition is trivially satisfied by choosing

$$r_0 = \nu. \quad (43)$$

236 Because $\nu > 0$, the wall reaction is indeed positive at leading order. It decreases as $r_0\tau^{-1/2}$. Putting all
 237 arguments together, the fast dynamics solution (28) is finally given by

$$\eta(\xi, \tau) = \frac{2\nu}{\pi} \int_0^\tau \sqrt{\frac{\tau - \bar{\tau}}{\bar{\tau}}} \mathcal{H}\left(\frac{\xi}{\sqrt{\tau - \bar{\tau}}}\right) d\bar{\tau}. \quad (44)$$

238

239 This discrete contact solution ceases to be valid as soon as the total curvature of the beam at the contact
 240 point vanishes, after which the beam unfolding is completed and a continuous contact pattern takes place.
 241 The total curvature of the beam, $\kappa + \partial_\xi^2 \eta(0, \tau)$, is obtained by adding the initial (slow timescale) curvature

242 and the curvature associated with the response under the reaction force. The later is expressed by (30)
 243 with the reaction force given by (42),

$$\kappa - \frac{1}{2\sqrt{2\pi}} \int_0^\tau \frac{\tilde{R}(\bar{\tau}) d\bar{\tau}}{\sqrt{\tau - \bar{\tau}}} = \kappa - \frac{r_0}{\pi} \int_0^\tau \frac{d\bar{\tau}}{\sqrt{\bar{\tau}}\sqrt{\tau - \bar{\tau}}} = \kappa - \frac{\nu}{\pi} \int_0^1 \frac{d\gamma}{\sqrt{\gamma}\sqrt{1-\gamma}} = \kappa - \nu. \quad (45)$$

244 Interestingly, the total curvature remains constant, at leading order, in the post-impact phase with a
 245 single discrete contact. Consequently, whether the contact pattern is a discrete or a continuous (double
 246 force) pattern, only depends on $\kappa - \nu$, and does not change, at leading order, during the post-impact
 247 phase. For small impact velocity, $0 < \nu \leq \kappa$, a single contact develops. For large impact velocity, $\kappa < \nu$,
 248 a double contact pattern develops. This solution is studied in the following Section.

249 5.3 Impact initiated with a double contact

250 When $\kappa < \nu$, the impact velocity is so high that the beam instantaneously unfolds. A continuous contact
 251 between the beam and the wall develops, over a finite distance $\xi \in [-\lambda_c(\tau); \lambda_c(\tau)]$. In the most general
 252 case, the reaction $r(\xi)$ is composed of two discrete forces at the extremities and a distributed pressure
 253 in-between. We show that this distributed pressure is equal to zero for constant velocity impact, at
 254 leading. Indeed, along the contact domain $\mathcal{D}(\tau) = [-\lambda_c(\tau); \lambda_c(\tau)]$, the deformed configuration of the
 255 beam complies with the geometry of the wall so that (38) holds over the finite domain \mathcal{D} , i.e. $\eta_0(\xi, \tau) =$
 256 $\nu\tau - \kappa\xi^2/2$. Substituting this expression for $\eta_0(\xi, \tau)$ in the governing equation (35) yields $\tilde{r}_0 = 0$, which
 257 indicates that there is no distributed reaction on the wall.

258 The reaction pattern therefore simply consists in two traveling forces which are symmetrically *ejected*
 259 from abscissa $\xi = 0$ at time $\tau = 0$, and are located at the ends of the growing contact region. The portion
 260 of beam left in-between is instantaneously at rest as it reaches the wall, as a consequence of the traveling
 261 forces. The magnitude $\tilde{R}_0(\tau)$ and the locations $\pm\lambda_c(\tau)$ of these contact forces are determined next.

262 The deformed configuration is given by equation (32), and in particular, for $\xi = 0$,

$$\eta_0(0, \tau) = \int_0^\tau \tilde{R}_0(\bar{\tau}) \mathcal{K}(\lambda_c(\bar{\tau}), \tau - \bar{\tau}) d\bar{\tau} \quad (46)$$

263 which should be equal to $\nu\tau$ to meet the complementarity equation. This is another integral equation in
 264 the two unknowns $\tilde{R}_0(\tau)$ and $\lambda_c(\tau)$. Seeking again a solution in the form (42), this yields

$$\frac{2r_0}{\pi} \int_0^\tau \sqrt{\frac{\tau - \bar{\tau}}{\bar{\tau}}} \mathcal{H}\left(\frac{\lambda_c(\bar{\tau})}{\sqrt{\tau - \bar{\tau}}}\right) d\bar{\tau} = \nu\tau. \quad (47)$$

265 The integral on the left can be rewritten with the scaled time $\gamma = \bar{\tau}/\tau$,

$$\frac{2}{\pi} r_0 \tau \int_0^1 \sqrt{\frac{1-\gamma}{\gamma}} \mathcal{H}\left(\frac{\sqrt{\gamma}}{\sqrt{1-\gamma}} \frac{\lambda_c(\gamma\tau)}{\sqrt{\gamma\tau}}\right) d\gamma = \nu\tau. \quad (48)$$

266 This expression shows that the result of the integral should be trivially independent of τ . This is only
 267 possible if the argument of $\mathcal{H}[\cdot]$ is independent of τ , which yields the functional form

$$\lambda_c(\tau) = \lambda_0 \sqrt{\tau}. \quad (49)$$

268 This indicates that the contact length grows like $\sqrt{\tau}$ at leading order. The velocity at which the contact
 269 length increases is therefore infinite when contact occurs, hence the use of wording 'ejection'. Substituting
 270 this form for $\lambda_c(\tau)$ into (47), and considering the change of variable

$$\beta = \sqrt{\frac{\bar{\tau}}{\tau - \bar{\tau}}}, \quad (50)$$

271 the complementarity condition at $\xi = 0$ becomes

$$\frac{2r_0}{\pi} \int_0^{+\infty} \frac{2\tau \mathcal{H}(\lambda_0 \beta)}{(1 + \beta^2)^2} d\beta = \nu \tau. \quad (51)$$

272 Simplification by τ yields

$$r_0 \varphi_\nu(\lambda_0) = \nu, \quad (52)$$

273 where $\varphi_\nu(\lambda)$ is defined as

$$\varphi_\nu(\lambda) := \frac{4}{\pi} \int_0^{+\infty} \frac{\mathcal{H}(\lambda \beta)}{(1 + \beta^2)^2} d\beta \quad (53)$$

274 This function is explicitly given in Appendix A and represented in Figure 4-a.

275 A second equation to determine r_0 and λ_0 is obtained by enforcing that the two traveling forces are
 276 such that the curvature of the beam is equal to the (zero) curvature of the wall along the contact. This
 277 is expressed by substituting (42) into (33). In particular, this curvature compliance should be valid for
 278 $\xi = 0$. Specifying (20) for $\xi = 0$ and using (46) for η_0 yields

$$\partial_\xi^2 \eta_0(0, \tau) = \frac{2}{\pi} \int_0^\tau \frac{r_0}{\sqrt{\bar{\tau}}(\tau - \bar{\tau})} \mathcal{H}'' \left(\lambda_0 \sqrt{\frac{\bar{\tau}}{\tau - \bar{\tau}}} \right) d\bar{\tau} = -\kappa. \quad (54)$$

279 Introducing the same change of variable $\beta = \sqrt{\bar{\tau}/(\tau - \bar{\tau})}$ as before, this condition becomes

$$r_0 \varphi_\kappa(\lambda_0) = \kappa \quad (55)$$

280 where $\varphi_\kappa(\lambda)$ is defined as

$$\varphi_\kappa(\lambda) := -\frac{4}{\pi} \int_0^{+\infty} \frac{\mathcal{H}''(\lambda \beta)}{1 + \beta^2} d\beta = \frac{8}{\pi \lambda^2} \int_0^{+\infty} \frac{(1 - 3\beta^2) \mathcal{H}(\lambda \beta)}{(1 + \beta^2)^3} d\beta, \quad (56)$$

281 and where the second equality is obtained after two integrations by parts. This function is explicitly given
 282 in Appendix A and represented in Figure 4-a.

283 Equations (52) and (55) form a set of two algebraic transcendental equations in the unknowns r_0 and
 284 λ_0 which govern the magnitude and position of the two reactions on the wall, at leading order. Eliminating
 285 r_0 readily provides a single equation in λ_0 ,

$$\frac{\varphi_\kappa(\lambda_0)}{\varphi_\nu(\lambda_0)} = \frac{\kappa}{\nu}. \quad (57)$$

286 Evaluation of r_0 then follows by back-substitution in either (52) or (55). By noticing that $\varphi_\kappa(\lambda_0)/\varphi_\nu(\lambda_0)$
 287 is monotonically decreasing from 1 (for $\lambda_0 = 0$) to 0 as $\lambda_0 \rightarrow +\infty$, see Figure 4-a, and recalling that

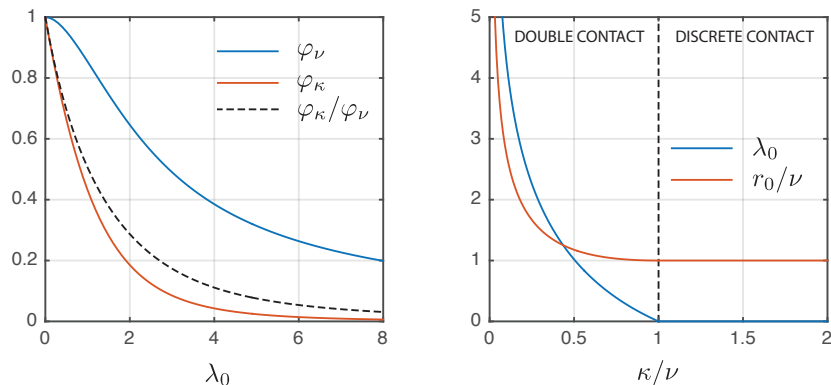


Fig. 4: (a) Functions φ_ν and φ_κ which are used to solve the problem at leading order; (b) Solutions λ_0 and r_0/ν . of the double contact problem.

κ/ν	0	0.02	0.04	0.06	0.08	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1
λ_0	∞	9.976	7.007	5.661	4.837	4.258	2.704	1.916	1.398	1.019	0.725	0.490	0.297	0.136	0
r_0/ν	∞	6.259	4.412	3.585	3.088	2.745	1.885	1.511	1.304	1.177	1.098	1.048	1.019	1.004	1

Tab. 1: Some numerical values for λ_0 and r_0/ν , given as a function of κ/ν .

288 the double contact solution only occurs for $\nu > \kappa$, we see that the problem possesses a unique solution.
 289 Furthermore, for $\kappa/\nu = 1$, the solution is $\lambda_0 = 0$ and $r_0 = \nu (= \kappa)$, which is consistent with the discrete
 290 contact solution developed previously. For other values of κ/ν , Figure 4-b shows the solutions λ_0 and
 291 r_0/ν . Table 1 summarizes some numerical values.

292 The values of the two unknowns r_0 and λ_0 have been obtained by expressing the complementarity
 293 condition at $\xi = 0$, in both the strict and weak senses. Since the derivation was obtained for an assumed
 294 functional form for $R_0(\tau)$, it is necessary to show that the complementarity condition is satisfied every-
 295 where along the contact domain $\mathcal{D}(\tau)$, not only for $\xi = 0$. This proof involves heavier mathematical
 296 derivations, given in Appendix B. This being proved, the contact problem initiated with a double contact
 297 force pattern admits therefore the unique solution

$$\eta(\xi, \tau) = \frac{r_0}{\pi} \int_0^\tau \sqrt{\frac{\tau - \bar{\tau}}{\bar{\tau}}} \left[\mathcal{H} \left(\frac{\xi - \lambda_0 \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}} \right) + \mathcal{H} \left(\frac{\xi + \lambda_0 \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}} \right) \right] d\bar{\tau}, \quad (58)$$

298 valid, $\forall \xi \in \mathbb{R}$, $\tau > 0$. When $\lambda_0 = 0$ and $r_0 = \nu$, this expression degenerates into (44).

299 6 Discussion

300 6.1 Illustration

301 Figure 5 illustrates the different types of solutions observed in the beam-wall interaction problem. The left
 302 panels depict an impact initiated with a discrete contact ($\nu < \kappa$), while the right panels show an impact
 303 characterized by a double contact ($\nu > \kappa$). In all cases, the macroscopic solution, $\nu\tau - \frac{\kappa\xi^2}{2}$, is represented
 304 by blue dashed lines. This macroscopic solution corresponds to the configuration of the beam that would
 305 continue to move as a rigid body if the wall were absent. Snapshots are provided for $\tau \in \{0.1, 0.25, 0.5\}$.

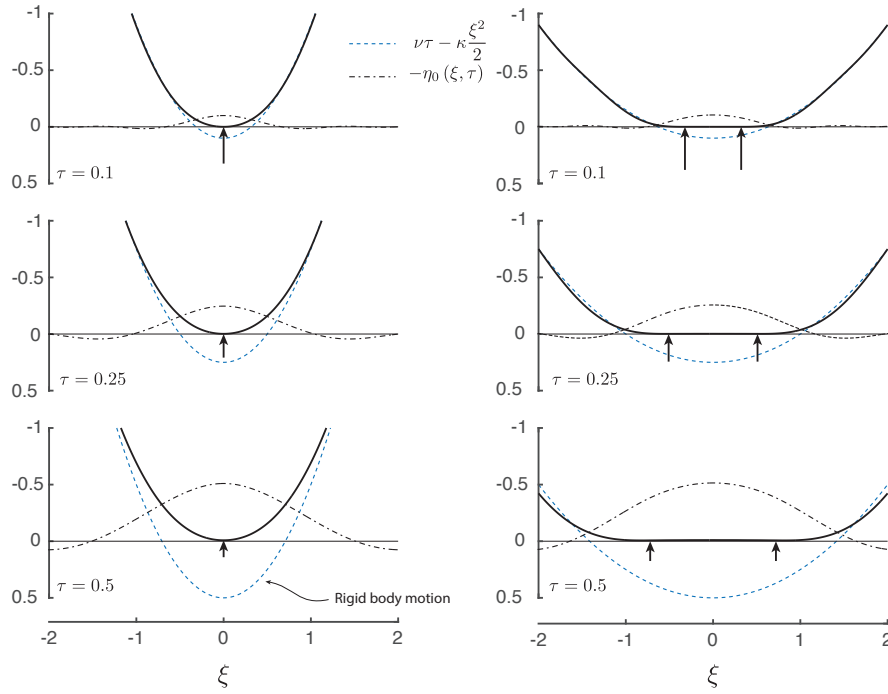


Fig. 5: Leading order solution of the beam impacting a rigid wall. Snapshots of the solution at time $\tau = 0.1, 0.25, 0.5$ for an impact velocity $\nu = 1$ (a) Discrete contact, $\kappa = 2(\geq \nu)$; (b) Double contact, $\kappa = 0.5(\leq \nu)$

306 The negative of the fast dynamics response, $-\eta_0(\xi, \tau)$, is represented by dash-dot lines. The expressions
 307 for η_0 are provided in Eqs. (44) and (58) for the cases of discrete and double contact, respectively.

308 The actual deformation of the beam is the sum of the macroscopic rigid body motion and the fast
 309 dynamics response. For discrete contact, the evolution of the reaction force, which is proportional to
 310 $1/\sqrt{\tau}$, ensures that the contact point remains fixed after the initial contact. The magnitude of the force is
 311 indicated by the size of the arrows in the figure. For the double contact case, the decrease in the contact
 312 reaction force, combined with the splitting of the two contact forces that localize at a growing distance
 313 $\lambda_c \sim \sqrt{\tau}$, ensures that the segment of the beam between the two contact forces remains stationary and
 314 maintains perfect contact with the rigid wall following the impact.

315 6.2 Beyond the leading order solution

316 The solution presented earlier corresponds to the leading-order approximation, which simplifies the prob-
 317 lem by considering an infinite beam with constant curvature and velocity. Extending this analysis to
 318 higher-order corrections introduces additional terms involving higher-order time and space derivatives.
 319 Specifically, these include the acceleration (α), the fourth-degree spatial derivative (ψ), and the rate
 320 of change of curvature (χ). These dimensionless quantities, defined in Equation (18), enable matching
 321 between the fast and slow dynamics, thus providing a more comprehensive description of the system's
 322 behavior. While a formal proof lies beyond the scope of this study, it is reasonable to hypothesize that the
 323 corrections are described with a Frobenius series expansion. For example, in the case of discrete contact,
 324 the reaction force could be expanded as $R_0 \sim \frac{1}{\sqrt{\tau}}(\nu + \varepsilon\alpha\tau + \dots)$.

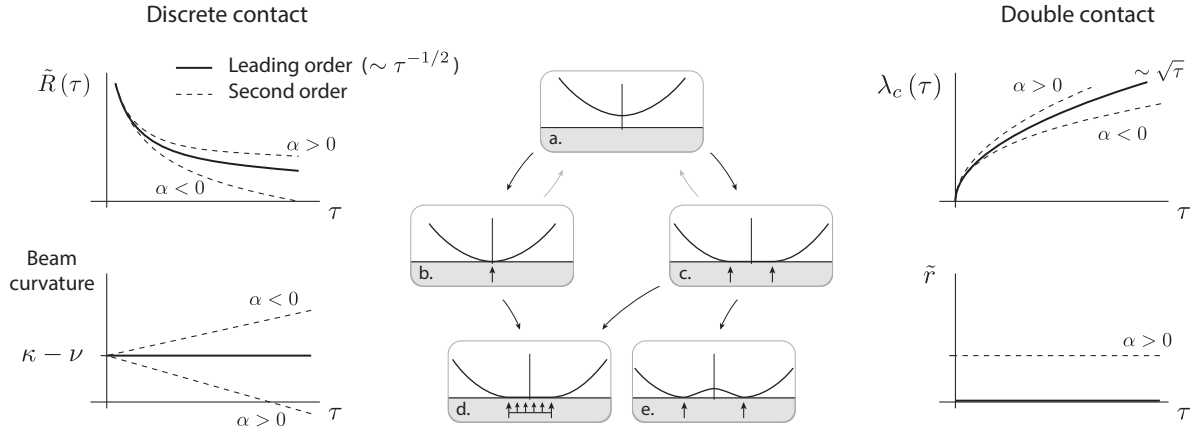


Fig. 6: Overview of contact patterns and possible transitions. a. no contact, b. discrete contact, c. double contact without wall pressure, d. double contact with pressure on wall, e. double contact with detached contact in-between.

325 At leading order, the reaction force decreases proportionally to $\nu\tau^{-1/2}$. However, as τ approaches $1/\varepsilon$,
 326 the first correction becomes significant. When the acceleration parameter α is positive, the reaction force
 327 initially increases, and the curvature at the contact point decreases. This behavior can potentially cause a
 328 transition from a discrete to a distributed contact pattern, a transition that is not captured by the leading-
 329 order solution for which the total beam curvature is an invariant. Conversely, for negative acceleration
 330 toward the wall ($\alpha < 0$), the reaction force decays more rapidly, eventually becoming negative and leading
 331 to detachment of the beam from the support, returning to a contact-free configuration as illustrated in
 332 Figure 6. This latter phenomenon, however, is practically less relevant, as the time required for detachment
 333 ($-\nu/(\varepsilon\alpha)$) is comparable to the macroscopic timescale, invalidating the timescale separation assumption.
 334 In very specific scenarios where $|\alpha| \gg \nu$, corresponding to grazing impacts with small finite momentum
 335 transfer, the duration of the contact is short enough so that the multiple timescale analysis remains valid.
 336 In all other cases, the model remains limited in its ability to fully describe scenarios involving beam
 337 rebound or long-duration interactions.

338 A notable feature of the higher-order correction arises in the context of continuous contact. While the
 339 leading-order reaction pattern consists solely of two concentrated forces at the contact points (pattern c. in
 340 Figure 6), higher-order corrections introduce a distributed pressure between these points. This distributed
 341 pressure stems from the compatibility condition along the finite contact length. Substituting the higher-
 342 order expression for the deformation (18) into the governing equation (9) reveals that the distributed
 343 reaction force is given by $\tilde{r} = \varepsilon(\psi + \alpha)$. The behavior of the beam between the two concentrated forces
 344 is thus determined by the sign of $\psi + \alpha$. A positive value indicates a pressing distributed contact on the
 345 wall (pattern d. in Figure 6) while a negative value is not physically acceptable and actually results in a
 346 detached zone between the two contact points (pattern e. in Figure 6).

347 Overall, the higher-order analysis can result in additional contact patterns that are inaccessible in the
 348 leading-order solution, as illustrated in Figure 6.

349 6.3 Utility of the current solution in numerical methods

350 The results presented in this study offer significant potential utility for applied mathematicians developing
 351 numerical techniques, particularly in two specific contexts.

352 The first application involves using the analytical solutions developed herein to validate numerical
353 codes. A particularly challenging scenario for numerical methods is the case of constant velocity impact.
354 Theoretically, this scenario results in a portion of the beam remaining in continuous contact with the rigid
355 wall but without any reaction force over a finite length. Capturing this behavior accurately is inherently
356 difficult for numerical methods. Numerically, the absence of a reaction force may be misrepresented as
357 either a small penetration of the wall, resulting in an artificial non-zero reaction force, or as a detached
358 configuration, creating a slight gap between the beam and the wall. Both artifacts could potentially be
359 controlled by refining the numerical discretization, but they remain indicative of the difficulty in resolving
360 this behavior. Additionally, the presence of local penetration or detachment, arising from numerical
361 artifacts, is unlikely to be uniformly distributed along the contact length. This could lead to high-frequency,
362 short-wavelength perturbations superimposed on the expected theoretical solution, further complicating
363 the numerical representation of the problem.

364 Along the same line, the current solutions highlight two clearly different contact patterns, discrete vs.
365 continuous contact, and capturing appropriately the transition from one pattern to the other is something
366 that could be used for benchmarking numerical codes.

367 The solutions derived in this paper could also potentially improve existing numerical methods by
368 incorporating insights from the analytical solutions. A critical challenge in numerical simulations of beam
369 impact is accurately capturing the rapid transfer of momentum as contact occurs. Theoretically, the
370 reaction force is both initially infinite and rapidly decreasing, necessitating extremely small time steps in
371 numerical simulations to resolve this behavior accurately. The analytical solutions derived in this study
372 offer an opportunity to relax this constraint, especially by providing asymptotic solutions as a function
373 of known macroscopic quantities such as the impact velocity and the beam curvature at impact. By
374 embedding these solutions into numerical models, it may be possible to bypass the need for excessively
375 small time steps and instead employ more macroscopic timesteps. For instance, an analytical sub-model
376 could describe the reaction force and momentum transfer over the initial phases of contact, allowing the
377 numerical simulation to focus on the subsequent dynamics at a coarser temporal resolution. While this
378 paper does not aim to develop a novel numerical methodology, these perspectives highlight the potential
379 for advancing numerical simulations in impact mechanics.

380 7 Conclusions

381 The impact of a compliant structure against a rigid obstacle is characterized by multiple time and spatial
382 scales, from rapid local deformations and stress wave propagation to slower global motions. These scales
383 pose significant challenges for numerical methods in accurately and efficiently capturing the short-time
384 response near the point of contact. In this paper, we addressed this problem using a beam impacting a
385 rigid obstacle as a minimal yet illustrative example of such multi-scale phenomena.

386 We derived a local asymptotic solution for the short-time response, leveraging the separation of time
387 and spatial scales. At leading order, this analysis reveals two distinct contact patterns, determined by the
388 ratio of the impact velocity to the curvature of the beam at the point of impact. For low impact velocity or
389 high curvature, a single concentrated contact force develops and decreases with the inverse square root of
390 time. For higher velocities or lower curvature, the contact splits into two traveling forces that move apart
391 at a rate inversely proportional to the square root of time. Notably, in the latter case, the beam segment
392 between the two forces remains at rest, presenting unique challenges for numerical methods, particularly
393 those relying on penetration conditions to enforce contact.

394 This work highlights the existence of bifurcation in contact patterns based on simple parameters,
395 offering new insights into the dynamic response of compliant structures during impact. The solution

396 presented in this study can serve as a valuable benchmark for evaluating and improving numerical methods,
397 particularly in scenarios where capturing singular responses and multi-scale phenomena is critical. Future
398 work could explore how these findings might be integrated into computational frameworks to enhance
399 their accuracy and efficiency in handling such complex dynamics.

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446 Appendix A

447 Functions $\varphi_\nu(\lambda)$ and $\varphi_\kappa(\lambda)$ that have been introduced in (53) and (56) admit the following expressions
448 as a function of the Fresnel integrals of the first kind $\mathcal{S}(\cdot)$ and $\mathcal{C}(\cdot)$:

$$\varphi_\nu(\lambda) = \frac{4}{\pi} \int_0^{+\infty} \frac{\mathcal{H}(\lambda\beta)}{(1+\beta^2)^2} d\beta = \cos\left(\frac{\lambda^2}{4}\right) \left[1 - 2\mathcal{S}\left(\frac{\lambda}{\sqrt{2\pi}}\right)\right] - \sin\left(\frac{\lambda^2}{4}\right) \left[1 - 2\mathcal{C}\left(\frac{\lambda}{\sqrt{2\pi}}\right)\right] \quad (59)$$

$$\varphi_\kappa(\lambda) = -\frac{4}{\pi} \int_0^{+\infty} \frac{\mathcal{H}''(\lambda\beta)}{1+\beta^2} d\beta = \cos\left(\frac{\lambda^2}{4}\right) \left[1 - 2\mathcal{C}\left(\frac{\lambda}{\sqrt{2\pi}}\right)\right] + \sin\left(\frac{\lambda^2}{4}\right) \left[1 - 2\mathcal{S}\left(\frac{\lambda}{\sqrt{2\pi}}\right)\right] \quad (60)$$

449 Appendix B

450 In the configuration with two contact forces, the values of r_0 and λ_0 have been determined by enforcing the
451 complementarity conditions at $\xi = 0$, both for η_0 and $\partial_\xi^2 \eta_0$. It is necessary to verify that the ansatz (42)
452 used for the reaction and these values of r_0 and λ_0 provide a solution that satisfies the complementarity
453 condition all along the contact, i.e. $\forall \xi \in [-\lambda_c(\tau), \lambda_c(\tau)]$. In a first step, we verify that the solution is
454 indeed valid for $\xi = \lambda_c(\tau) = \lambda_0 \sqrt{\tau}$ where the fast dynamics displacement reads

$$\eta_0(\lambda_0 \sqrt{\tau}, \tau) = \frac{r_0}{\pi} \int_0^\tau \sqrt{\frac{\tau - \bar{\tau}}{\bar{\tau}}} \left[\mathcal{H}\left(\lambda_0 \frac{\sqrt{\tau} - \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}}\right) + \mathcal{H}\left(\lambda_0 \frac{\sqrt{\tau} + \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}}\right) \right] d\bar{\tau}. \quad (61)$$

455 Considering the changes of variable $\beta_{\pm} = \frac{\sqrt{\tau} \pm \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}}$ for the two terms in the brackets above, respectively, a
 456 similar integrand is obtained,

$$\int_0^{\tau} \sqrt{\frac{\tau - \bar{\tau}}{\bar{\tau}}} \mathcal{H} \left(\lambda_0 \frac{\sqrt{\tau} + \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}} \right) d\bar{\tau} = 16\tau \int_1^{\infty} \frac{\beta_+^2 \mathcal{H}(\lambda_0 \beta_+)}{(1 + \beta_+^2)^3} d\beta_+ \quad (62)$$

$$\int_0^{\tau} \sqrt{\frac{\tau - \bar{\tau}}{\bar{\tau}}} \mathcal{H} \left(\lambda_0 \frac{\sqrt{\tau} - \sqrt{\bar{\tau}}}{\sqrt{\tau - \bar{\tau}}} \right) d\bar{\tau} = 16\tau \int_0^1 \frac{\beta_-^2 \mathcal{H}(\lambda_0 \beta_-)}{(1 + \beta_-^2)^3} d\beta_- \quad (63)$$

457 while one change of variables maps the interval $[0; \tau]$ onto $[0; 1]$ and the other onto $[1; +\infty]$, so that merging
 458 them yields

$$\eta_0(\lambda_0 \sqrt{\tau}, \tau) = \frac{16}{\pi} r_0 \tau \int_0^{\infty} \frac{\beta^2 \mathcal{H}(\lambda_0 \beta)}{(1 + \beta^2)^3} d\beta. \quad (64)$$

459 It is now required to check whether this compensates the slow dynamics solution, expressed with the fast
 460 coordinates, i.e.

$$\nu\tau - \kappa \frac{\xi^2}{2} = \nu\tau - \kappa \frac{\lambda_0^2 \tau}{2} = r_0 \tau \left(\varphi_{\nu}(\lambda_0) - \varphi_{\kappa}(\lambda_0) \frac{\lambda_0^2}{2} \right). \quad (65)$$

461 Introducing the definitions (53) and (56) for φ_{ν} and φ_{κ} , we obtain

$$r_0 \tau \left(\varphi_{\nu}(\lambda_0) - \varphi_{\kappa}(\lambda_0) \frac{\lambda_0^2}{2} \right) = \frac{4}{\pi} r_0 \tau \int_0^{+\infty} \frac{\mathcal{H}(\lambda_0 \beta)}{(1 + \beta^2)^2} - \frac{(1 - 3\beta^2) \mathcal{H}(\lambda_0 \beta)}{(1 + \beta^2)^3} d\beta = \frac{16}{\pi} r_0 \tau \int_0^{+\infty} \frac{\beta^2 \mathcal{H}(\lambda_0 \beta)}{(1 + \beta^2)^3} d\beta. \quad (66)$$

462 Comparison with (64) confirms that the complementarity condition is also satisfied at the ends of the
 463 contact.

464 In a second stage, we prove that the complementarity condition is also satisfied all along the contact,
 465 by considering a generic expression $\xi = \lambda_0 \zeta \sqrt{\tau}$, with $\zeta \in [-1; 1]$. For symmetry reason, only positive
 466 values of ζ will be considered. Using the change of variable $\bar{\tau} = \gamma \tau$, the fast dynamics displacement reads

$$\eta_0(\zeta, \tau) = \frac{r_0 \tau}{\pi} \int_0^1 \sqrt{\frac{1 - \gamma}{\gamma}} \left[\mathcal{H} \left(\lambda_0 \frac{\zeta + \sqrt{\gamma}}{\sqrt{1 - \gamma}} \right) + \mathcal{H} \left(\lambda_0 \frac{\zeta - \sqrt{\gamma}}{\sqrt{1 - \gamma}} \right) \right] d\gamma \quad (67)$$

467 This integral is composed of two terms. The explicit computation of each term is likely not tractable.
 468 Nevertheless conveniently splitting this integral into three parts

$$\eta_0(\zeta, \tau) = I_1(\zeta, \tau) + I_2(\zeta, \tau) + I_3(\zeta, \tau) \quad (68)$$

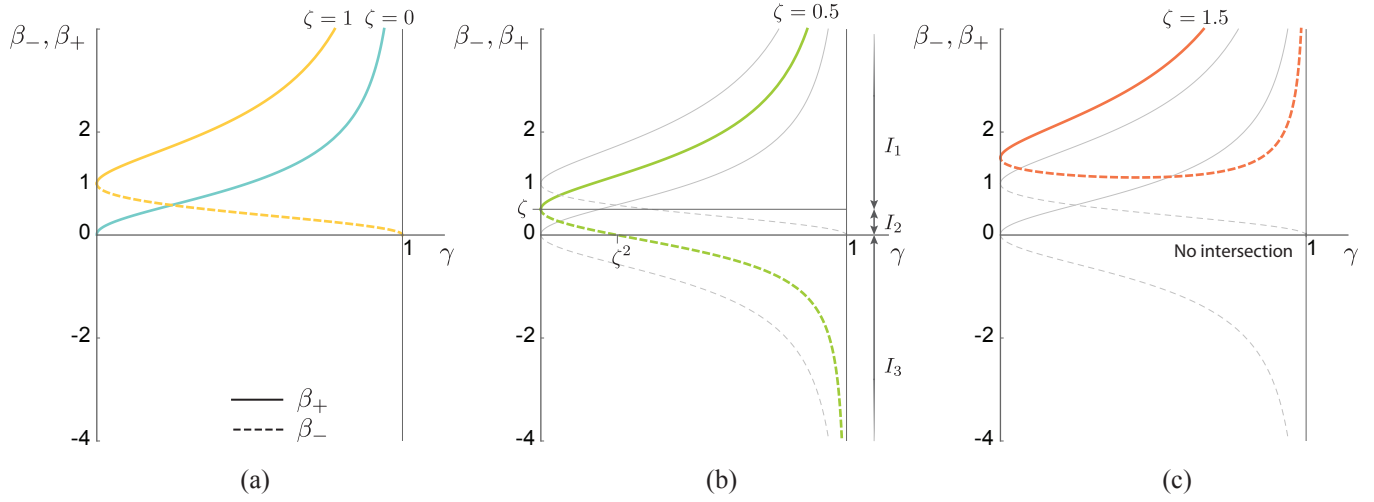


Fig. 7: Illustration of the changes of variable $\beta_-(\gamma)$ and $\beta_+(\gamma)$ used to demonstrate that the complementarity condition holds all along the domain $\mathcal{D}(\tau)$.

469 where

$$\begin{aligned}
 I_1 &= \frac{r_0\tau}{\pi} \int_0^1 \sqrt{\frac{1-\gamma}{\gamma}} \mathcal{H}\left(\lambda_0 \frac{\zeta + \sqrt{\gamma}}{\sqrt{1-\gamma}}\right) d\gamma, \\
 I_2 &= \frac{r_0\tau}{\pi} \int_0^{\zeta^2} \sqrt{\frac{1-\gamma}{\gamma}} \mathcal{H}\left(\lambda_0 \frac{\zeta - \sqrt{\gamma}}{\sqrt{1-\gamma}}\right) d\gamma, \\
 I_3 &= \frac{r_0\tau}{\pi} \int_{\zeta^2}^1 \sqrt{\frac{1-\gamma}{\gamma}} \mathcal{H}\left(\lambda_0 \frac{\zeta - \sqrt{\gamma}}{\sqrt{1-\gamma}}\right) d\gamma,
 \end{aligned} \tag{69}$$

470 allows for some grouping and simplifications that eventually allow the computation of $\eta_0(\zeta, \tau)$, as shown
 471 in the rest of this Appendix.

472 The split of the domain of integration for the last two terms is motivated by the fact that we want
 473 to make use of the changes of variable $\beta_+ = \frac{\zeta + \sqrt{\gamma}}{\sqrt{1-\gamma}}$ and $\beta_- = \frac{\zeta - \sqrt{\gamma}}{\sqrt{1-\gamma}}$ so that the arguments of $\mathcal{H}[\cdot]$ are
 474 all the same in each integral. Figure 7 sketches these expressions of β_+ and β_- as a function of γ (in
 475 solid and dashed lines for each case). For $\zeta = 0$ ($\xi = 0$), the panel (a) shows that only β_+ matters, see
 476 (50). For $\zeta = 1$ ($\xi = \lambda_0\sqrt{\tau}$), the two previously used expressions for β_+ and β_- are shown, (62–63).
 477 When γ spans the range $[0, 1]$, they respectively go from 0 to 1, and 1 to $+\infty$, as seen earlier. In the
 478 more general case shown in panel (b), three cases need to be distinguished. Indeed, it is observed that the
 479 first transformation, β_+ , maps $\gamma \in [0, 1]$ onto $\beta_+ \in [\zeta, +\infty]$ and the second one, β_- , maps $\gamma \in [0, 1]$ onto
 480 $\beta_- \in [-\infty, \zeta]$. Since $\zeta > 0$, the later interval includes $\beta_- = 0$ where the domain of integration is split in
 481 order to explicitly obtain a domain running from 0 to $+\infty$, i.e. $\beta \in [0, \zeta] \cup [\zeta, +\infty]$. The pivotal value
 482 $\beta_- = 0$ corresponds to $\gamma = \zeta^2$, which explains the chosen decomposition.

483 The first integral is analyzed with a change of variable $\sqrt{\gamma} = f_+(\beta)$, such that $\beta = \frac{\zeta + \sqrt{\gamma}}{\sqrt{1-\gamma}}$, i.e.
 484 $\frac{\zeta + f_+}{\sqrt{1-f_+^2}} = \beta$. Inverting the mapping in these conditions (with $f_+(\beta)$ required to be positive) yields

$$f_+(\beta) = \frac{-\zeta + \beta\sqrt{\Delta}}{\beta^2 + 1} \quad (70)$$

485 with $\Delta = 1 - \zeta^2 + \beta^2$. It maps $\gamma \in [0, 1]$ onto $\beta \in [\zeta, +\infty]$, as required, with positive Jacobian. It is
 486 observed that $f_+(\beta) \geq 0$ when $\zeta \geq 0$ for $\beta \in [\zeta, +\infty]$.

487 Similarly, integrals I_2 and I_3 are solved with a change of variable $\sqrt{\gamma} = f_-(\beta)$, such that $\beta = \frac{\zeta - \sqrt{\gamma}}{\sqrt{1-\gamma}}$,
 488 i.e. $\lambda_0 \frac{\zeta - f_-}{\sqrt{1-f_-^2}} = \beta$. The expression of $f_-(\beta)$ is now different for β positive and negative.

$$f_-(\beta) = \begin{cases} \frac{\zeta + \beta\sqrt{\Delta}}{\beta^2 + 1} & \text{if } \beta \leq 0 \\ \frac{\zeta - \beta\sqrt{\Delta}}{\beta^2 + 1} & \text{if } \beta \geq 0 \end{cases} \quad (71)$$

489 These two transformations respectively map $\gamma \in [0, \zeta^2]$ onto $\beta \in [0, \zeta]$ and $\gamma \in [\zeta^2, 1]$ onto $\beta \in [-\infty, 0]$,
 490 both with negative Jacobian. Using notation f momentarily to represent either f_+ or f_- , the Jacobian
 491 can be expressed as

$$\frac{d\gamma}{2\sqrt{\gamma}} = f'(\beta) d\beta. \quad (72)$$

492 Completion of the change of variable will require expressing $\sqrt{1-\gamma}$ for each integral, which can
 493 be expressed as a function of β . For the first integral, it is noticed that $\sqrt{1-\gamma} = (\zeta + f_+)/\beta$, while
 494 $\sqrt{1-\gamma} = (\zeta - f_+)/\beta$ for the other two. Therefore, using (70)-(71) yields

$$I_1 = \frac{2r_0\tau}{\pi} \int_{\zeta}^{+\infty} \frac{(\zeta + f_+) f'_+}{\beta} \mathcal{H}(\lambda_0\beta) d\beta = \frac{2r_0\tau}{\pi} \int_{\zeta}^{+\infty} \frac{(\beta\zeta + \sqrt{\Delta}) (2\beta\zeta\sqrt{\Delta} + (\beta^2 - 1)\zeta^2 + \beta^2 + 1)}{(\beta^2 + 1)^3 \sqrt{\Delta}} \mathcal{H}(\lambda_0\beta) d\beta \quad (73)$$

$$I_2 = \frac{2r_0\tau}{\pi} \int_0^{\zeta} \frac{(\zeta - f_-) f'_-}{\beta} \mathcal{H}(\lambda_0\beta) d\beta = \frac{2r_0\tau}{\pi} \int_0^{\zeta} \frac{(\beta\zeta + \sqrt{\Delta}) (2\beta\zeta\sqrt{\Delta} + (\beta^2 - 1)\zeta^2 + \beta^2 + 1)}{(\beta^2 + 1)^3 \sqrt{\Delta}} \mathcal{H}(\lambda_0\beta) d\beta \quad (74)$$

$$I_3 = \frac{2r_0\tau}{\pi} \int_{-\infty}^0 \frac{(\zeta - f_-) f'_-}{\beta} \mathcal{H}(\lambda_0\beta) d\beta = \frac{2r_0\tau}{\pi} \int_{-\infty}^0 \frac{(\beta\zeta - \sqrt{\Delta}) (2\beta\zeta\sqrt{\Delta} - (\beta^2 - 1)\zeta^2 - \beta^2 - 1)}{(\beta^2 + 1)^3 \sqrt{\Delta}} \mathcal{H}(\lambda_0\beta) d\beta \quad (75)$$

495 Interestingly, the integrands in I_1 and I_2 are now the same. These two integrals combine into a single
 496 one, on the domain $[0; +\infty]$. Furthermore, integral I_3 can be evaluated on the positive support using the
 497 change of variable $\beta \rightarrow -\beta$ and the symmetry property $\mathcal{H}(\lambda_0\beta) = \mathcal{H}(-\lambda_0\beta)$, so that

$$\eta_0(\zeta, \tau) = I_1 + I_2 + I_3 = \frac{4r_0\tau}{\pi} \int_0^{+\infty} \frac{(3\beta^2 - 1)\zeta^2 + \beta^2 + 1}{(\beta^2 + 1)^3} \mathcal{H}(\lambda_0\beta) d\beta \quad (76)$$

498 after putting together (73)-(75), and some simplifications. After comparison with the definitions of φ_ν
 499 and φ_κ , it appears that this latter expression coincides with the macroscopic solution, $\nu\tau - \kappa\frac{\xi^2}{2}$, written
 500 with $\xi = \lambda_0\zeta\sqrt{\tau}$, i.e.

$$\nu\tau - \kappa\frac{(\lambda_0\zeta\sqrt{\tau})^2}{2} = \left(\nu - \kappa\frac{\lambda_0^2\zeta^2}{2}\right)\tau = r_0\tau\left(\varphi_\nu(\lambda_0) - \varphi_\kappa(\lambda_0)\frac{\lambda_0^2\zeta^2}{2}\right). \quad (77)$$

501 The complementarity condition is therefore satisfied all along the contact domain, for $\zeta \geq 0$. The demon-
 502 stration for $\zeta \leq 0$ is justified by symmetry or obtained following similar arguments.

503 Finally, Figure 7-c shows that the decomposition of the integral into three terms, as considered in this
 504 Appendix does not hold as soon as $\zeta > 1$, as β_- then does not cross the γ -axis any longer. Therefore,
 505 the complementary condition is only verified along the contact domain \mathcal{D} . Despite the fact that a unique
 506 expression $\eta_0(\xi, \tau)$ is used for the deformed configuration everywhere along the beam, it matches the
 507 quadratic formulation along $\mathcal{D}(\tau)$ only, not elsewhere.