



From constitutive modeling to THM couplings in geomaterials

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1. Context

Long-term management of radioactive wastes



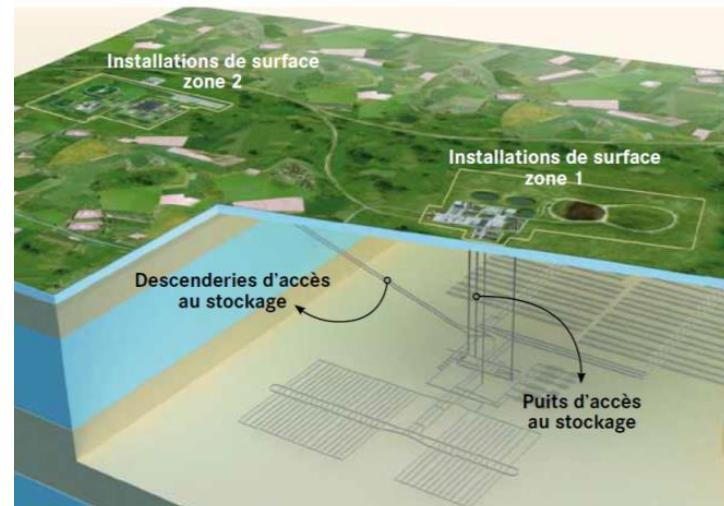
Intermediate
(long-lived)
&
high activity
wastes



Deep geological disposal

Repository in deep
geological media with
good confining properties
(Low permeability
 $K < 10^{-12} \text{ m/s}$)

Underground structures
= network of galleries

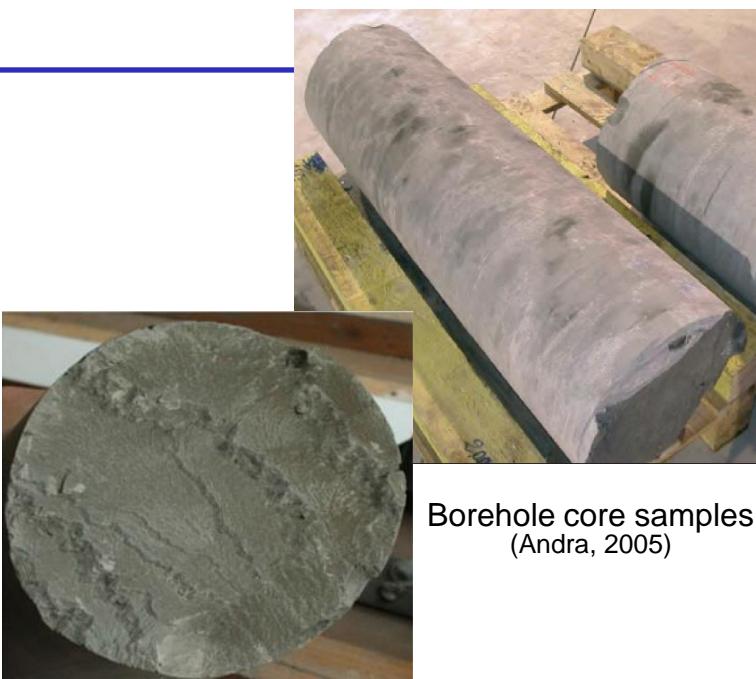
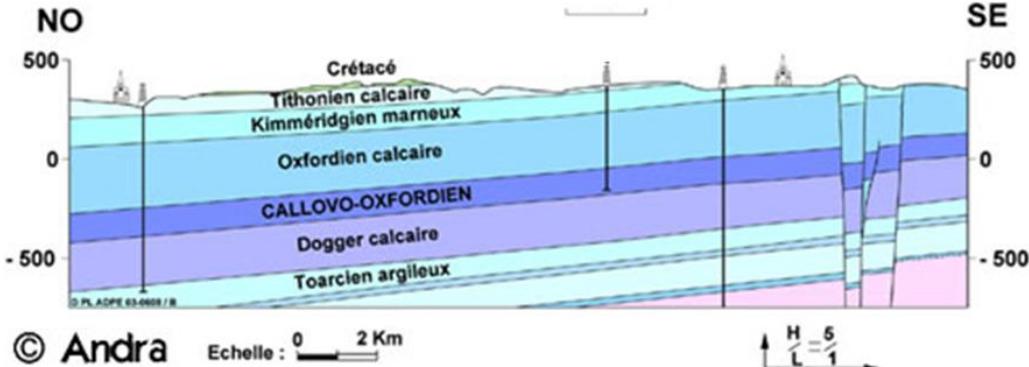


Disposal facility of Cigéo project in France
(Labalette et al., 2013)

1. Context

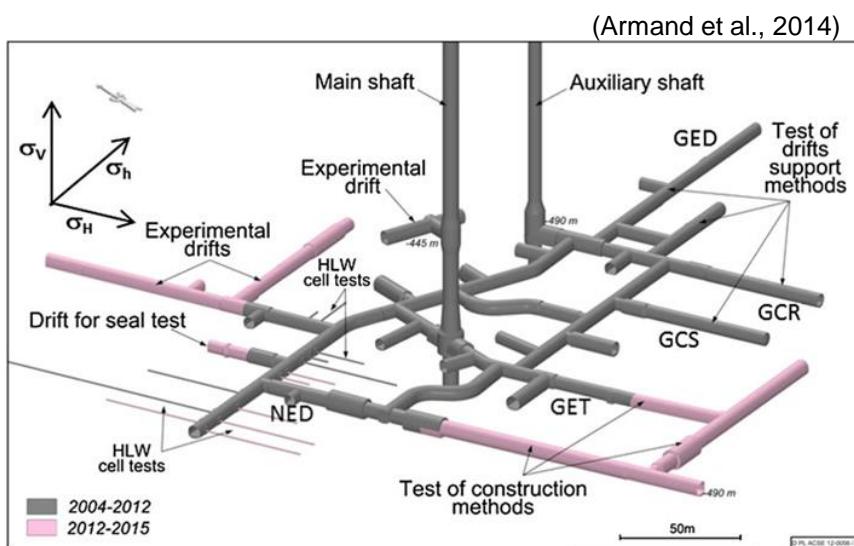
Callovo-Oxfordian claystone (COx)

Sedimentary clay rock (France).



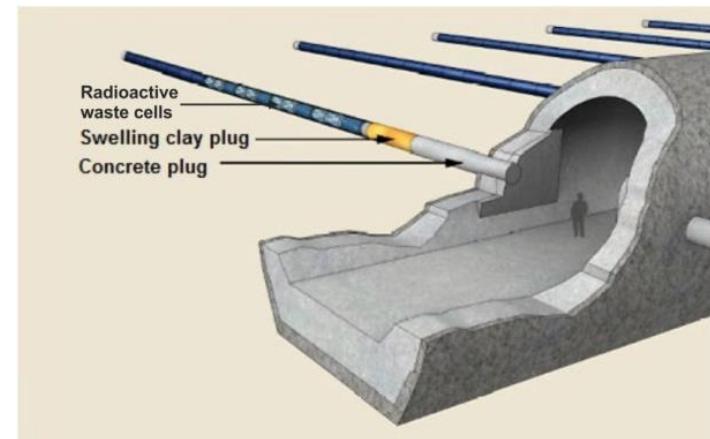
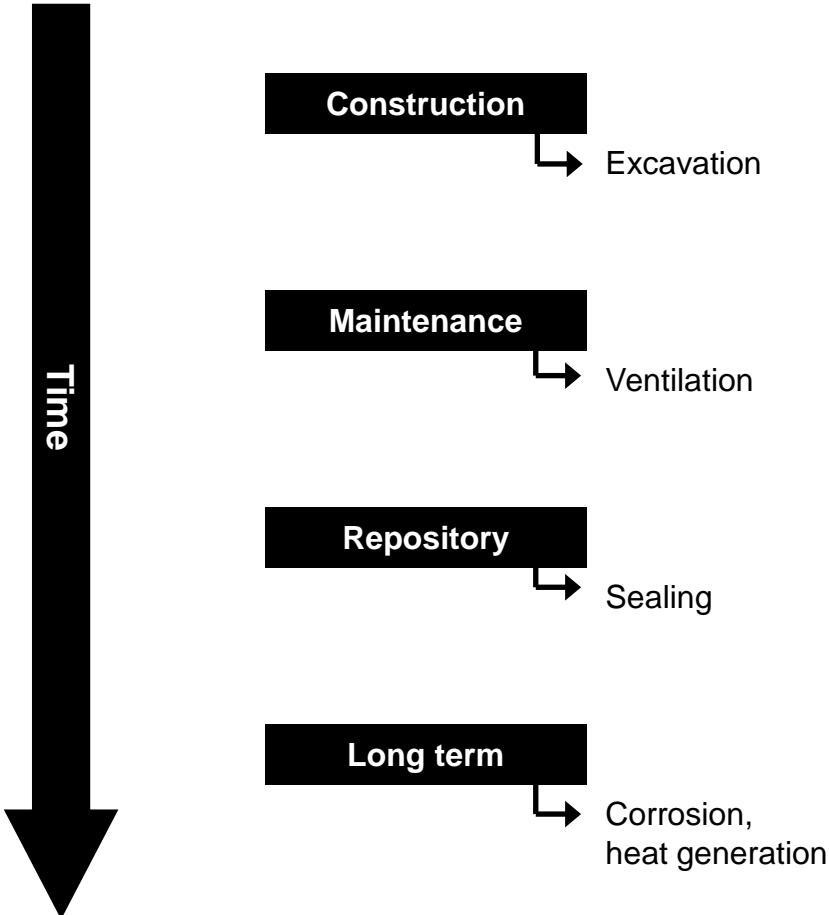
Borehole core samples
(Andra, 2005)

- Underground research laboratory
- Feasibility of a safe repository
- France (Meuse / Haute-Marne, Bure)



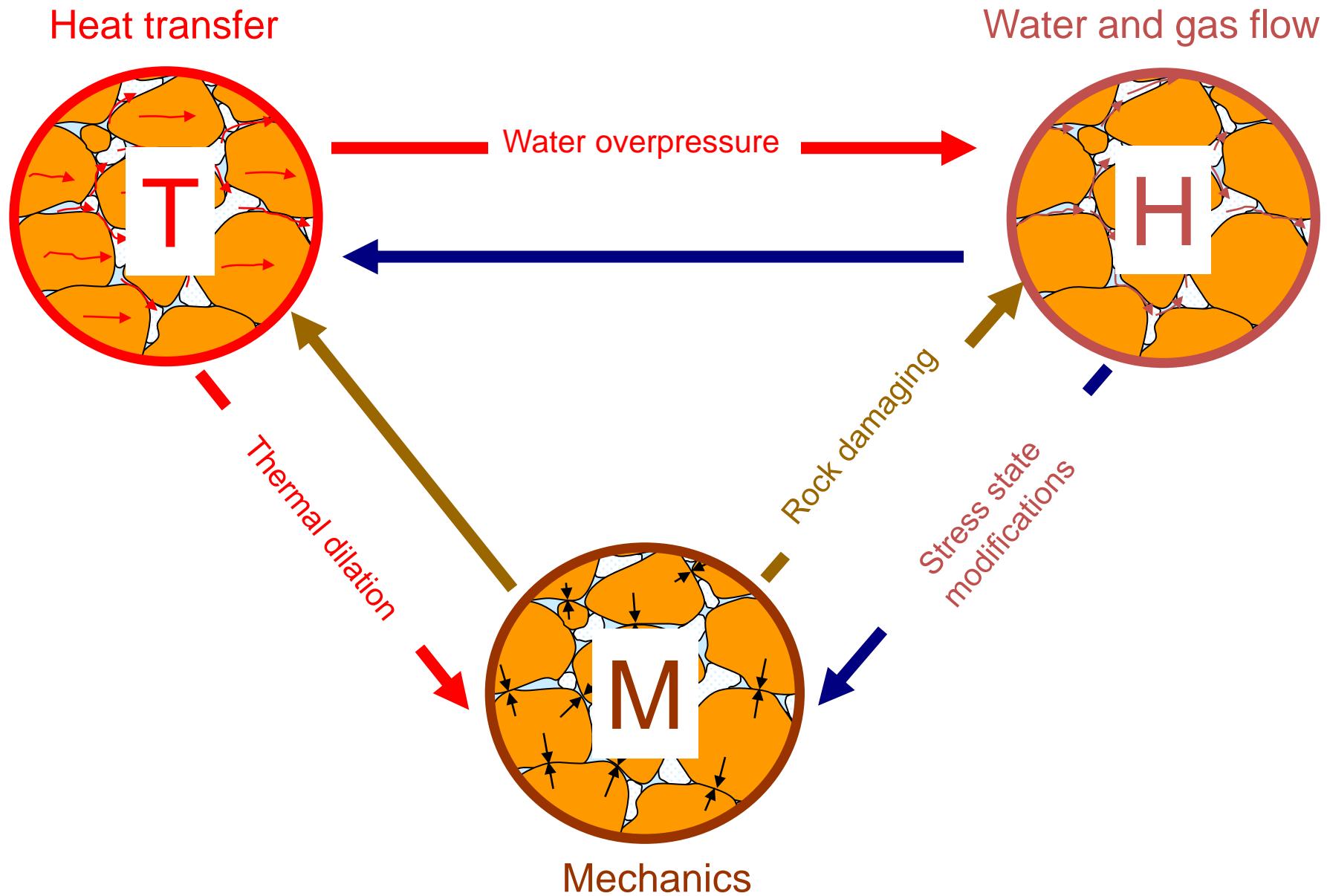
1. Context

Repository phases



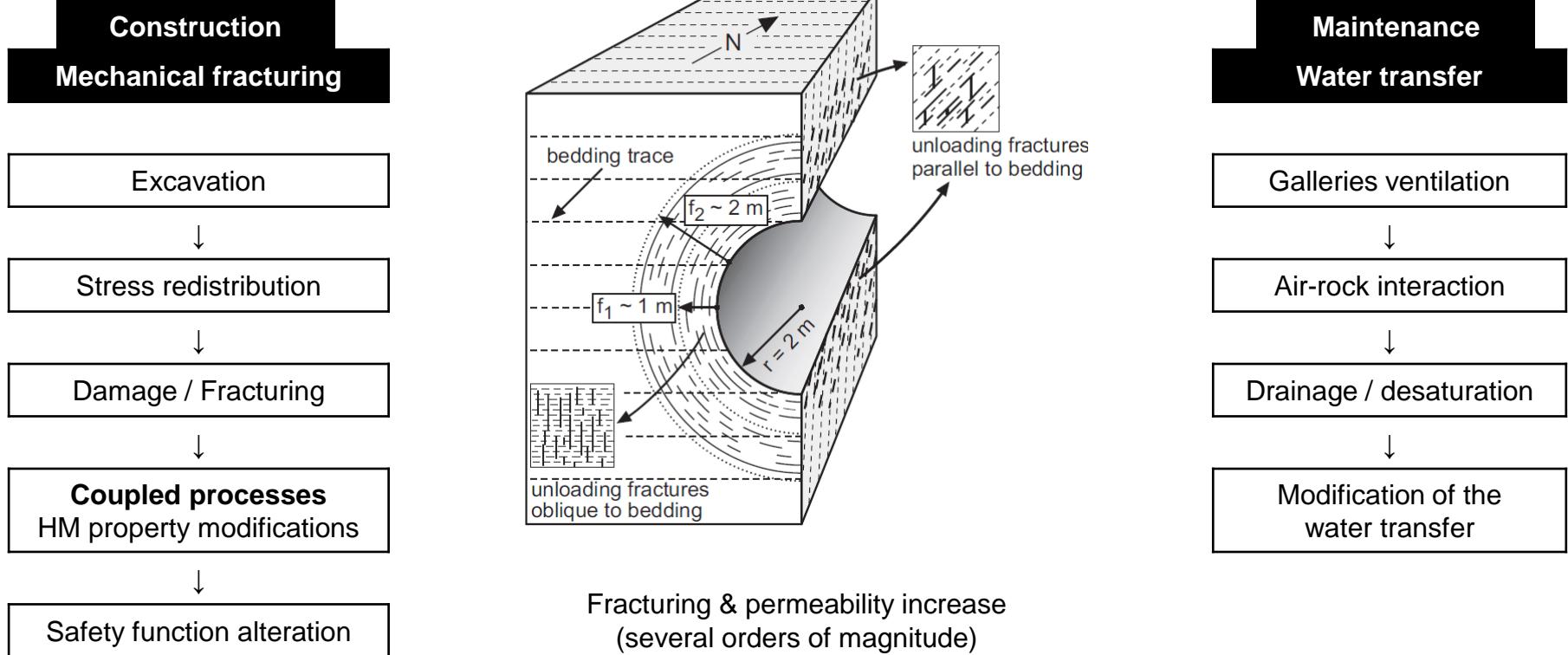
Type C wastes (Andra, 2005)

1. Context



1. Context

Excavation Damaged Zone (EDZ)

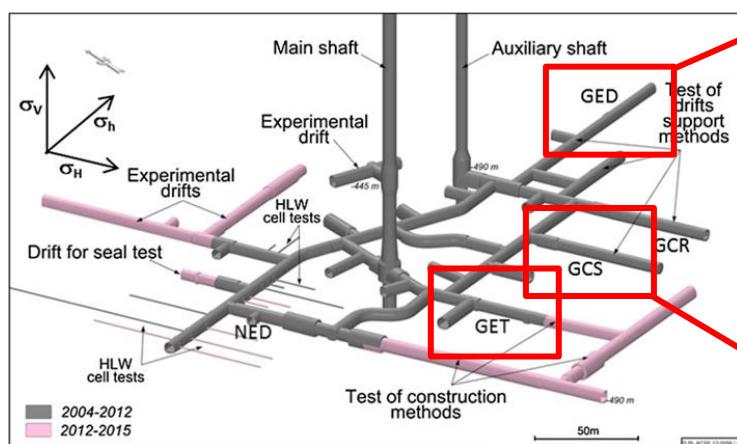


1. Context

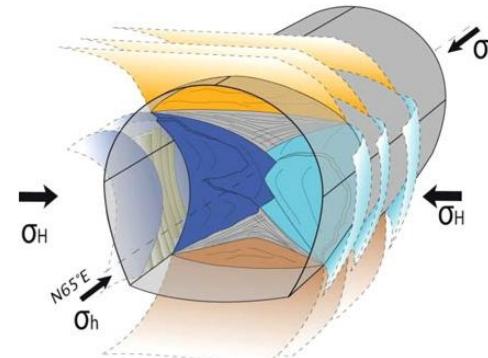
- Fracturing

Anisotropies: - stress : $\sigma_H > \sigma_h \sim \sigma_v$
 - material : HM cross-anisotropy.

(Armand et al., 2014)

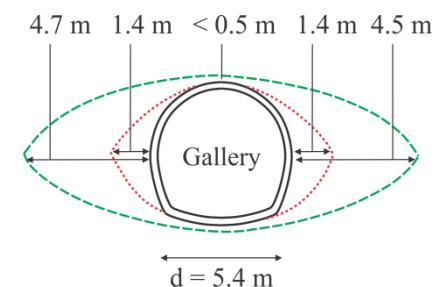
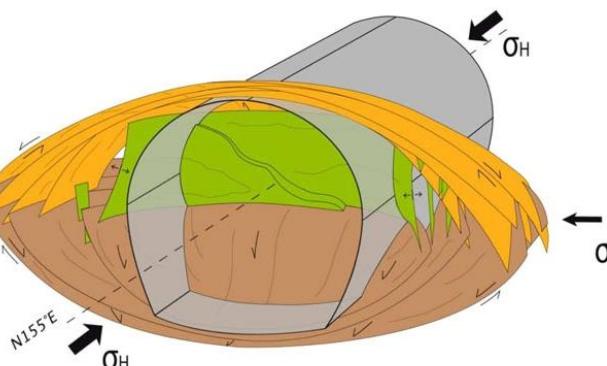


Galery // to σ_h



Shear fractures
Mixed fractures

Galery // to σ_H



Issues:

- Prediction of the fracturing.
- Effect of anisotropies ?
- Permeability evolution & relation to fractures ?

1. Context
2. **Fracture modelling with shearbands**
3. Influence of mechanical anisotropy
4. Permeability evolution and water transfer
5. Creep deformation
6. THM couplings
7. Conclusions and perspectives

2. Fracture modelling with shear bands

2.2. Constitutive models for COx

- Mechanical law - 1st gradient model

Isotropic elasto-plastic internal friction model

Non-associated plasticity, Van Eeckelen yield surface :

$$F \equiv II_{\hat{\sigma}} - m \left(I_{\sigma'} + \frac{3c}{\tan \varphi_c} \right) = 0$$

φ hardening / c softening

$$c = c_0 + \frac{(c_f - c_0) \hat{\varepsilon}_{eq}^p}{B_c + \hat{\varepsilon}_{eq}^p}$$

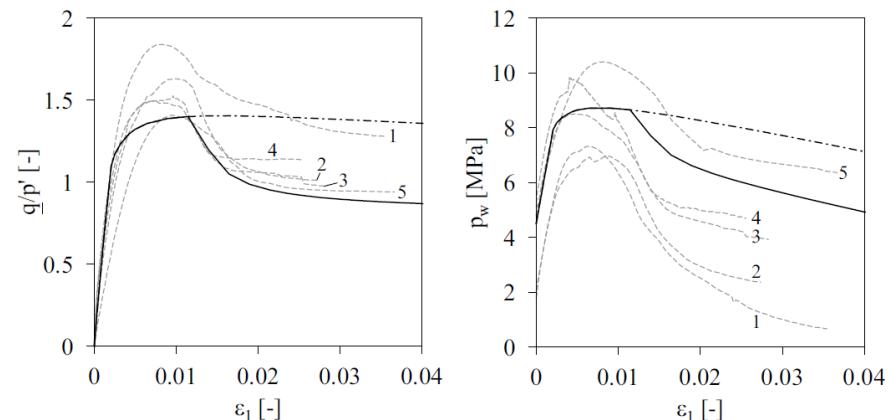
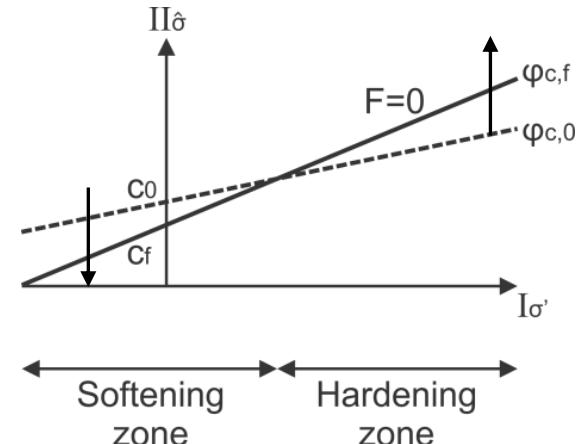
→ Strain localisation

- Hydraulic law

Fluid mass flow (advection, Darcy) :

$$f_{w,i} = -\rho_w \frac{k_{w,ij} k_{r,w}}{\mu_w} \left(\frac{\partial p_w}{\partial x_j} + \rho_w g_j \right)$$

Water retention and permeability curves (Mualem - Van Genuchten's model)



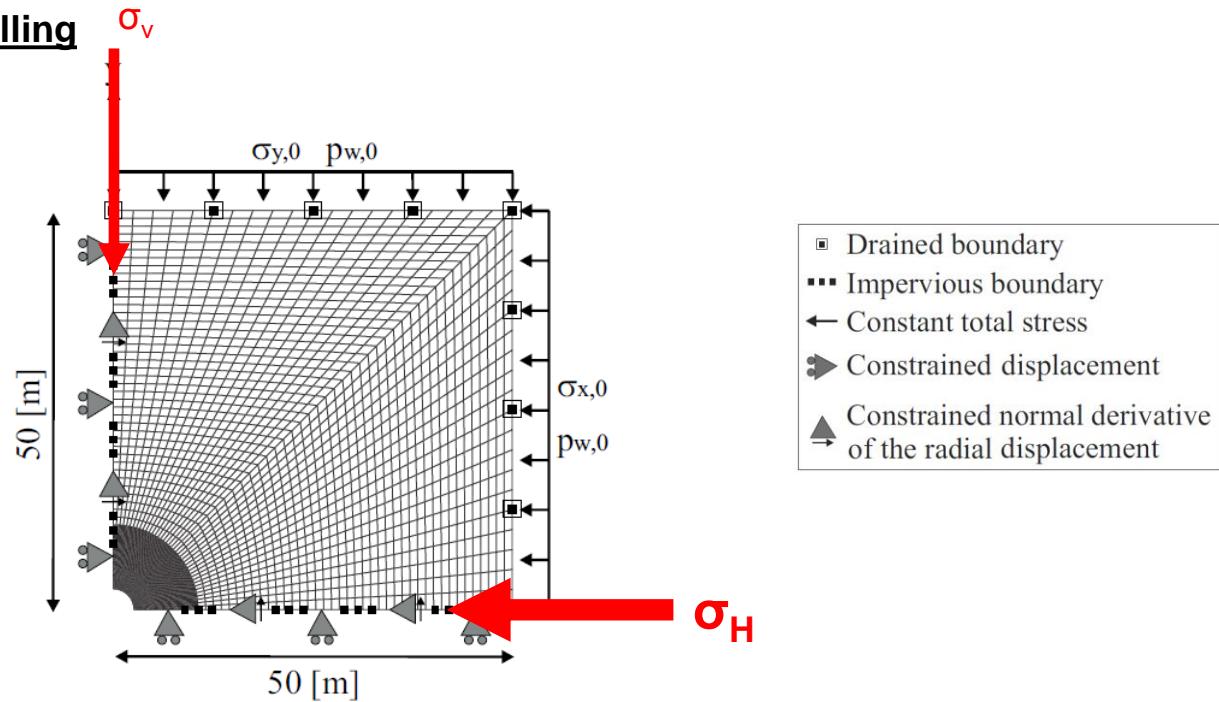
2. Fracture modelling with shear bands

2.3. Gallery excavation modelling

- Numerical model

HM modelling in 2D plane strain state

Gallery radius = 2.3 m



- Gallery in $\text{CO}_x // \sigma_h$

Effect of stress anisotropy

Anisotropic stress state

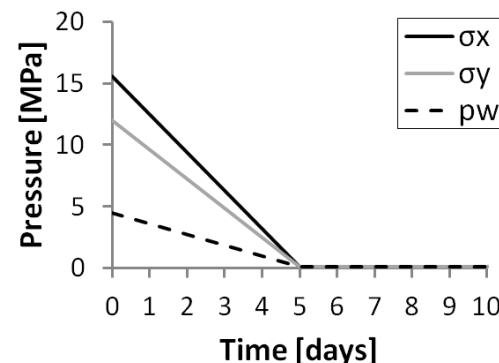
$$p_{w,0} = 4.5 \text{ [MPa]}$$

$$\sigma_{x,0} = \sigma_h = 1.3 \quad \sigma_v = 15.6 \text{ [MPa]}$$

$$\sigma_{y,0} = \sigma_v = 12 \text{ [MPa]}$$

$$\sigma_{z,0} = \sigma_h = 12 \text{ [MPa]}$$

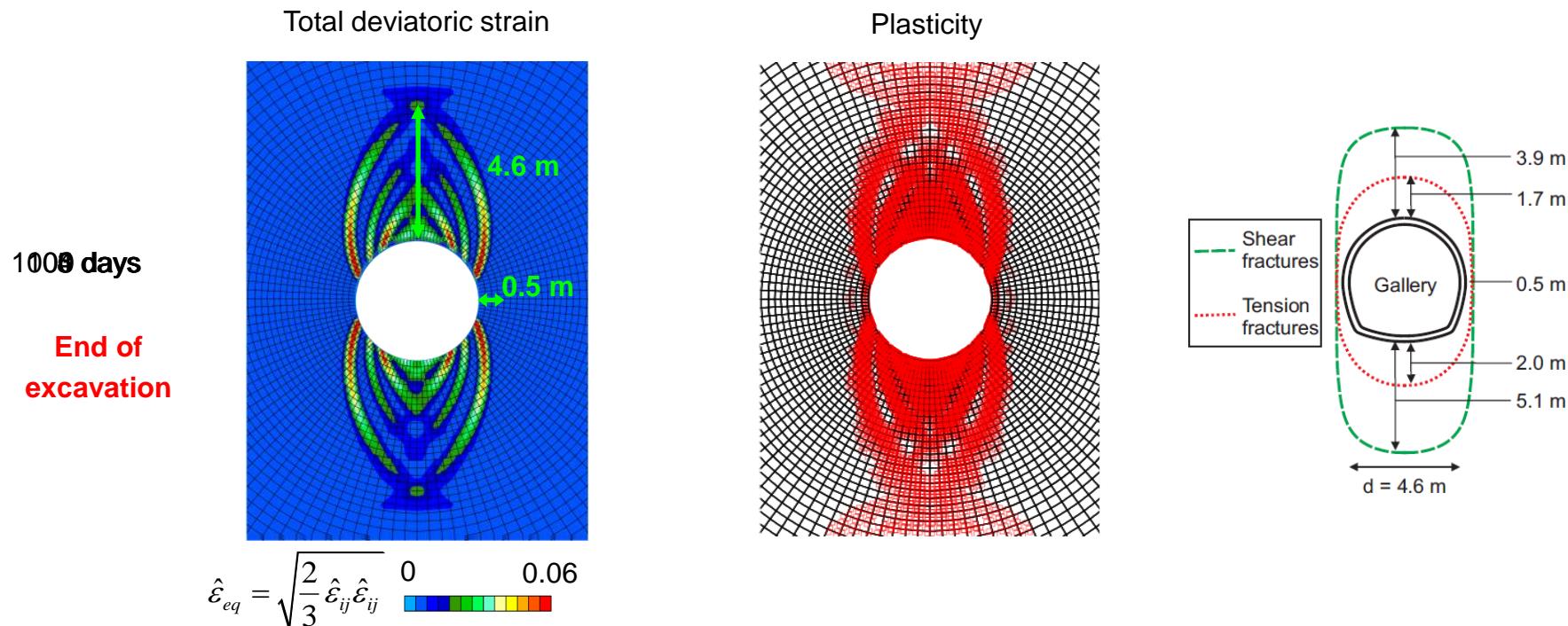
- Excavation



2. Fracture modelling with shear bands

- Localisation zone

Incompressible solid grains, $b=1$



→ For an isotropic mechanical behaviour, the appearance and shape of the strain localisation are mainly due to mechanical effects linked to the anisotropic stress state.

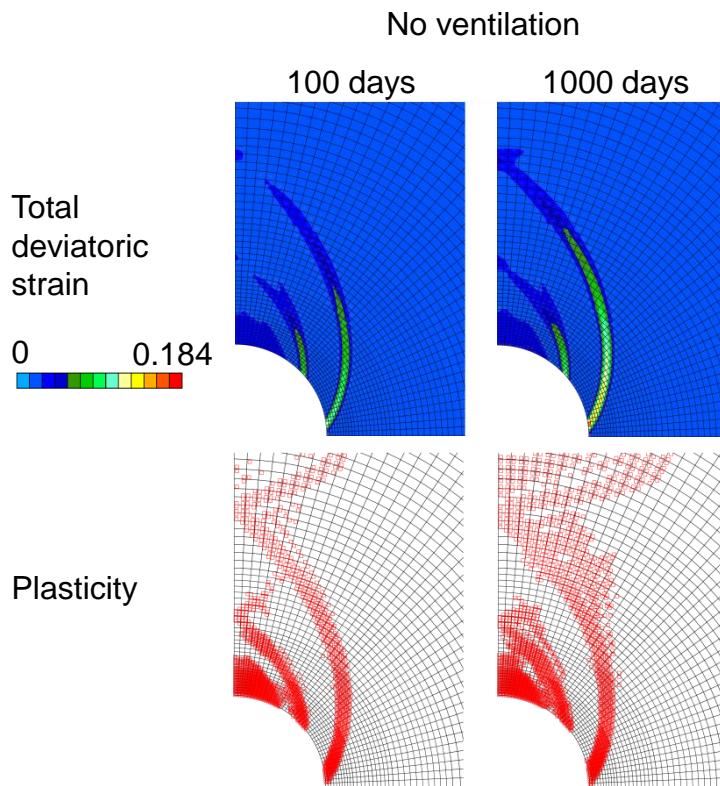
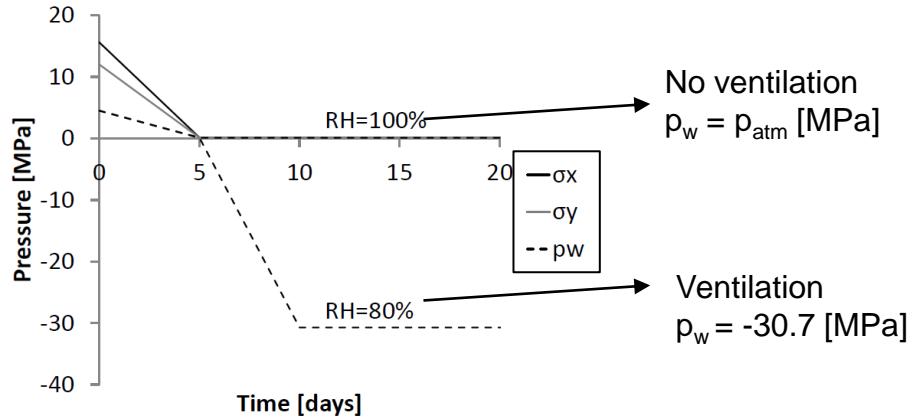
2. Fracture modelling with shear bands

- Gallery air ventilation :

Water phases equilibrium at gallery wall (Kelvin's law)

$$RH = \frac{p_v}{p_{v,0}} = \exp\left(\frac{-p_c M_v}{RT \rho_w}\right)$$

Compressibility of the solid grains: $b=0.6$



2. Fracture modelling with shear bands

- Convergence:

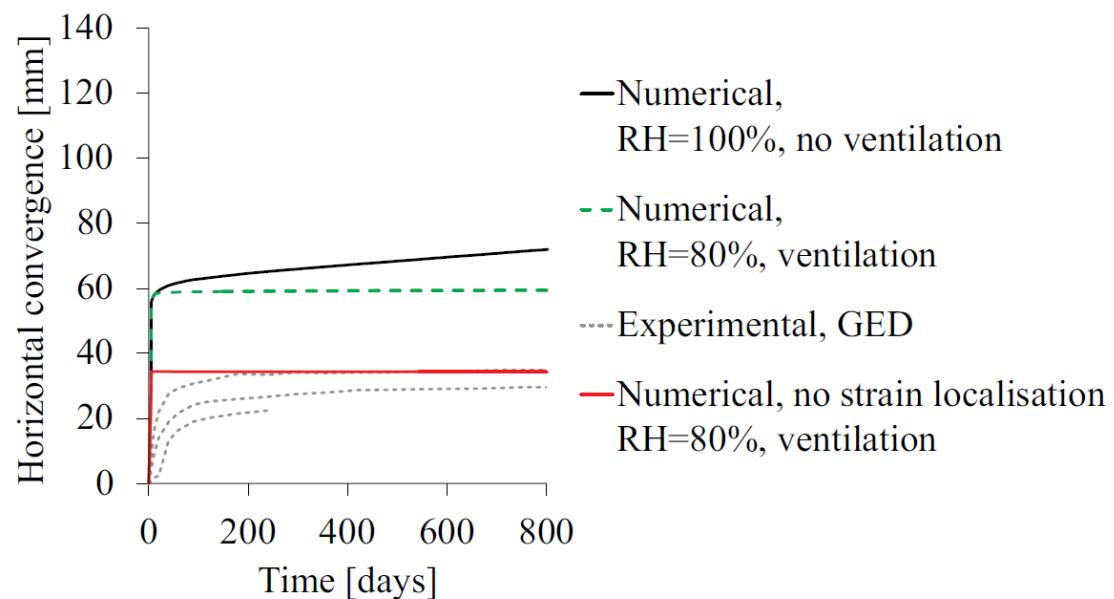
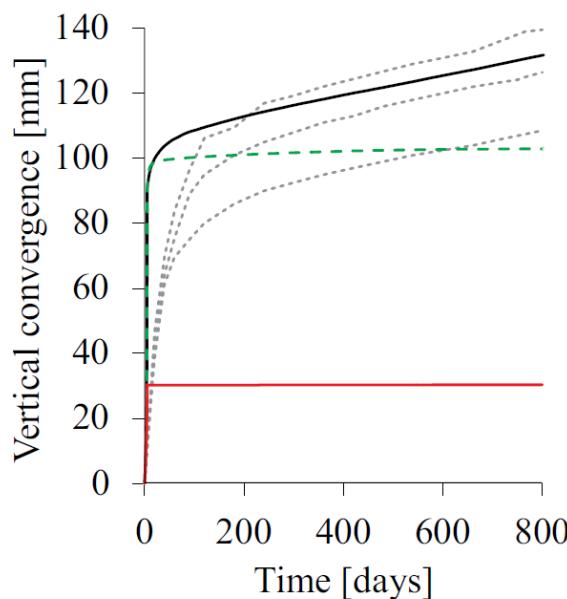
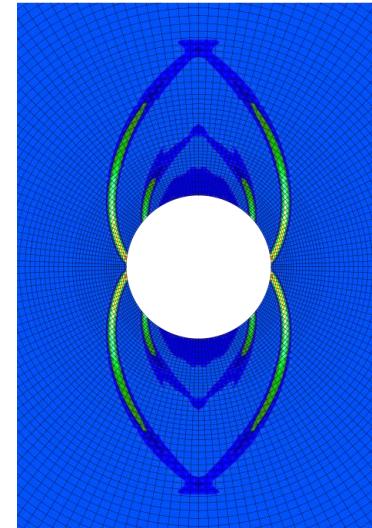
Important during the excavation

Anisotropic convergence

Influence of the ventilation

Experimental results (GED - Andra's URL)

No strain localisation

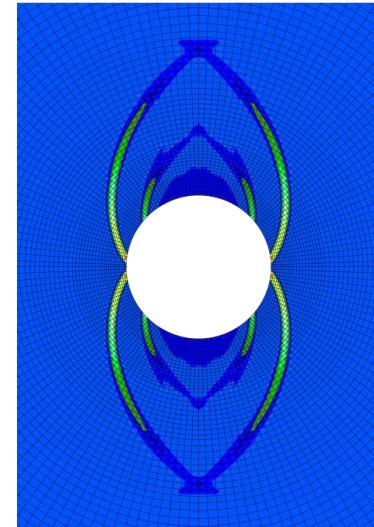


2. Fracture modelling with shear bands

2.5. Conclusions and outlooks

- Conclusions

- ✓ Reproduction of EDZ with shear bands.
- ✓ Shape and extent of EDZ **governed by anisotropic stress state.**



- Next steps ...

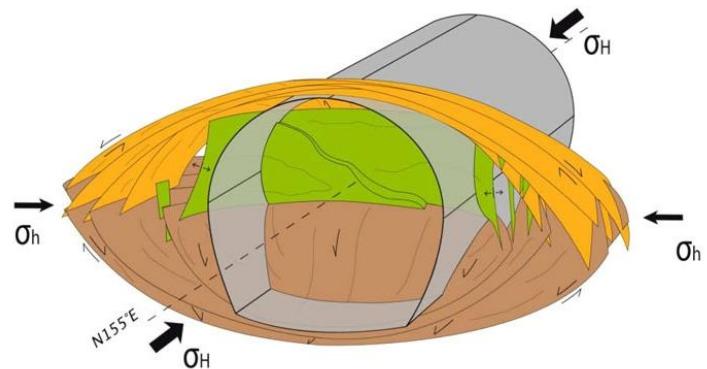
X Mechanical rock behaviour.

→ Material anisotropy, gallery // σ_H .

X HM coupling in EDZ.

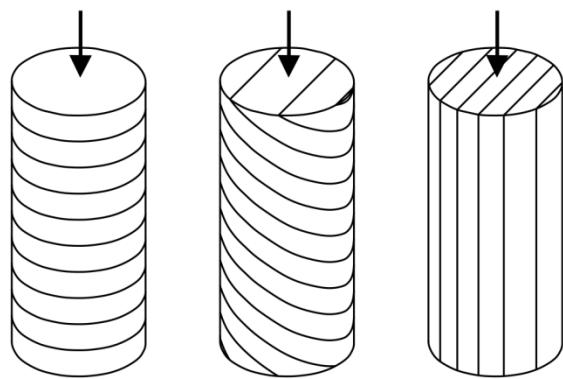
→ Influence of fracturing on hydraulic properties.

X Gallery air ventilation and water transfer (drainage / desaturation).



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- 3. Influence of mechanical anisotropy**
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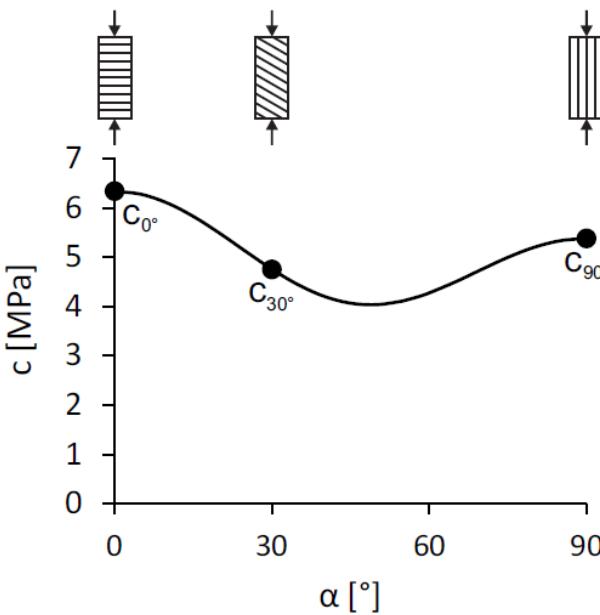
3. Influence of mechanical anisotropy



- Linear elasticity :

Cross-anisotropic (5 param.) + Biot's coefficients

$$E_{//}, E_{\perp}, \nu_{////}, \nu_{/\perp}, G_{//\perp} \quad b_{//}, b_{\perp}$$



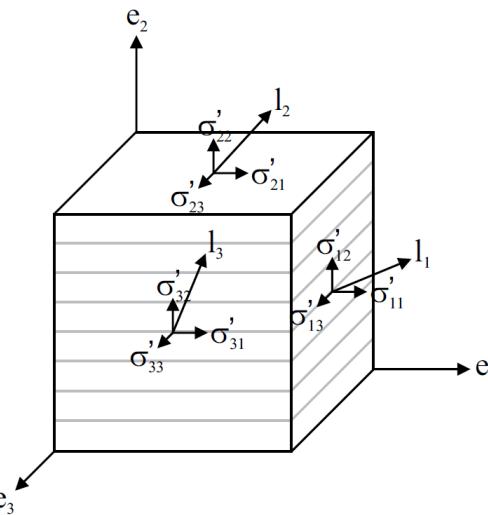
- Plasticity :

Cohesion anisotropy with fabric tensor

$$c_0 = a_{ij} l_i l_j \quad l_i = \sqrt{\frac{\sigma_{i1}^{'2} + \sigma_{i2}^{'2} + \sigma_{i3}^{'2}}{\sigma_{ij}' \sigma_{ij}'}}$$

Cross-anisotropy

$$c_0 = \bar{c} \left(1 + A_{////} (1 - 3l_2^2) + b_1 A_{////}^2 (1 - 3l_2^2)^2 + \dots \right)$$



3. Influence of mechanical anisotropy

3.3. Gallery excavation modelling for anisotropic initial stress state

- Stress state

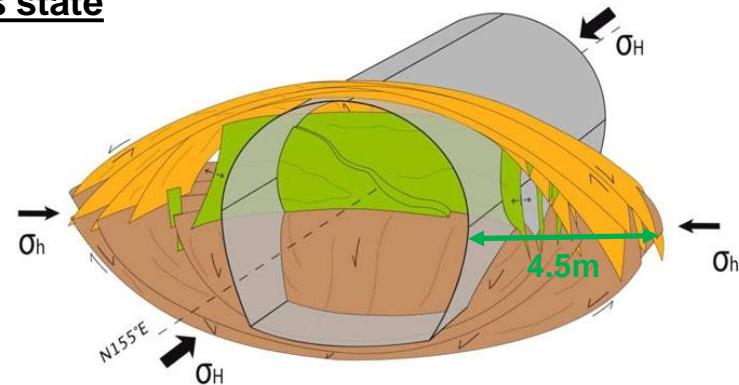
Major stress in the axial direction

Gallery // to σ_h

$$\sigma_{x,0} = \sigma_h = 12.40 \text{ MPa}$$

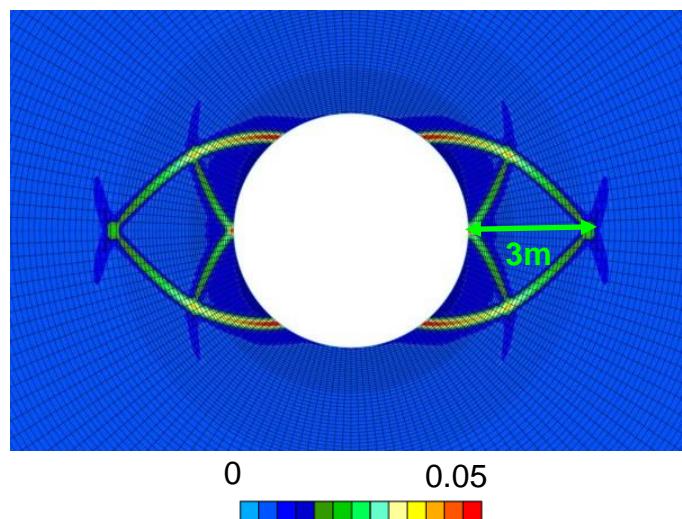
$$\sigma_{y,0} = \sigma_v = 12.70 \text{ MPa}$$

$$\sigma_{z,0} = \sigma_H = 1.3 \times \sigma_h = 16.12 \text{ MPa}$$



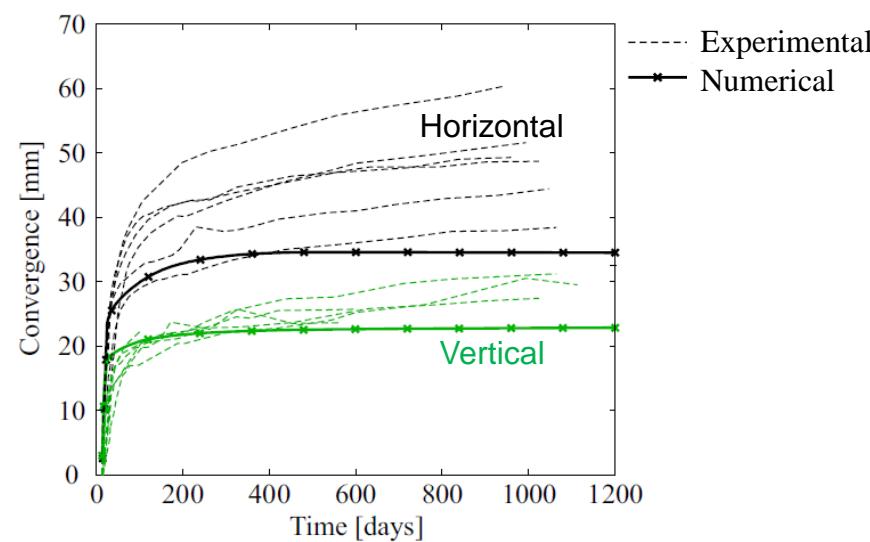
- Shear banding

Total deviatoric strain



→ Shape modification due to σ_h

- Convergence



→ Long-term deformation

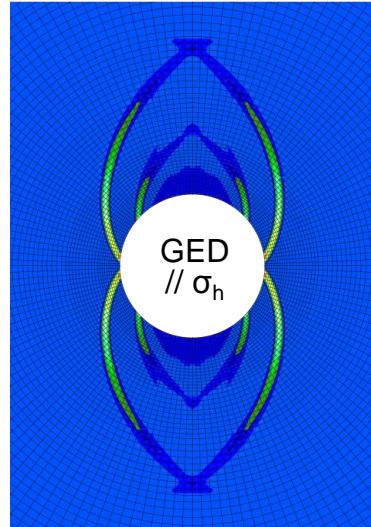
→ Creep deformation

3. Influence of mechanical anisotropy

3.4. Conclusions and outlooks

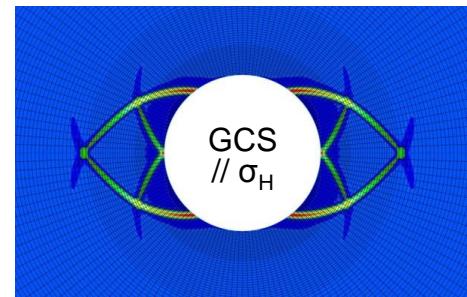
- Conclusions

- ✓ Reproduction of EDZ in both directions.
- ✓ Shape and extent of EDZ governed by:
 - **anisotropic stress state.**
 - **anisotropic mechanical behaviour.**



- Next steps ...

- X HM coupling in EDZ.
 - Influence of fracturing on hydraulic properties.
- X Gallery air ventilation and water transfer.



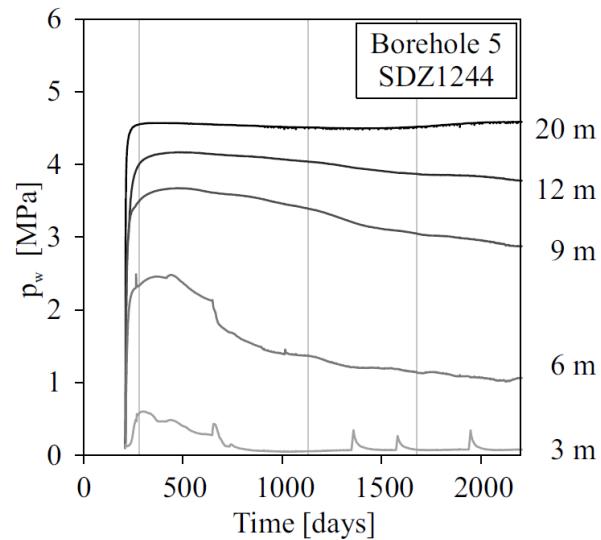
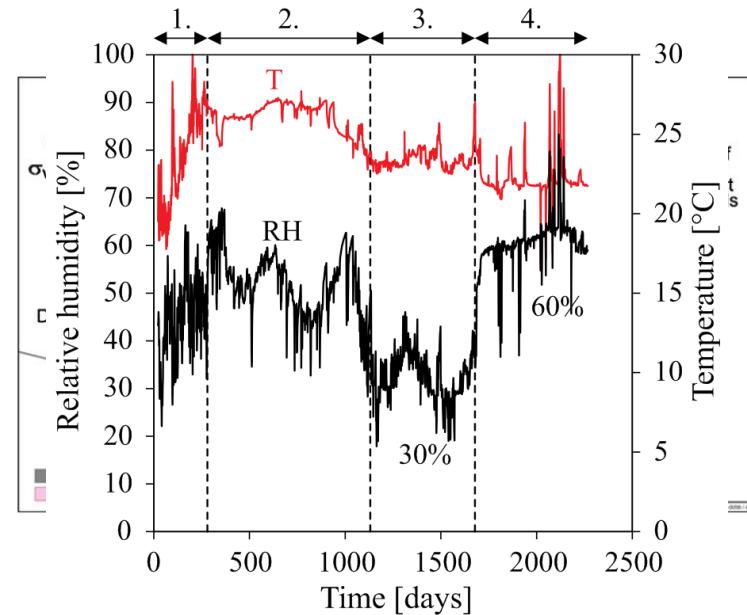
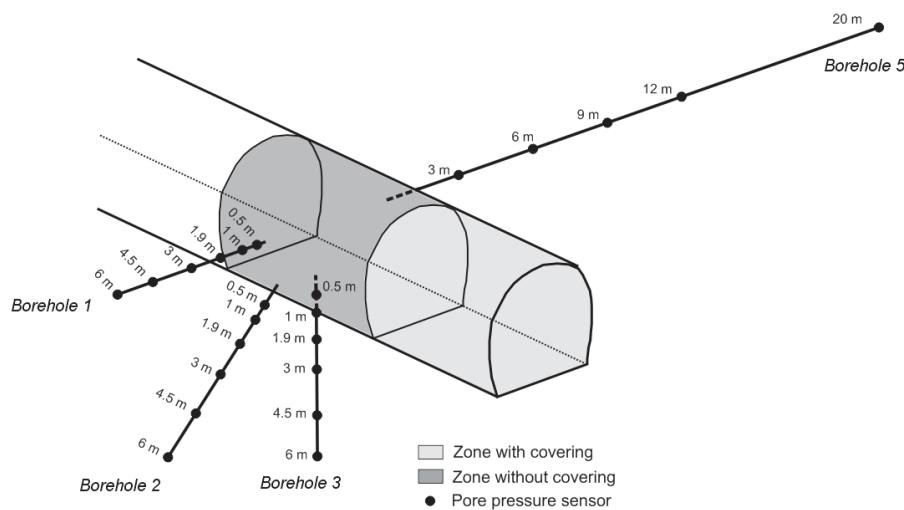
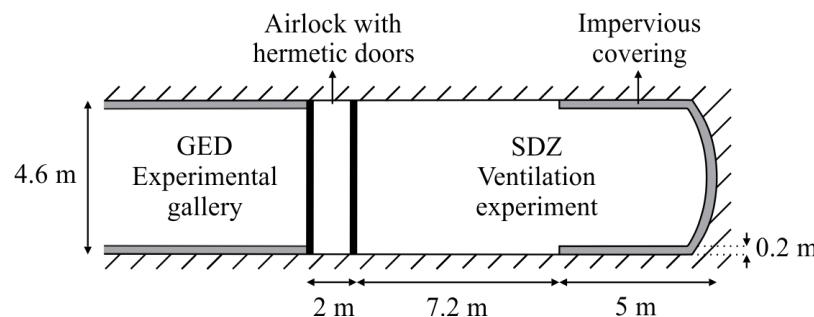
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4. Permeability evolution and water transfer

4.1. Large-scale experiment of gallery ventilation (SDZ)

Characterise the effect of gallery ventilation on the hydraulic transfer around it.

- drainage / desaturation
- exchange at gallery wall



4. Permeability evolution and water transfer

4.4. Modelling of excavation and SDZ experiment

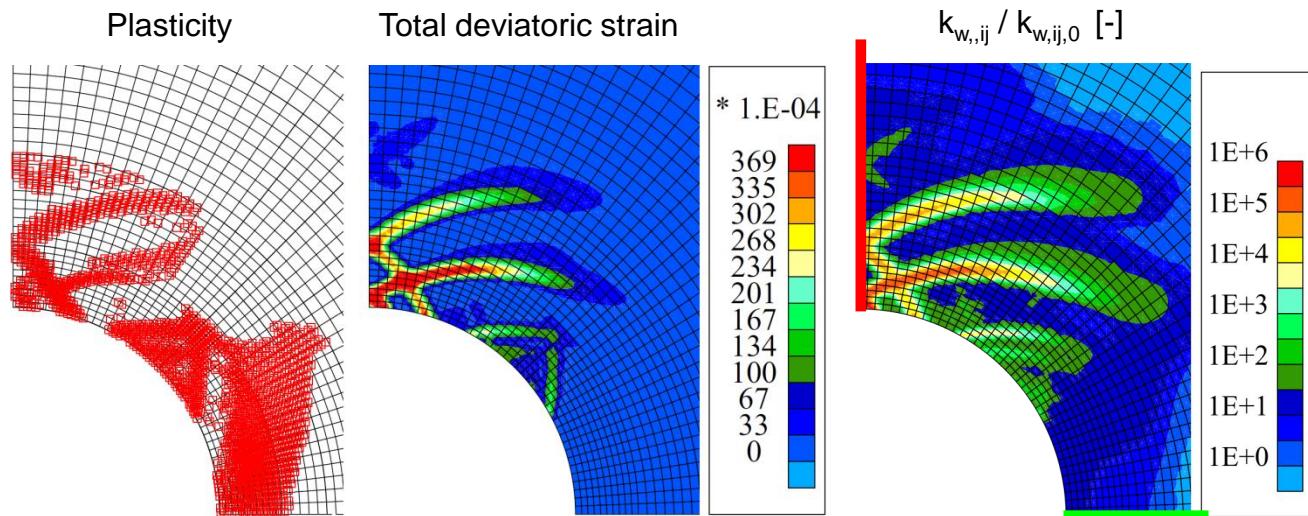
4.4.1. HM coupling in EDZ

- Gallery excavation

SDZ \rightarrow GED gallery // σ_h

Anisotropic $\sigma_{ij,0}$ and material

\rightarrow Localisation zone dominated by stress anisotropy

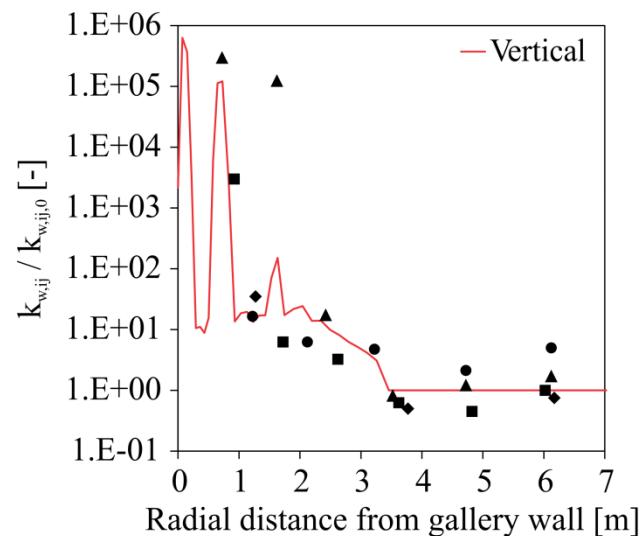


- Intrinsic permeability evolution

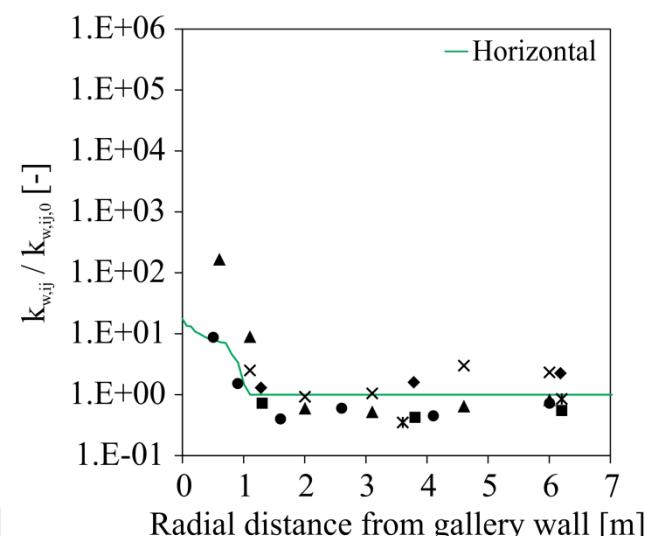
$$\frac{k_{w,ij}}{k_{w,ij,0}} = \left(1 + \beta \langle YI - YI^{thr} \rangle \hat{\varepsilon}_{eq}^3\right)$$

$$YI^{thr} = 0.95$$

Cross-sections

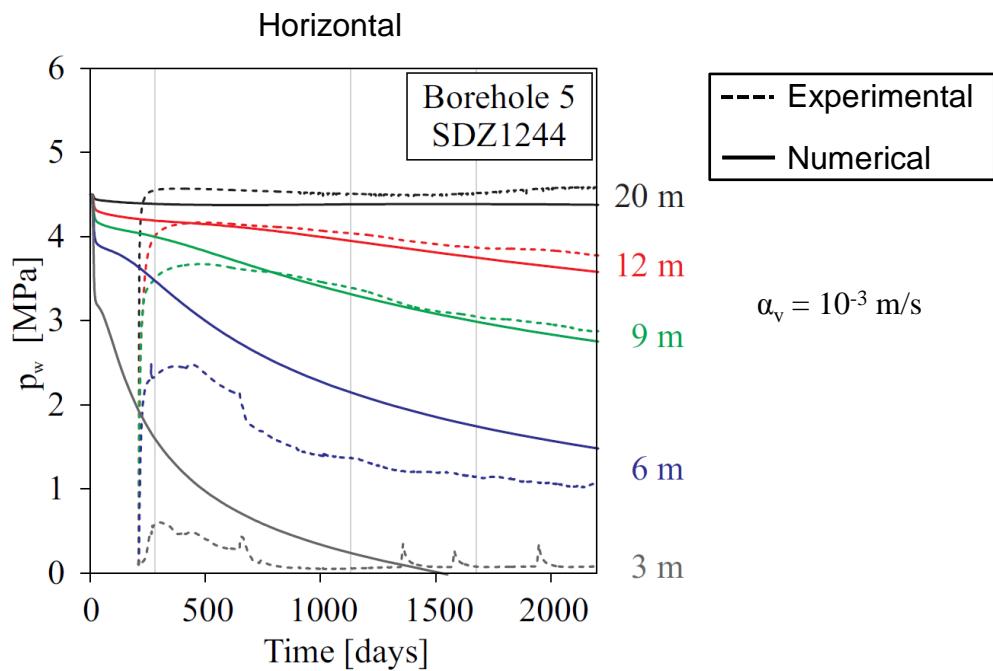
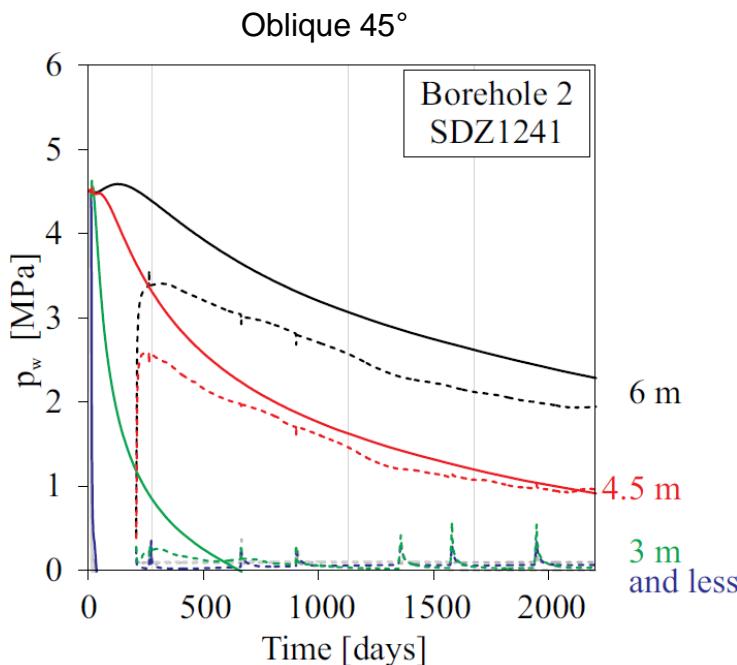
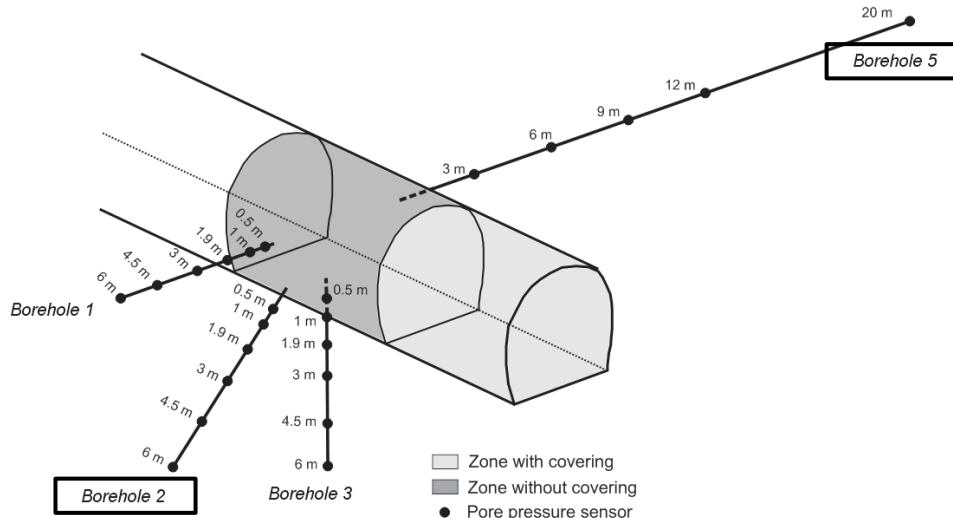


Plastic strain and a part of the elastic one \rightarrow EDZ extension + k_w increase



4. Permeability evolution and water transfer

- Drainage / p_w reproduction



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Viscosity

Viscosity

Time-dependent plastic strain, delayed plastic deformation

Progressive evolution of the material microstructure or to mechanical properties degradation (damage)

$$\dot{\varepsilon}_{ij} = \dot{\varepsilon}_{ij}^e + \dot{\varepsilon}_{ij}^p + \dot{\varepsilon}_{ij}^{vp}$$

Viscoplastic loading surface and potential surface:

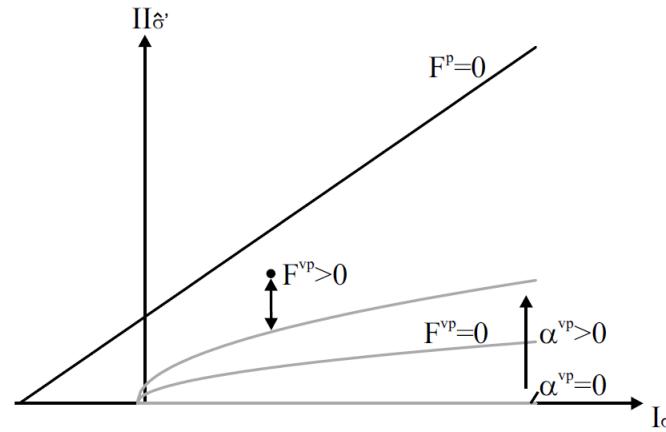
$$F^{vp} \equiv \sqrt{3} II_{\hat{\sigma}} - \alpha^{vp} g(\beta) R_c \sqrt{A^{vp} \left(C^{vp} + \frac{I_{\sigma'}}{3R_c} \right)} = 0$$

$$G^{vp} \equiv \sqrt{3} II_{\hat{\sigma}} - (\alpha^{vp} - \beta^{vp}) g(\beta) R_c \left(C^{vp} + \frac{I_{\sigma'}}{3R_c} \right) = 0$$

$$\dot{\varepsilon}_{ij}^{vp} = \gamma \left\langle \frac{F^{vp}}{R_c} \right\rangle^N \frac{\partial G^{vp}}{\partial \sigma_{ij}}$$

Delayed viscoplastic hardening function:

$$\alpha^{vp} = \alpha_0^{vp} + (1 - \alpha_0^{vp}) \frac{\varepsilon_{eq}^{vp}}{B^{vp} + \varepsilon_{eq}^{vp}}$$



- Current stress state
- Plastic loading surface
- Viscoplastic loading surface with hardening ($\alpha^{vp} \geq 0$)

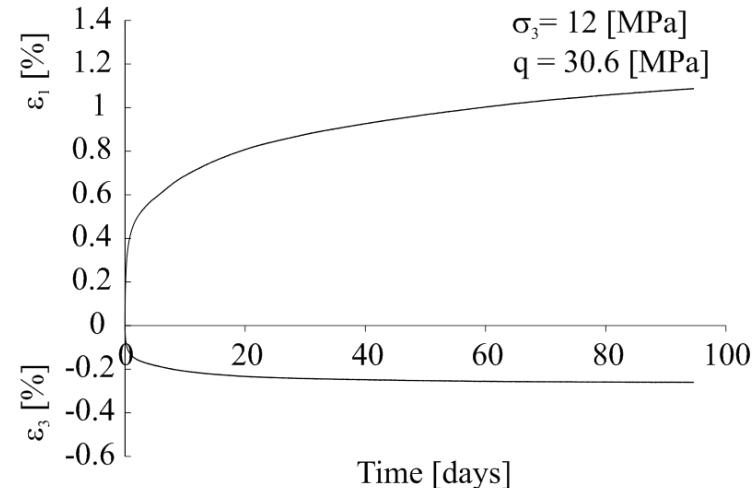
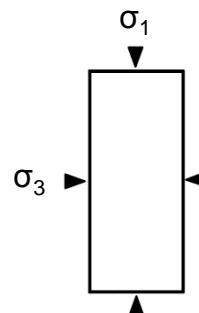
3. Influence of mechanical anisotropy

- Creep deformation

Permanent strain

In the long term

Under constant stress
below the yield strength



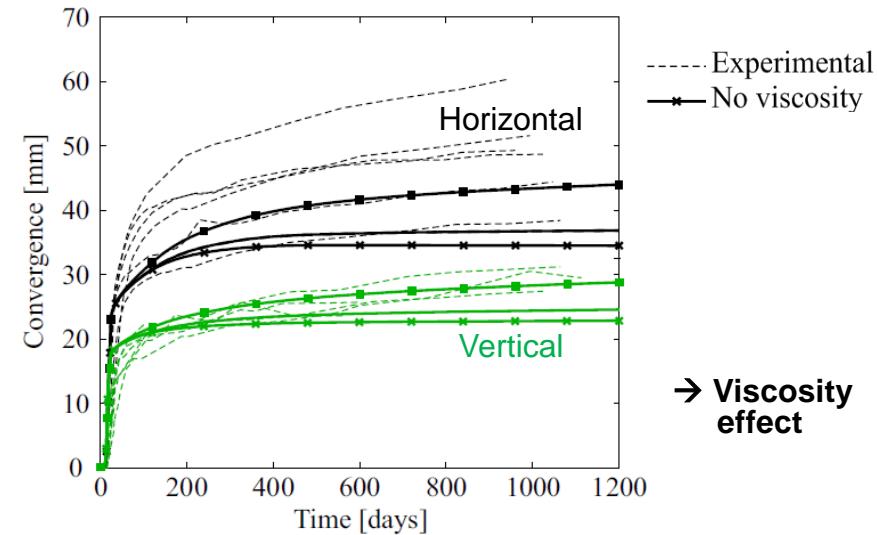
- Viscosity

Time-dependent plastic strain
(Jia et al., 2008; Zhou et al. 2008)

$$\dot{\varepsilon}_{ij} = \dot{\varepsilon}_{ij}^e + \dot{\varepsilon}_{ij}^p + \dot{\varepsilon}_{ij}^{vp}$$

$$F^{vp} \equiv \sqrt{3} II_{\hat{\sigma}} - \alpha^{vp} g(\beta) R_c \sqrt{A^{vp} \left(C^{vp} + \frac{I_{\sigma'}}{3R_c} \right)} = 0$$

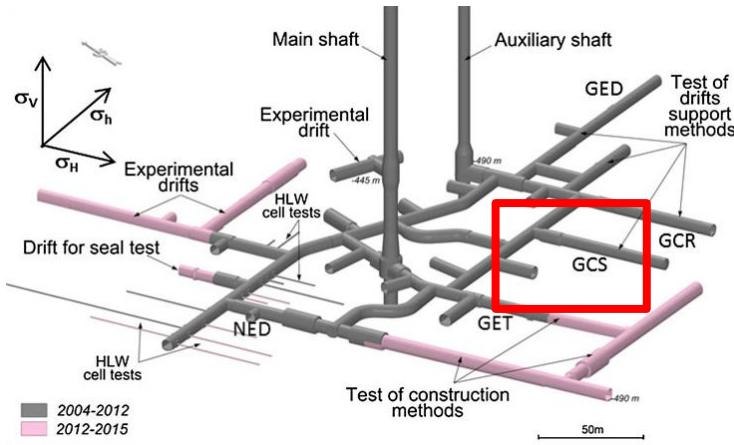
- Convergence



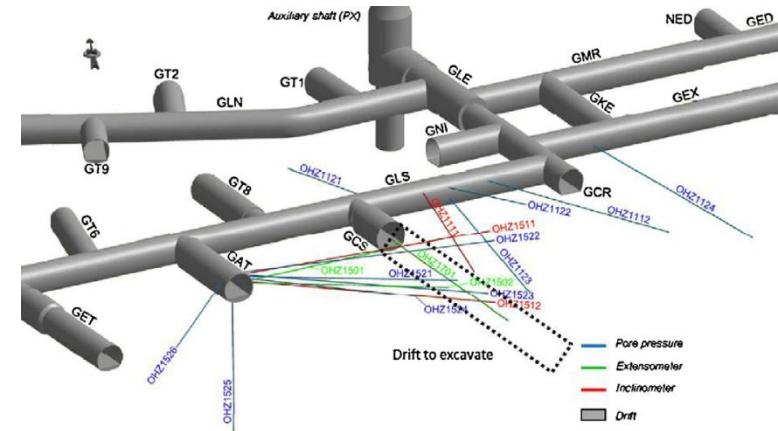
Mine-by experiment

- Displacements

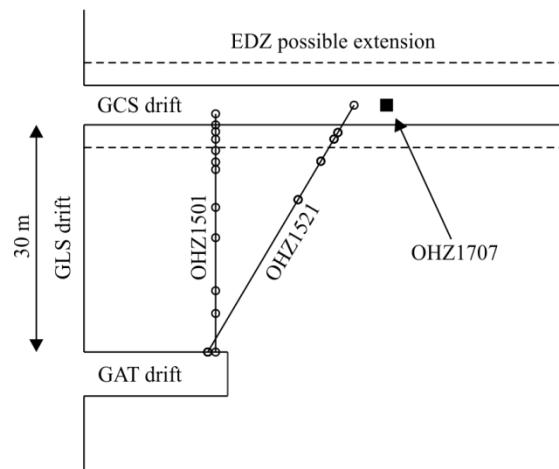
Andra's URL



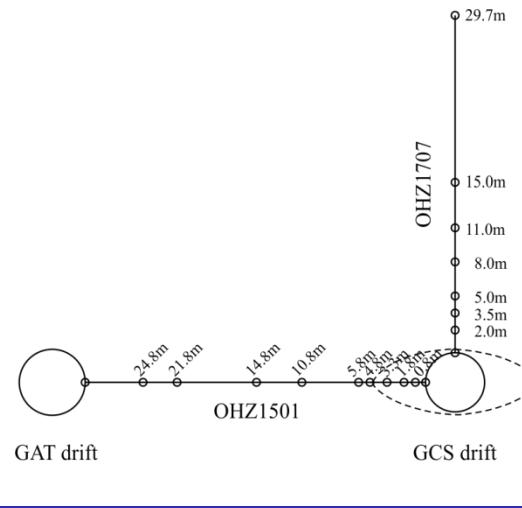
Mine-by experiment



Borehole – extensometers and pore pressure



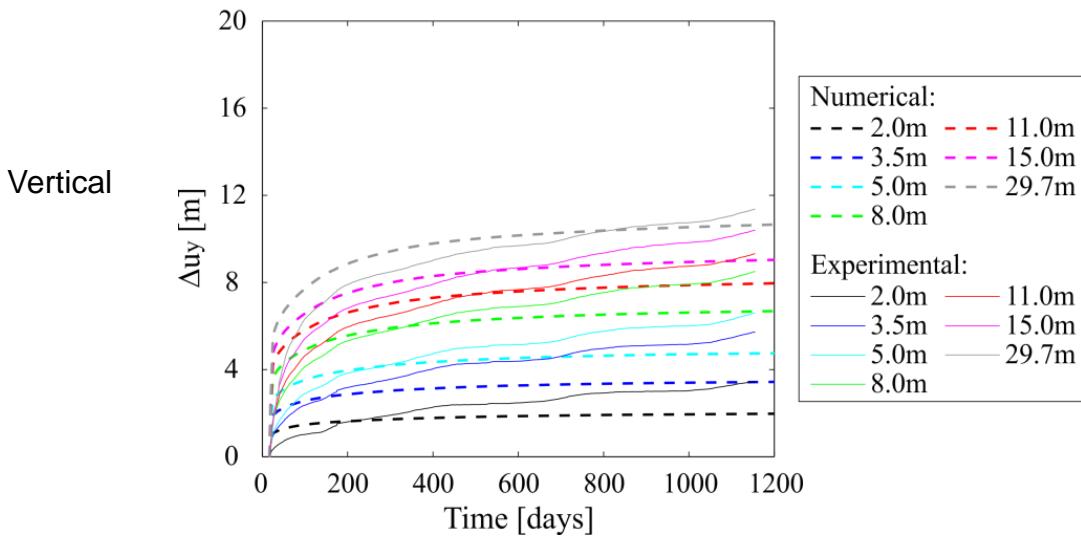
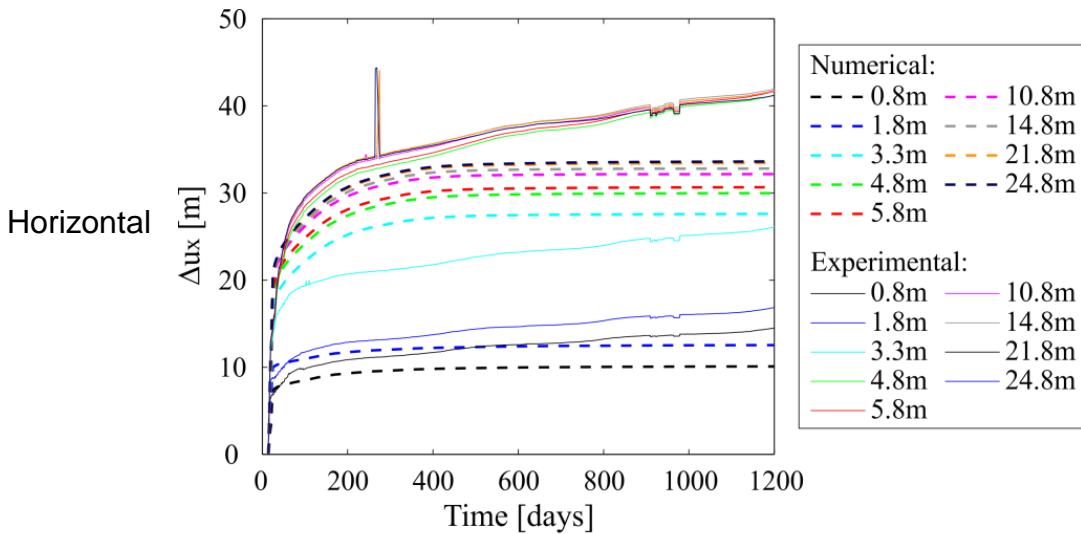
→ Characterise the displacements in the rock mass



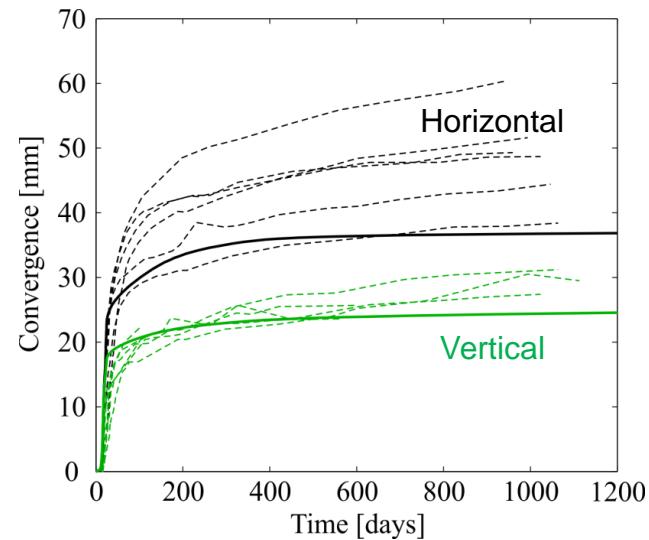
Mine-by experiment

- Displacements

Viscosity based on **creep tests**



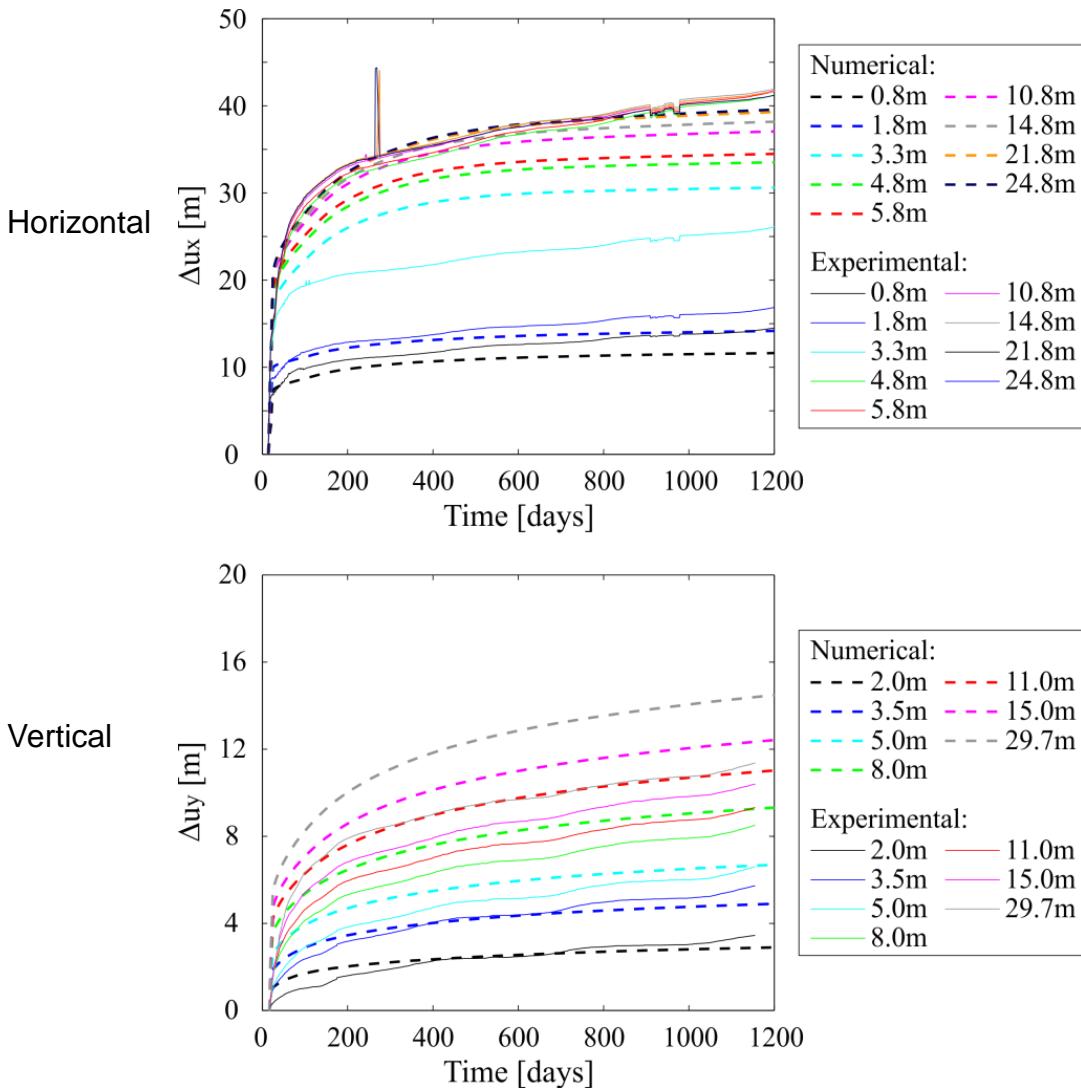
- Convergence



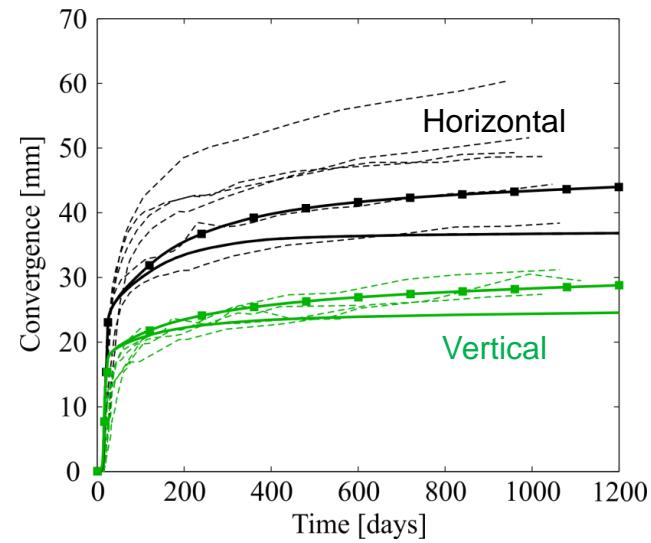
Mine-by experiment

- Displacements

Viscosity based on **in situ measurements** → Viscosity influence



- Convergence



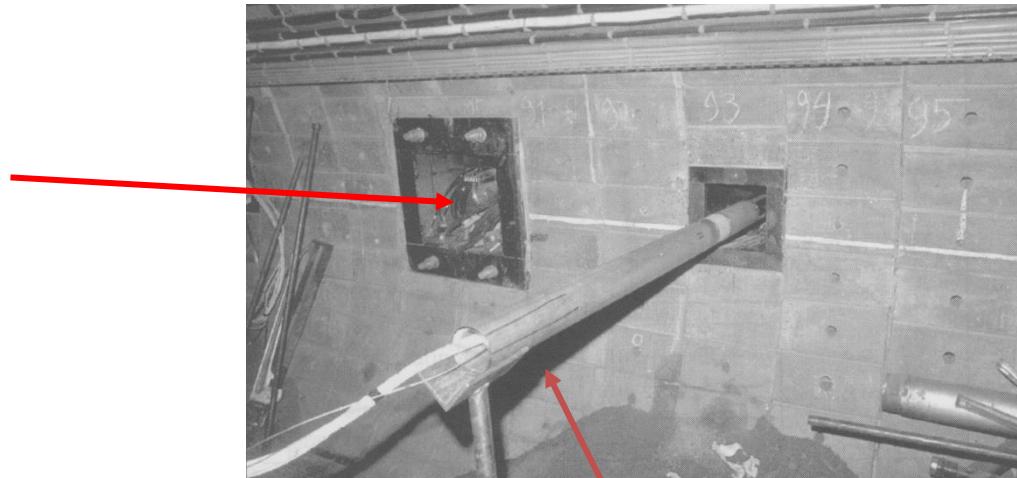
Viscosity allows to reproduce the increase of convergence in the long term.

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General framework of ATLAS experiment at Mol URL

ATLAS (Admissible Thermal Loading for Argillaceous Storage)

Heating
borehole



- ATLAS I & II

Part of the EC INTERCLAY-II project (1990-1994)
An experiment for modellers (blind predictions)

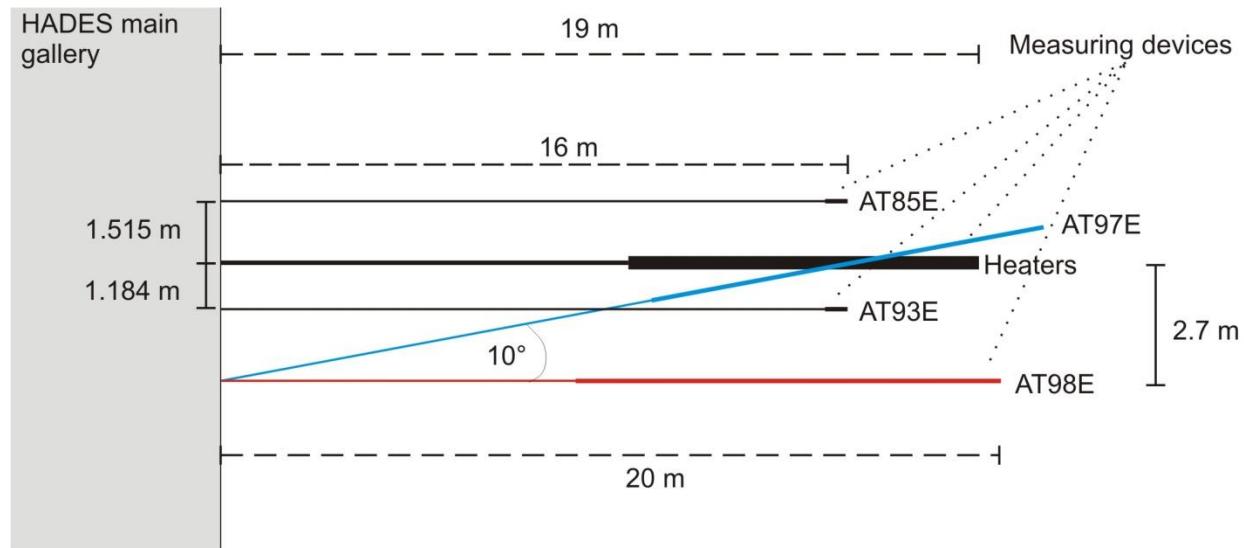
- ATLAS III (April 2007 → April 2008) – EC TIMODAZ project

Investigate the characterisation of the effect of thermal loading on thermo-hydro-mechanical properties of Boom Clay (thermal conductivity, THM coupling in clay...)

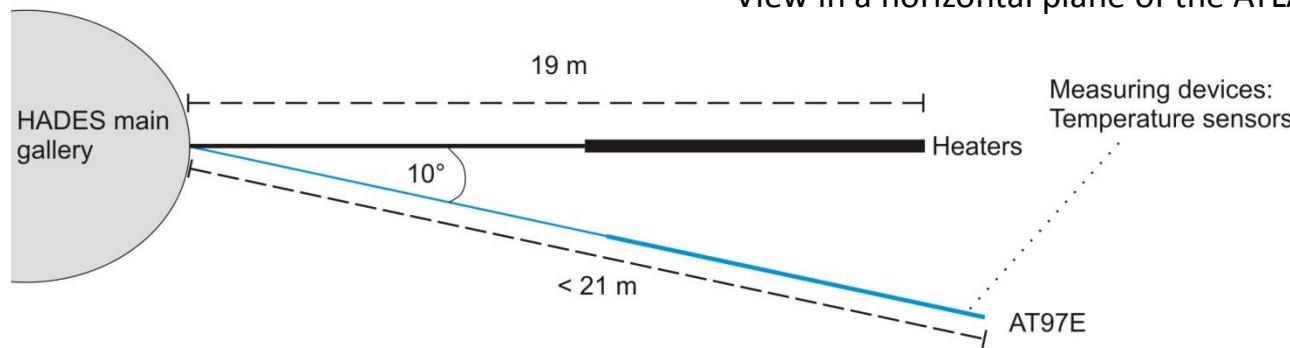
Installation of
instrumentation in
observation borehole

ATLAS III

- 2 new boreholes
 - AT98E (P_w , T)
 - AT97E (T)



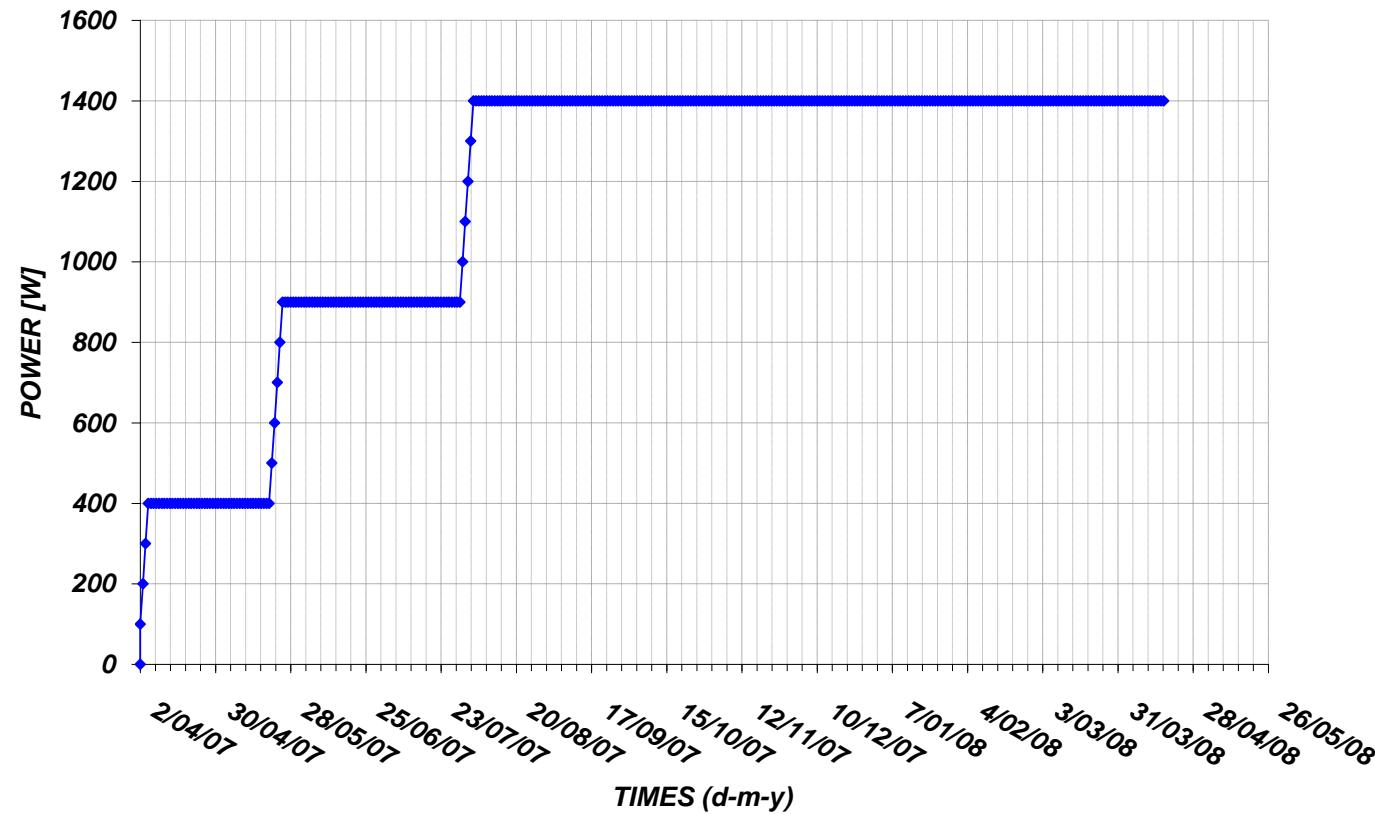
View in a horizontal plane of the ATLAS experiment

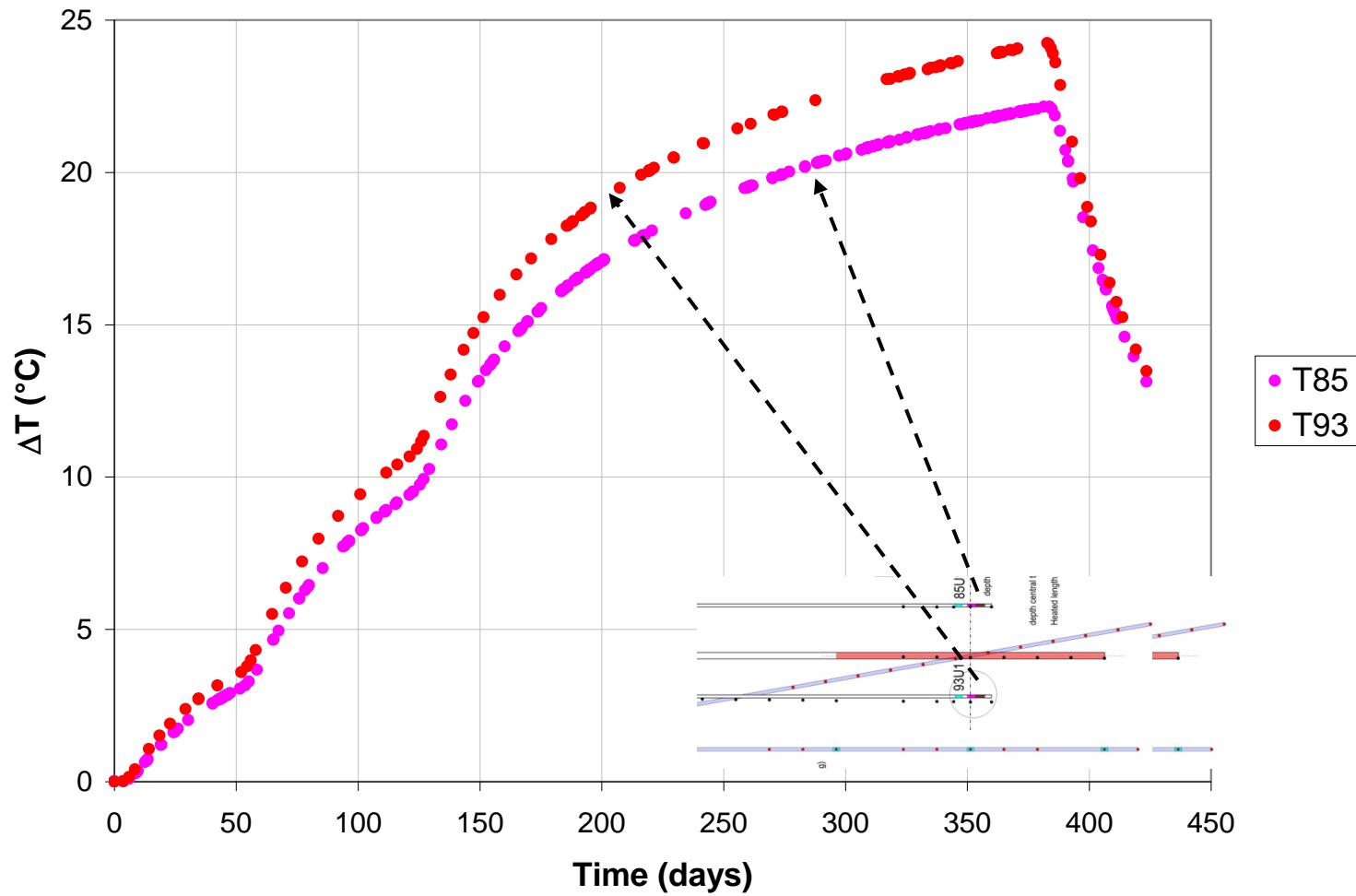


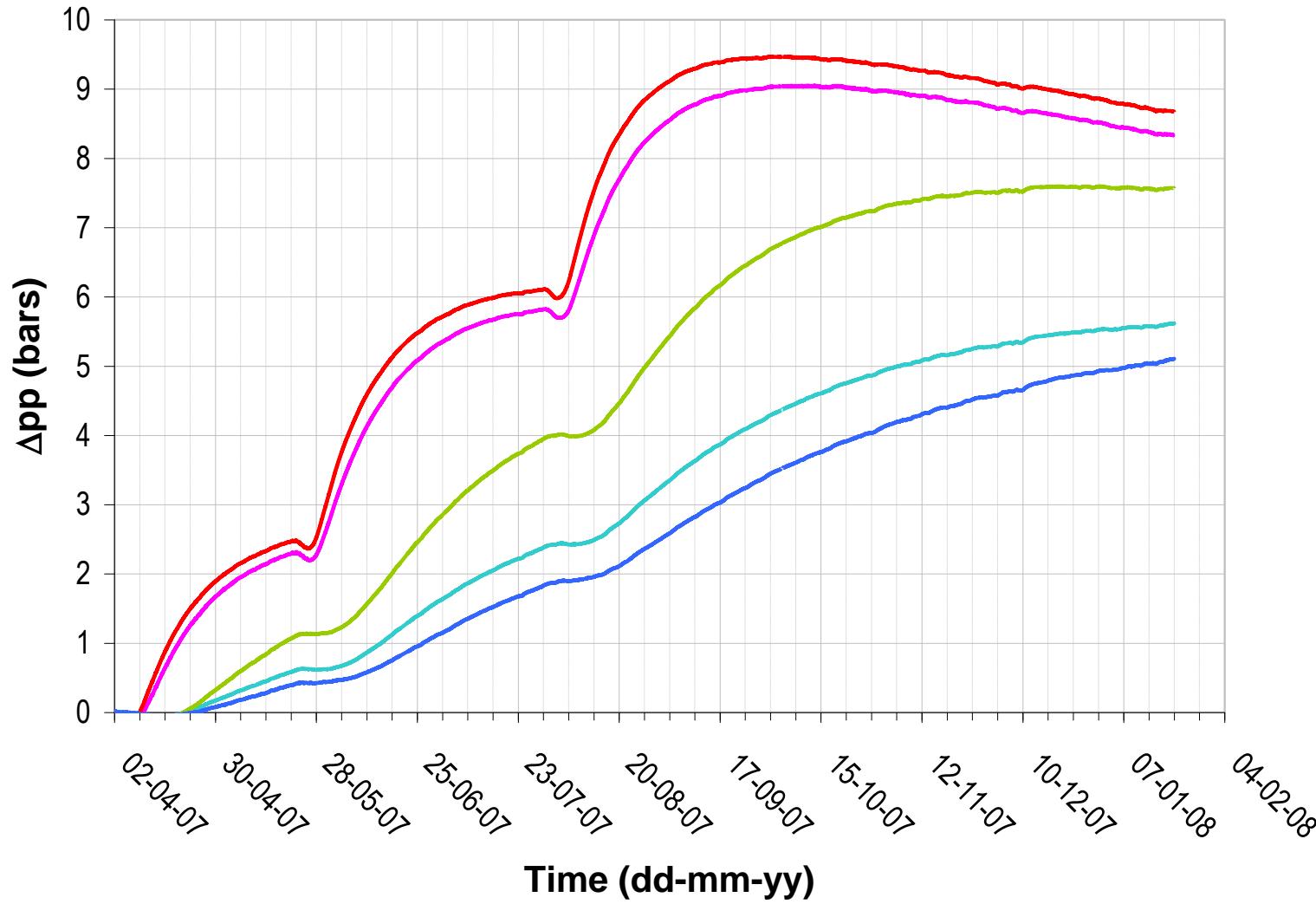
View in a vertical plane of the ATLAS experiment

Power evolution during *ATLAS III*

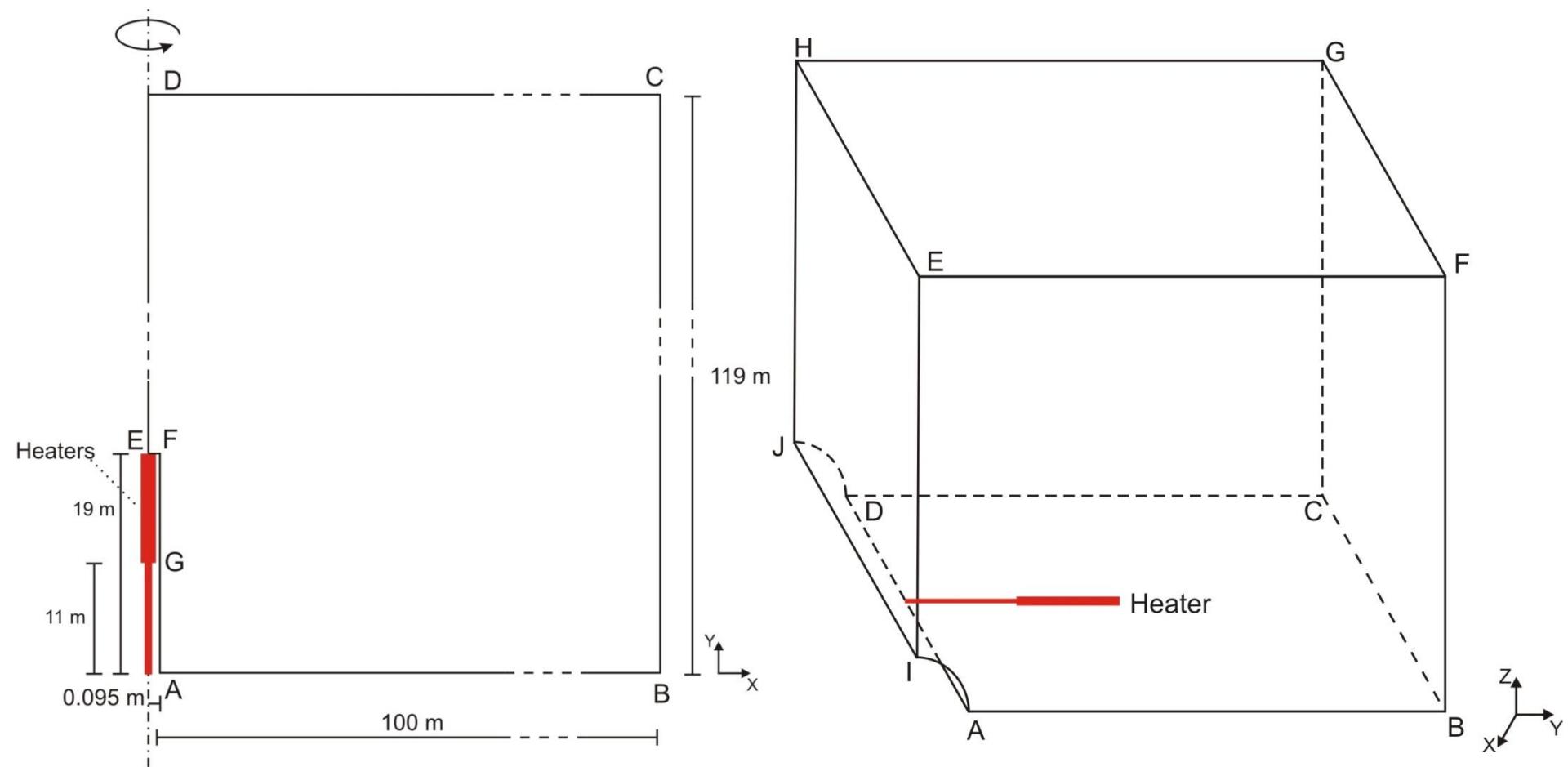
Heating phase in 3 steps: 7 weeks, 10 weeks, 43 weeks



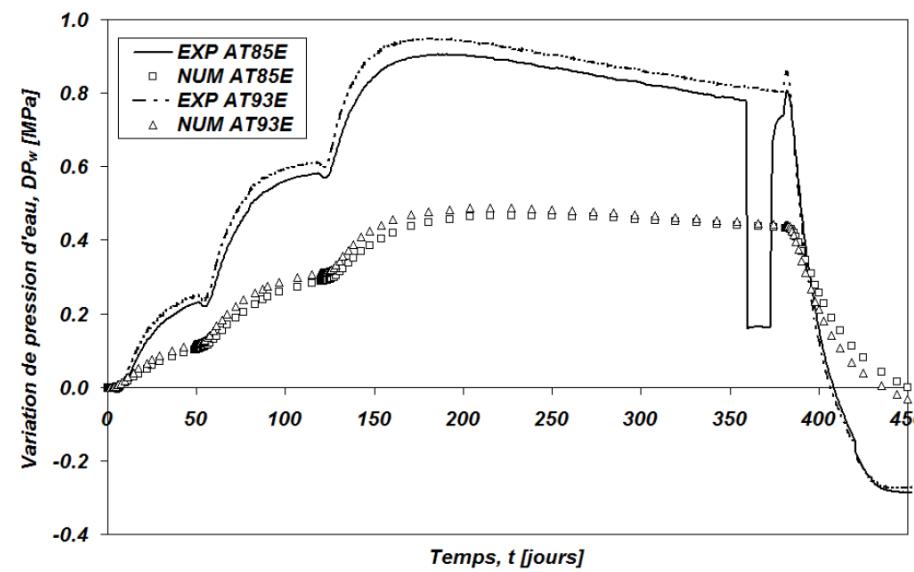
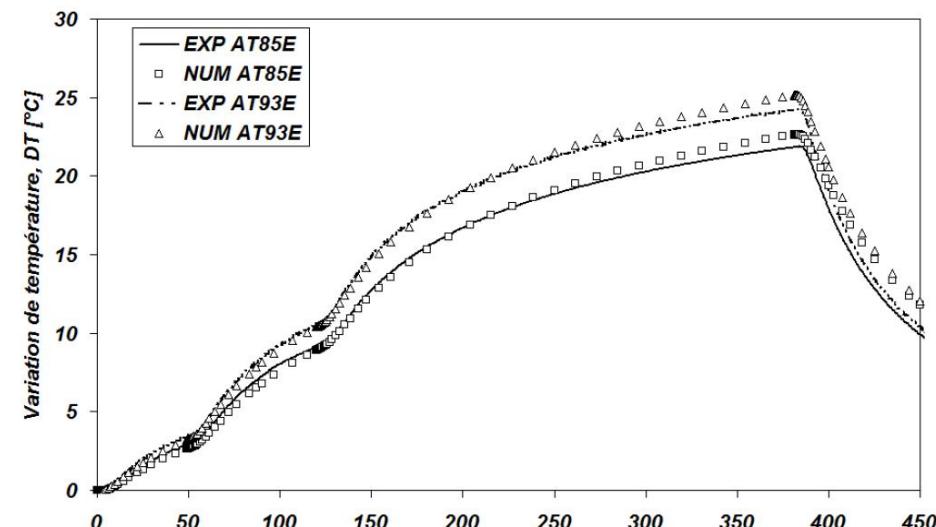
Results **ATLAS III: evolution of the temperature with time**

Results **ATLAS III: evolution of pore pressure with time**

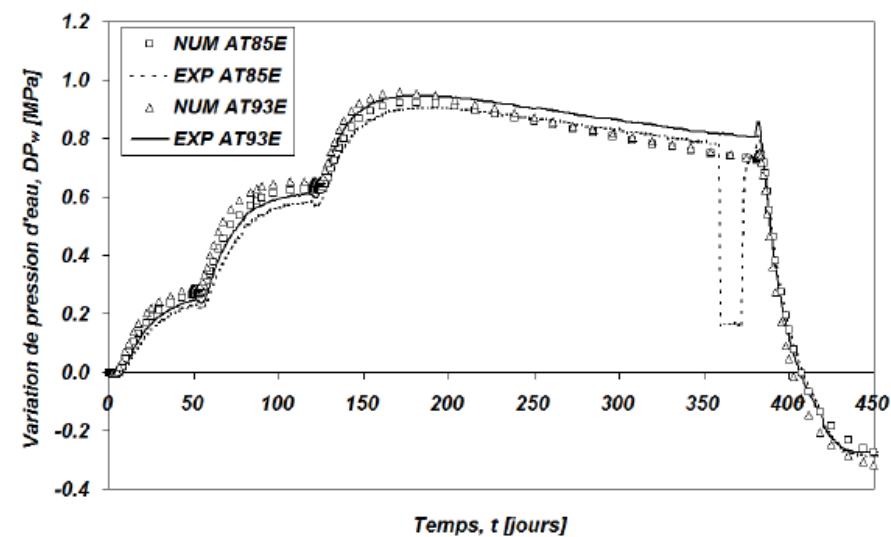
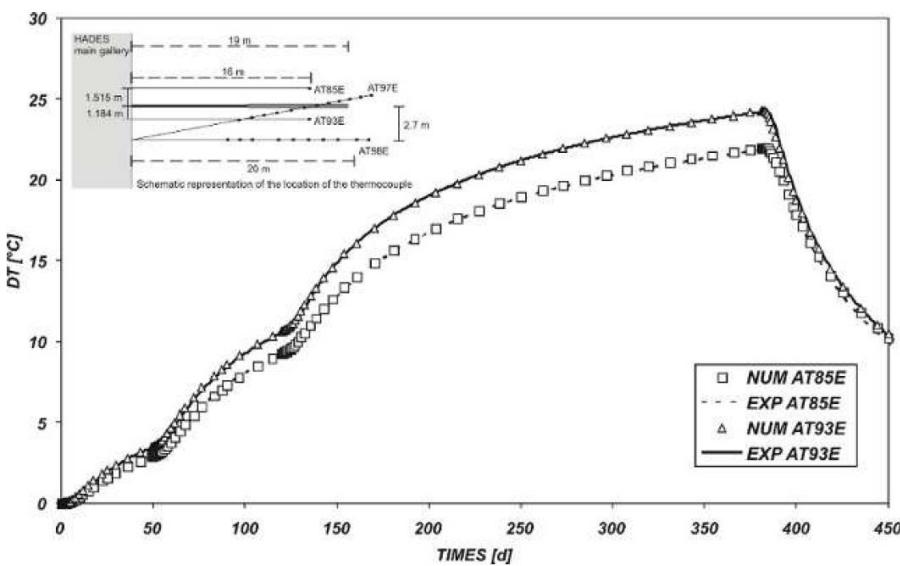
Prediction ***ATLAS III: choice of the FE model (2D vs 3D)***



Prediction *ATLAS III: 2D axisymmetric (isotropic) model*



Prediction ATLAS III: 3D with anisotropic constitutive model and small strain stiffness

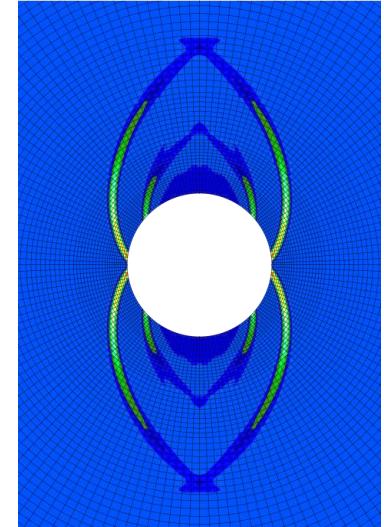
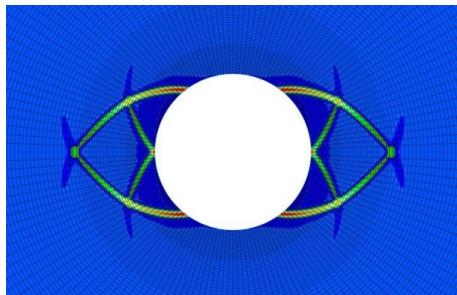


1. Context
2. Fracture modelling with shear bands
3. Influence of mechanical anisotropy
4. Permeability evolution and water transfer
5. Creep deformation
6. THM couplings
7. **Conclusions and perspectives**

5. Conclusions and perspectives

Conclusions

Better understand, predict, and model the behaviour of the EDZ in partially saturated clay rock, at large scale.



Fracture description

EDZ with strain localisation.

Constitutive models

Mechanics: anisotropy, viscosity.

Coupled: fracture influence on permeability.

Numerical modelling

Shape, extent.

Influence of fracturing, permeability variation, anisotropy.

Water transfer.

Contribution : Provide new elements for the prediction and understanding of the HM behaviour of the EDZ.

Innovations : Fracturing process is predicted on a **large scale** with **shear bands**.

Strain localisation effects are taken into account in **coupled processes** (water flow).