

The influence of beam kinematic assumptions in a beam contact benchmark

A. Bosten^{1,2}, V. Denoël³, A. Cosimo⁴, J. Linn² and O. Brüs¹

¹Department of Aerospace and Mechanical Engineering, University of Liège

²Fraunhofer Institute for Industrial Mathematics, Kaiserslautern

³Department of Urban and Environmental Engineering, University of Liège

⁴Siemens Industry Software, Liège

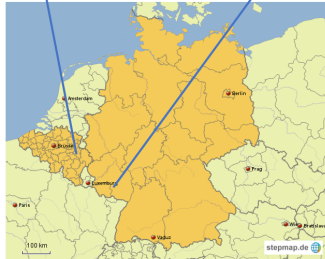


ECCM 2022, Oslo



Context and applications

ULiège & ITWM Kaiserslautern

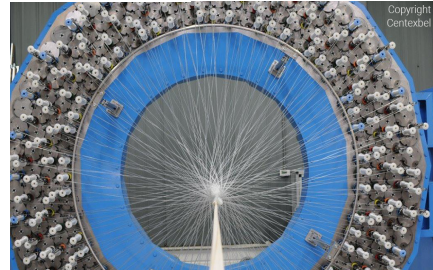


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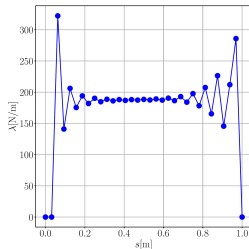
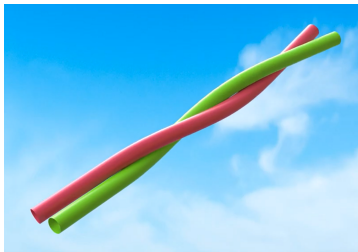


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Motivation for a beam contact benchmark

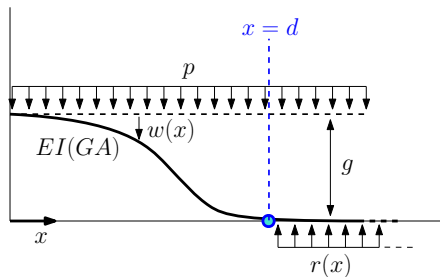


Line contact model between geometrically exact beams (mortar approach)

- Is the observed distributed contact force reasonable?
- What is the influence of the **beam kinematic assumption**? (with or without shear deformation)
- **Benchmarking** with other methods? (point-to-point contact model, penalty formulation)
- Representative test problem with an **analytical solution**?

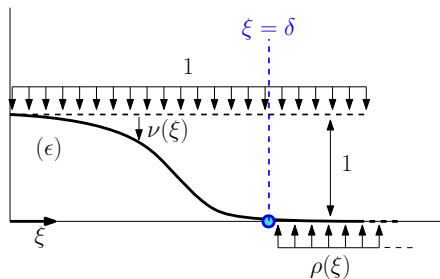
 A. Bosten, A. Cosimo, J. Linn and O. Bröls, A mortar formulation for frictionless line-to-line beam contact, *Multibody System Dynamics*, 54:31-52 (2021)

Proposed test problem



- 2D cantilever beam with infinite length
- Small displacements \Rightarrow linear beam model
- Euler-Bernoulli vs Timoshenko model
- g : initial gap w.r.t. rigid support
- p : uniformly applied lineic force
- Cross-section deformations are neglected in the contact kinematics

Adimensionalization



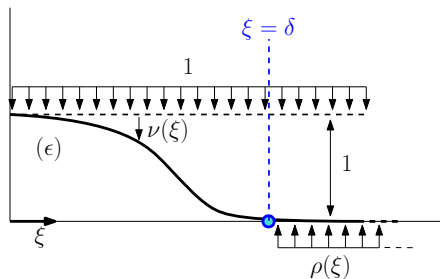
Characteristic lengths and angle :

$$u_1^* = g, u_2^* = \left(\frac{gEI}{p} \right)^{1/4}, \theta^* = \frac{u_1^*}{u_2^*}$$

Change of variables:

$$\xi = \frac{x}{u_2^*}, \nu = \frac{w}{u_1^*}, \vartheta = \frac{\theta}{\theta^*}, \delta = \frac{d}{u_2^*}, \rho = \frac{r}{p}, \epsilon^2 = \frac{EI/GA}{u_2^{*2}}$$

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Equilibrium and kinematic equations

Euler-Bernoulli and Timoshenko models

$$\begin{aligned}\theta'''(x) - \frac{p - r(x)}{EI} &= 0 \\ \frac{EI}{GA}\theta''(x) - \theta(x) + w'(x) &= 0\end{aligned}$$

$$\begin{aligned}\vartheta'''(\xi) + \rho(\xi) - 1 &= 0 \\ \epsilon^2\vartheta''(\xi) - \vartheta(\xi) + \nu'(\xi) &= 0\end{aligned}$$

- In the free region, the reaction is zero: $r(x) = 0$ or $\rho(\xi) = 0$
- In the contact region, the displacement is imposed: $w(x) = g$ or $\nu(\xi) = 1$, $w'(x) = 0$ or $\nu'(\xi) = 0$
- At the transition point $x = d$ or $\xi = \delta$, these equations do not necessarily hold

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Boundary conditions

Clamp:

$$w(0) = 0, \quad \theta(0) = 0$$

Free end:

$$\theta(x) \rightarrow 0 \quad \text{as} \quad x \rightarrow +\infty$$

Transition point:

$$w(d) = g, \quad w'(d) = 0, \quad \theta(d) = \theta_d$$

Free boundary problem:

- the governing equations are solved analytically on each subdomain that are defined by the limits of the contact region
- Continuity equations are used to localize this transition point

Euler-Bernoulli solution

The dimensionless free length is

$$\delta = 72^{1/4}$$

The dimensionless shear force is

$$\mathcal{T}(\xi) = \begin{cases} \xi - \delta \left(\frac{1}{2} + \frac{12}{\delta^4} \right) & \text{if } \xi < \delta \\ 0 & \text{if } \xi > \delta \end{cases}$$

A **point contact force** appears due to discontinuity in the shear force

$$f_\delta = \mathcal{T}(\delta^-) - \mathcal{T}(\delta^+) = \frac{1}{3}\delta$$

Timoshenko solution

The dimensionless free length is obtained by solving the following equation

$$\delta^6 + 6\epsilon\delta^5 + 30\epsilon^2\delta^4 + 72\epsilon^3\delta^3 + 72(\epsilon^4 - 1)\delta^2 - 144\epsilon(\delta + \epsilon) = 0.$$

The shear force is given by

$$\mathcal{T}(\xi) = \begin{cases} \xi - \delta \left(\frac{1}{2} + \frac{12}{\delta^4} \right) \frac{\delta^2}{\delta^2 + 6\epsilon^2} & \text{if } \xi < \delta \\ \frac{\vartheta_\delta}{\epsilon^2} \exp\{(\delta - \xi)/\epsilon\} & \text{if } \xi > \delta \end{cases}$$

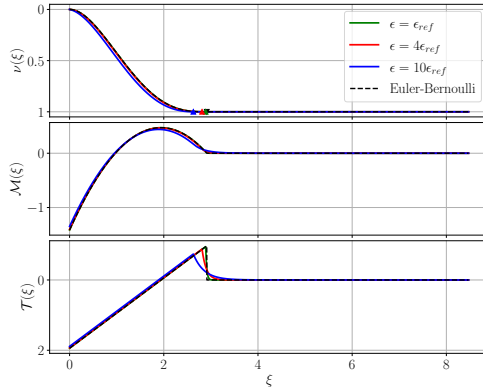
where

$$\vartheta_\delta = \delta\epsilon^2 \left(\frac{1}{2} + 6\frac{\epsilon^2}{\delta^2} - \frac{12}{\delta^4} \right) \frac{\delta^2}{\delta^2 + 6\epsilon^2},$$

such that

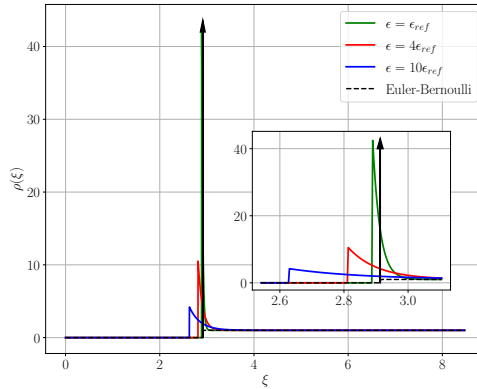
$$\boxed{\mathcal{T}(\delta^-) = \mathcal{T}(\delta^+)}$$

Analytical solutions



Transverse displacement, bending moment, shear force. The markers denote the first contact point:
 $\epsilon_{ref} = 0.023$.

Analytical solutions



Distributed contact force: $\epsilon_{ref} = 0.023$.

Properties of the analytical solution

Timoshenko model (with shear deformation)

- the shear force is continuous
- the distributed contact force is discontinuous & takes large values in a boundary layer of length ϵ

Euler-Bernoulli model (no shear deformation)

- the shear force is discontinuous
- the distributed contact force is discontinuous & has an atom (Dirac delta)

⇒ Adding **kinematic restrictions** on the deformation pattern increases the **level of nonsmoothness**

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Timoshenko model (with shear deformation)

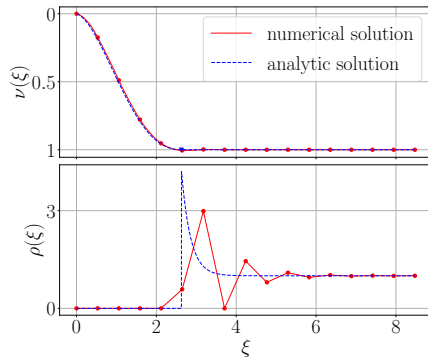
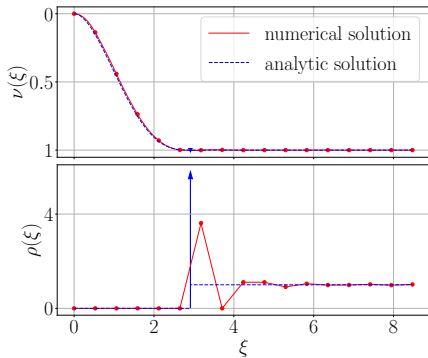
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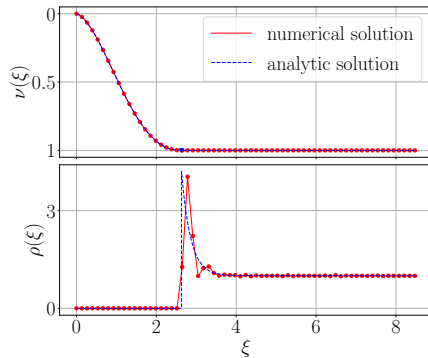
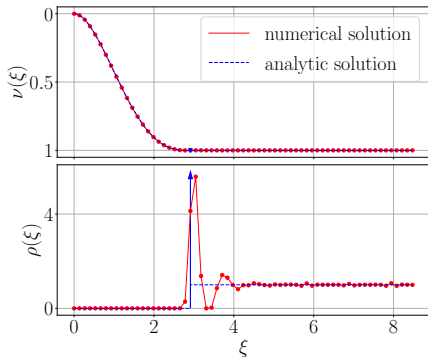
⇒ Adding **kinematic restrictions** on the deformation pattern increases the **level of nonsmoothness**

Comparison with a mortar numerical solution



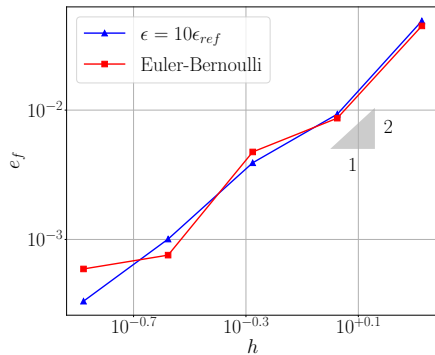
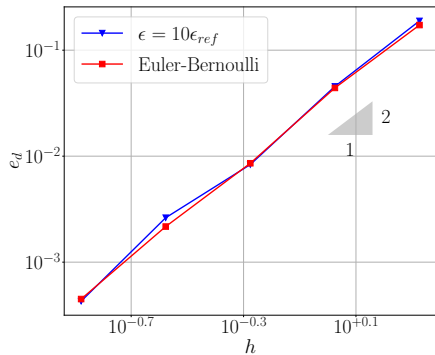
Left: Euler-Bernoulli. Right: Timoshenko with $\epsilon = 10\epsilon_{ref}$ and $h = 0.53 > \epsilon$.

Comparison with a mortar numerical solution



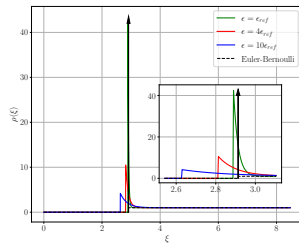
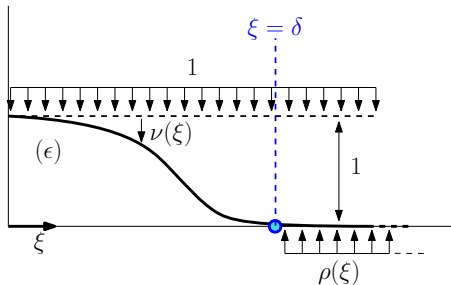
Left: Euler-Bernoulli. Right: Timoshenko with $\epsilon = 10\epsilon_{ref}$ and $h = 0.13 < \epsilon$.

Comparison with a mortar numerical solution



Left: Error on the vertical displacement. Right: Error on the integrated contact force.

Conclusion



A **benchmark for beam contact problems** has been proposed and an **analytical solution** has been presented.

The analytical solution reveals the **significant influence of the kinematic assumptions of the beam model** on the solution **at the contact transition**.

Compared to the Euler-Bernoulli model, the **Timoshenko model** leads to **smoother solutions** which are easier to capture by numerical schemes.

Thank you for your attention!

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