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#### Differentiated influence of the double porosity of the chalk on solute and heat transport

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12 **Abstract:** The chalk porosity plays a decisive role in the transport of solutes and heat in saturated chalk. From a geological point of view, there are at least two types of porosities: the 13 14 porosity of pores corresponding to the micro-spaces between the fossil coccoliths that form 15 the chalk matrix, and the porosity due to the micro- and macro-fractures (i.e., secondary 16 porosity). For groundwater flow, the fracture porosity is a determining factor at the macroscopic scale. The multi-scale heterogeneity of the porous/fractured chalk is inducing 17 18 different effects on solute and heat transport. For solute transport considered at the 19 macroscopic scale, tracer tests have shown that the 'effective transport porosity' is 20 substantially lower than the 'effective drainable porosity'. Moreover, breakthrough curves of tracer tests are showing an important influence of diffusion in a large portion of 'immobile 21 22 water' ('matrix diffusion') together with quick preferential advection through the fractures. For heat transport, the matrix diffusion in the 'immobile water' of the chalk is hard to distinguish 23 24 from conduction within the saturated chalk.

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26 Chalk aquifers are considered as excellent groundwater reservoirs (among others: Foster &

27 Milton 1974; Price 1987; Dassargues et al. 1988; Price et al. 1993; Dassargues & Monjoie

28 1993; Younger & Elliot 1995; MacDonald & Allen 2001; Nativ *et al.* 2003; William *et al.* 

29 2006) but at the same time, the saturated chalk of which they are composed is one of the most

30 complex geological media due to multi-scale heterogeneity (Fig. 1). Groundwater quality

issues in chalk aquifers have been studied and discussed for a very long time (among others:
Gray & Morgan-Jones 1980; Jackson *et al.* 1984; Hallet 1998; Nativ *et al.* 1999; Gooddy *et*

*al.* 2001; Brouyère *et al.* 2004; Batlle-Aguilar *et al.* 2007; Orban *et al.* 2010; Hakoun *et al.* 

34 2017; Boudjana *et al.* 2019). A detailed understanding of solute transport in the chalk aquifers

35 is thus an important issue.

36 More recently, the increasing consideration of renewable energies is resulting in a huge

37 expansion of the interest on the quantification of heat transfer in partially and fully saturated

38 aquifers including chalk aquifers. Site specific values for heat transport properties/parameters

39 are needed to design shallow geothermal reservoirs and heat storage systems (i.e. Borehole

40 Thermal Energy Storage (BTES) and Aquifer Thermal Energy Storage (ATES) systems) in

41 chalk aquifers. They will influence the short-, mid- and long-term performances of the

42 systems, as well as the estimated impacts on the groundwater resources.

43 For groundwater quality as for geothermal applications, it could be very important to have a

44 realistic quantification of the highly heterogeneous groundwater flow and transport processes

45 in porous/fractured chalk formations. This information is important for modelling doublets in

46 a double porosity context (Barker *et al.* 2010). Groundwater quantity and quality assessments

47 and the subsequent protection and remediation measures should be based on a sound and

accurate understanding of the processes and their quantification at the scale under consideration. Flow and solute transport are mainly driven by open fractures as preferential paths with very high velocities, while matrix processes can also have a strong influence on contamination duration (among others: Bloomfield 1996, Brouyère et al. 2000, Brouyère et al. 2004, Weiss et al. 2006, Massei et al. 2006, El Janyani et al. 2014, Tamayo-Mas et al. 2018, Hoffmann et al. 2020). Thermal storage properties could also be strongly dependent on the multi-scale heterogeneity of the chalk as shown by studies recently published for other geological media (Molson et al. 1992, Wagner et al. 2014, Wildemeersch et al. 2014, Klepikova et al. 2016, De La Bernardie et al. 2019). 30 cm 

Fig. 1. Multi-scale heterogeneity of the chalk, from the micro-scale with coccoliths forming the chalk matrix
 showing a pore porosity (above) towards macro-scales at which the porosity is also influenced by cracks and
 fractures (center and below).

#### 96 **Porosity definitions**

- 97 The *total porosity* is defined as:
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 $n = V_v / V_t$ 

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101 where  $V_{v}$  is the void volume and  $V_{t}$  the total volume with respect to a defined representative

102 elementary volume (REV). Here, the void volume includes the pores but also all kind of

fractures, cracks, channels, and bedding planes. The porosity is indeed the result of various processes (i.e., physical and chemical) having induced changes in the chalk throughout its

105 geological history. In geology, the terms *primary porosity* and *secondary porosity* are used to

106 describe, respectively, the initial porosity of the chalk, and the acquired additional porosity

107 due to diagenesis, fracturing or deformation, and then dissolution. This latter can often be

108 considered as the dominant process for enhancing permeability (Price 1987, Price *et al.* 1993,

109 Foster 1993). In chalks, the secondary porosity consists most often of a fracture porosity as

opposed to interstitial or matrix porosity. This fracture porosity can have more influence on groundwater flow and the storage properties than the primary porosity. In fact, chalks can be

112 considered as dual porosity media with fracture network and matrix porosities, even if a part

113 of this matrix porosity can be reduced by cementing materials in the pore spaces.

114 The definition of porosity implies conceptually that a given volume of the chalk is considered,

115 which is big enough to yield a value considered as representative at that scale. In practice, the

116 REV concept is always implicitly considered even though this is not necessarily

117 acknowledged (de Marsily 1986). Behind this concept lies the fact that an equivalent value is

118 used (Bachmat and Bear 1986, Bear and Verruijt 1987) that is the result of a homogenization

119 process at that REV scale. The size of the later is chosen essentially according to the aims of 120 the study.

121 In many different aquifers, but especially in chalk aquifers, observations at the scale of lab

tests (up to a few decimetres) or at the scale of field tests (e.g., pumping tests) imply that a

123 part of the groundwater in the aquifers can be considered as immobile in the pores of the

124 matrix. In all textbooks about hydrogeology an effective porosity is distinguished from the

125 total porosity.

126 In saturated conditions and for groundwater flow problems, an *effective porosity* is usually

- 127 defined as the drainable porosity under the in-situ pressure conditions:
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$$n_e = V_m / V_t$$

(2)

(1)

131 where  $V_m$  is the volume of mobile water in the REV. This corresponds to the specific yield

132  $(S_{\nu})$ . This specific yield or drainable effective porosity gives also a reliable value for the

133 storage coefficient in unconfined aquifers (i.e., as consolidation effects can be neglected in

134 unconfined conditions).

135 If solute transport is considered, it remains difficult to know accurately which part of the 136 saturated porosity is occupied by mobile water, on which the transport velocity is based 137 (Dassargues 2018). In particular, in aquifers where a fracture porosity is usually lower or around 138 1 %, very rapid transport (advection) velocities are observed (Worthington 2015). On the basis 139 of all observations from tracer tests and contaminations, distinction must be made 140 systematically between the effective drainable porosity and an effective transport porosity 141 which takes typically a lower value than the former one (Hallet & Dassargues 1998, Brouyère et al. 2000, Payne et al. 2008, Hadley & Newell, 2014). In addition, Worthington et al. (2019) have recently demonstrated that the transient nature of effective porosity and specific yield is of importance for an unconfined chalk aquifer, highlighting that these values should not necessarily be treated as similar, time-invariant values.

The advection velocity is defined as: 

$$\boldsymbol{v}_{\boldsymbol{a}} = \boldsymbol{q}/n_m \tag{3}$$

where q is the groundwater Darcy flux, and  $n_m$  is the *effective transport porosity* (Dassargues 2018). This later is in theory very dependent on what is actually defined as mobile and immobile groundwater in the REV. In practice however, if experimental results (i.e., pumping tests and tracer tests results) have to be reproduced by model results, a lower value for the

effective transport porosity is to be chosen than for the effective drainable porosity. 

Especially in the chalk, if the Darcy flux were divided by the total porosity n (or even by the effective drainable porosity  $n_e$ ), this would lead to a kind of averaged velocity which can be

misleading (i.e., leading to underestimation of actual velocities). However, one can argue that

this could be partially and artificially compensated in models by considering larger values of 

the longitudinal dispersivity, but it is also misleading as it only counterbalances a correlation

between parameters and not improve the model conceptualisation towards an accurate 

- parameterisation (Dassargues 2018).

#### Dual porosity and hydraulic conductivity of chalk

The dual porosity concept, with micro-pores in the solid porous matrix and macro-pores

corresponding to fractures or bedding, was already considered by Gerke & van Genuchten

(1993). If it is applied at larger scale, subzones of low and high hydraulic conductivity also

form a dual permeability medium (Dassargues 2018).

Indeed, a dual or triple porosity of the chalk could be considered as a result of the geological

processes having led to these pores, micro- and macro-fractures (Fig. 1 and Fig. 2). The

influence of this multi-scale heterogeneity can be considered differently in function of the

studied processes (i.e., groundwater flow, solute transport and heat transport). In each case,

the parameter values are considered at a given scale (i.e., the scale of the REV) depending on the problem to be solved.





- 189 As an example, if only groundwater flow properties are considered in the chalk aquifer of
- 190 Hesbaye (Cretaceous chalks of the Maastrichtian and Campanian ages of the Geer basin in
- 191 Belgium), different values (see below) were found according to the considered size of the
- 192 REV (Dassargues 2018).
- 193 At the *microscopic scale* (up to a few centimeters), one considers essentially the matrix of a
- 194 chalk sample. Aggregated coccolith fossils of a few microns are forming this chalk matrix.
- 195 The measured porosity may vary depending on localized calcium carbonate (CaCO<sub>3</sub>)
- 196 precipitation. The overall spatial distribution of voids seems relatively homogeneous in the
- 197 samples of a few centimeters, so they are apparently adequate REVs to measure a reliable
- pore porosity. The following values have been measured: n = 0.40 0.42 and  $n_e = 0.35$ .
- 199 Consequently, the hydraulic conductivity values are quite low  $K \cong 1.0 \ 10^{-8}$  (m/s).
- 200 At the *macroscopic scale* (up to a few decimeters), any REV actually includes conjugate
- 201 micro-fractures networks (or micro-fractures) and chalk layering. The effective drainable
- 202 porosity (i.e., to be considered for a groundwater flow through this REV and influencing the
- hydraulic conductivity at this scale) is now strongly dependent the porosity of these micro-
- fractures: n = 0.42 0.45 and  $n_e = 0.01 0.03$ . Accordingly, larger values of hydraulic

205 conductivity are found with 
$$1.0 \ 10^{-5} \le K \le 1.0 \ 10^{-4}$$
 (m/s).

- 206 At the *megascopic* scale (scale of field tests such as pumping tests, up to a few hundred
- 207 meters), it is observed (through interpretation of tests and inverse modelling) that
- 208 'homogenization' in a large REV ensures that all fractures, faults and discontinuities should
- 209 be considered as interconnected, which increases the effective porosity and the hydraulic
- 210 conductivity values: n = 0.42 0.45 and  $n_e = 0.05 0.10$ ;  $1.0 \ 10^{-4} \le K \le 1.0 \ 10^{-3}$
- 211 (m/s).
- 212 Now if <u>solute or heat advection</u> is considered, the  $n_m$  values are systematically lower than the
- 213  $n_e$  values, representing a more accurate assessment of the actual mobile water volume for
- advective mass transport. For example, for solute transport, on the basis of 47 different tracer
- 215 tests in Belgian unconfined chalk aquifers performed for delineation of protection zones
- around pumping wells, the arithmetic mean, geometric mean and median values for  $n_m$  were found to be 0.0187, 0.0058, and 0.008, respectively (Briers *et al.* 2017).
- In summary, this concept of dual porosity is at the origin mostly influenced by the chalk
- 219 geological conditions and history. Its relative importance for possible consideration in the
- 219 geological conditions and history. Its relative importance for possible consideration in the
- quantification of solute and heat processes is largely dependent on the scope of the study and the associated scale of consideration.
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#### 223 Solute transport in chalk accounting for matrix diffusion/immobile water effect

Adding the contribution of the different solute mass processes, the solute conservation
equation for mobile groundwater can be written for a REV in aquifers (Dassargues 2018):

- $R\frac{\partial C}{\partial t} = -\boldsymbol{v}_{a} \cdot \nabla C + \nabla \cdot (\boldsymbol{D}_{h} \cdot \nabla C) R\lambda C \frac{q_{s}}{n_{m}}(C C_{s})$ (4)
- where *C* is the volume concentration (kg/m<sup>3</sup>) in the mobile groundwater,  $D_h$  is the
- 230 hydrodynamic dispersion tensor( $m^2/s$ ) including the mechanical dispersion and the diffusion
- 231 in the mobile water, R is the retardation factor due to adsorption-desorption processes (-),  $\lambda$  is
- 232 a first-order (linear) decay constant (s<sup>-1</sup>),  $q_s$  is the source/sink volumetric flow rate per unit
- volume of porous medium (s<sup>-1</sup>) flowing into ( $q_s > 0$ ) or flowing out from ( $q_s < 0$ ) the control
- volume and  $C_s$  is the associated concentration (kg/m<sup>3</sup>). In this equation, diffusion is
- considered only in the mobile water fraction of the saturated media.

In reality, diffusion occurs between mobile water and immobile water as a result of solute concentration differences. Especially in a porous/fractured chalk, the concentration (C) first rises in the very mobile groundwater in the fractures (Fig. 3). In contact with the immobile water at a concentration ( $C_{im}$ ) in the matrix pores, diffusion occurs (i.e., assuming  $C > C_{im}$ ). On the contrary, when the peak in mobile water concentration C has passed, back diffusion can take place from the immobile into the mobile water (i.e., assuming  $C < C_{im}$ ).



Fig. 3 Schematic view of the highly mobile water in fractures or macro-pores and the quasi immobile water in the pores of the chalk.

This immobile water effect on the solute transport is often referred to as 'matrix diffusion' in
the literature (Rasmusen & Neretnieks 1981, Wood *et al.* 1990, Carrera *et al.* 1998). It was
detected by tracer tests showing longer tailing of the breakthrough curve (BTC). One of the
way to take this matrix diffusion into account is to express a classical linear exchange
relationship as (Coats & Smith 1964, Bear & Verruijt 1987, Brouyère 2001):

$$f_m^{im} = \alpha_d^m (\rho C - \rho_{im} C_{im})$$

(5)

where  $f_m^{im}$  is the diffusive solute mass flux (kg/m<sup>3</sup>s) from mobile to immobile water (or vice-264 versa),  $\rho$  and  $\rho_{im}$  are the density of mobile and immobile water respectively,  $\alpha_d^m$  is defined 265 as the matrix diffusion coefficient or immobile water diffusion coefficient (s<sup>-1</sup>), similar to a 266 diffusion coefficient  $(m^2/s)$  divided by a surface area  $(m^2)$  intended to describe the mobile 267 268 water - immobile water contact area. This flux could be added to the solute conservation equation expressed for the mobile water in the REV (equation 4). In practice, to obtain the net 269 270 gain or loss of solute mass in the mobile water at each time step (i.e., dual-domain mass 271 transfer), a solute mass balance equation in the immobile water must also be considered. Values for the matrix diffusion coefficient ( $\alpha_d^m$ ) are inferred from lab and field tracer tests 272 results and diffusion coefficient values. As studied by Brouyère (2001), this value can have a 273 274 crucial influence on the simulated BTC (Fig. 4).

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A few examples of BTC from tracer tests performed in the unconfined chalk aquifer of Hesbaye
(Geer basin, Belgium) show the wide diversity of the obtained BTC in function of the local
multi-scale heterogeneity of the chalk (Fig. 5).

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- 318 behaviour.
- 319 More details about the tracing conditions (Hallet 1998) and interpretation:
- 320 (a) 52 kg of potassium, injected (in depth-averaged conditions) during 2h at a distance of 93 m from the 321 pumping well with a discharge rate of 9 m<sup>3</sup>/h. Local hydraulic conductivity values were assessed 322 between 4.0 and 20.0  $10^{-4}$  m/s in the fractured zones and about 8.5  $10^{-5}$  m/s in the less fractured chalk 323 from pumping tests interpretation. The clear advection dominated solute behaviour with a moderate 324 tailing is due to preferential flow through open fractures or conduits. 325 (b) 22.9 kg of iodide, injected (in depth-averaged conditions) during 9h45 at a distance of 54 m from a 326 drainage gallery (BTC was measured in the gallery). Local hydraulic conductivity values were assessed 327 between 3.0 and 10.0  $10^{-4}$  m/s in the fractured zones and between 0.1 and 8.0  $10^{-5}$  m/s in the less 328 fractured chalk from pumping tests interpretation. The long tailing with relatively high concentrations 329 clearly shows a strong immobile water effect with occurrence of matrix diffusion and back-diffusion. 330 (c) 4.1 kg of lithium, injected (in depth-averaged conditions) during 30h at a distance of 110 m from the 331 pumping well with a discharge rate of 4.4 m<sup>3</sup>/h. Local hydraulic conductivity values are assessed 332 between 1.0 and 10.0  $10^{-3}$  m/s in the fractured zones and between 0.3 and 1.0  $10^{-5}$  m/s in the less 333 fractured chalk from pumping tests interpretation. The atypical BTC can be interpreted as showing a 334 limited first advection arrival at very short times corresponding to a preferential flowpath through a

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fractured zone detected previously by geophysics, and a quite delayed second peak due to the solute pathway through the less fractured chalk matrix.

#### 338 Heat transport in chalk

Heat can be transported through both the pore/fracture space and the solid matrix of the chalk,
whereas solutes in groundwater are transported only through the pores and fractures. The
main processes for heat transfer in a saturated chalk are thermal conduction, advection and
dispersion. The corresponding heat conservation equation can be written:

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$$\frac{\partial \rho_b c_b T}{\partial t} = -\nabla \cdot \left[ \rho_w c_w \boldsymbol{q} T - (\lambda_b + c_b \rho_b \boldsymbol{D}) \cdot \nabla T \right] + Q_T$$
(6)

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where  $\lambda_b$  is the heat (or thermal) conductivity (W/(m°K)) of the bulk saturated chalk,  $\rho_b$  is the bulk density (kg/m<sup>3</sup>),  $c_b$  is the bulk heat capacity (J/(kg°K)) ( $\rho_b c_b$  is the bulk volumetric heat capacity J/(m<sup>3</sup>°K)),  $\rho_w$  is the water density (kg/m<sup>3</sup>),  $c_w$  is the water heat capacity (J/(kg°K)) ( $\rho_w c_w$  is the water volumetric heat capacity in J/(m<sup>3</sup>°K)),  $\boldsymbol{q}$  is the total water flux vector (m/s) from Darcy's law and including the possible temperature effect on the water density and viscosity (natural convection),  $\boldsymbol{D}$  is the thermal dispersion tensor (m<sup>2</sup>/s),  $\nabla T$  the temperature gradient (°K/m),  $Q_T$  is the heat source (if  $Q_T > 0$ ) or sink (if  $Q_T < 0$ ) term.

In aquifers, most often, the thermal dispersion is considered as negligible compared to the thermal conduction term and compared to the advection-convection term (Anderson 2005, Irvine *et al.* 2015). Then, the equation is accordingly simplified and divided by the bulk volumetric heat capacity ( $\rho_b c_b$ ) to obtain (Dassargues 2018):

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$$\frac{\partial T}{\partial t} = -\nabla \cdot \left[ \frac{\rho_w c_w}{\rho_b c_b} \boldsymbol{q} T - \frac{\lambda_b}{\rho_b c_b} \nabla T \right] + Q_T / (\rho_b c_b)$$
(7)

where the coefficient  $\lambda_b/(\rho_b c_b)$  is named thermal diffusivity (m<sup>2</sup>/s) and the term  $Q_T/(\rho_b c_b)$ (°K/s) can also be understood as the temperature associated with a groundwater flux entering or exiting 1 m<sup>3</sup> of the domain

For heat transport the 'effective heat transport porosity' could probably be considered as equalto the 'effective solute transport porosity' in the advection term. On the contrary, the

365 'diffusive term' in the heat transport equation corresponds to conduction (as heat dispersion is

366 often considered as to be neglected with regards to conduction) that occurs in solid matrix and 367 both mobile and immobile water. So, the total porosity is required to assess the values of the

both mobile and minimobile water. So, the total porosity is required to assess the values of the bulk saturated chalk matrix heat capacity ( $c_h$ ) and density ( $\rho_h$ ) that are considered in the heat

transport equation. Conduction is actually boosted by the temperature difference between the

370 mobile water and the immobile water, and also between the mobile water and the solid matrix.

371 These effects are fully included in the diffusive term of the heat solute transport equation

372 (equation 7). This last term is most often three orders of magnitude higher than a solute

diffusion term in a solute transport equation. Thus, looking at equation (7), the immobile

water effect on the heat transfer is clearly integrated in the thermal diffusivity term describingconduction in the bulk saturated media.

376 Usual values for bulk thermal conductivities ( $\lambda_b$ ) are comprised between 0.5 and 2.5 (W m<sup>-</sup>

 $^{10}$ K<sup>-1</sup>) for a dry chalk and between 0.57 and 0.60 for water. Usual values for bulk heat

378 capacity ( $c_b$ ) and volumetric heat capacity ( $\rho_b c_b$ ) for a dry chalk are about 0.90 10<sup>3</sup> (J kg<sup>-1</sup>°K<sup>-1</sup>) K<sup>-1</sup>°K<sup>-1</sup>

- 379 <sup>1</sup>) and  $2.2 2.25 \ 10^6 \ (J \ m^{-3} \ K^{-1})$  respectively, and  $4.18 \ 10^3 \ (J \ kg^{-1} \ K^{-1})$  and  $4.18 \ 10^6 \ (J \ m^{-3} \ K^{-1})$
- 380 <sup>1</sup>) for water.

381 An example of a temperature BTC from a tracer test performed in a continuous chalk fracture 382 isolated between packers is shown in Fig. 6. The injection well (i.e. with the isolated fracture 383 between packers at 35 m of depth) is located at 7.55 meters from the pumping well in the 384 unconfined chalk of the Mons basin (Belgium) (more details in the Fig. 6 legend). It shows clearly heat advection and then the combined effect of the conduction and possibly matrix 385 386 diffusion in the delayed answer of heat evolution with time. As long as the injection of hot 387 water is in progress the curve rises showing essentially advection, but as soon as the injection 388 is stopped, apart from a small and temporary drop in temperature (due to the drop in 389 temperature in the fracture itself), a new but slow temperature rise is observed by conduction 390 and possibly matrix diffusion (Fig. 6). Interestingly, the finally measured temperature change in the recovery well is 0.44 °C, and thus already 10 % higher than the peak measured during 391 392 the active injection of hot water. Due to the stop of the hot water injection, but the ongoing 393 pumping in Pz1 during this period, heat stored in the rock matrix around the injection well 394 and around the fractures between the injection and pumping wells, is probably progressively 395 released from the rock matrix to the colder water circulating in the fractures towards the 396 pumping well. In contrast, the comparison with the solute (uranine) behavior is striking with a 397 very quick advection of solute, as expected, since the injection was performed in the fracture 398 between packers.





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412 Fig. 6 Observed concentration of uranine and temperature change at a recovery pumping well 7.2 m<sup>3</sup>/h) located 413 at a 7.55 m distance from the injection well. Both wells are crossing an identified sub-horizontal fracture 414 connecting them at a depth of 35 m. A 70 h injection of hot water ( $\Delta T = +40$  °C) was complemented by two 10 415 minutes uranine pulse injections within an inflatable double packer system isolating the sub-horizontal chalk 416 fracture of interest. The uranine tracer was injected simultaneously with the start of the hot water injection. 417 During the first period (i.e., continuous hot water injection period during the first 70 h), both uranine injections 418 show first arrival times of about 10 minutes at the recovery well. In contrast, the first measured temperature 419 increases of 0.01 °C is observed after 12.5 h. The followed observed heat increasing increments are always 0.01 420  $^{\circ}$ C and the time between two increments is getting shorter till the peak is reached. After 70 h of hot water 421 injection, the measured temperature change in the recovery well is 0.40 °C, far from having reached a steady-422 state temperature value. After the stop of hot water injection, the temperature decreases first rapidly to 0.33 °C, 423 before a further temperature rebound.

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# 425 **Conclusion and perspectives**

426 In chalk aquifers, it could be particularly recommended to distinguish between the different

- 427 kinds of porosity. A pumping test in an unconfined chalk aquifer in transient conditions
- 428 allows to deduce interpreted values for hydraulic conductivity and effective drainable porosity
- 429 (specific yield), while a tracer test (with an ideal solute tracer) would confirm that this latter is

- 430 not the adequate porosity to calculate advection groundwater velocity. In reality, a systematic
- 431 difference is observed between the portion of mobile water for flow calculation and the
- 432 portion of mobile water actually transporting the solute by advection. This is particularly true
- 433 in chalk aquifers showing a multi-scale heterogeneity with pores and various micro- and
- 434 macro-fractures.
- 435 For solute transport, the matrix diffusion in the immobile water should be accounted for to
- 436 obtain a reasonably realistic quantification and characterisation. For heat transport on the
- 437 contrary, possible matrix diffusion is masked and thus can be included in thermal conduction
- that is active through both the pore/fracture space and the solid matrix of the chalk.
- 439 Finally, determining the breakthrough curve of a solute contaminant at a given pumping well
- 440 (i.e., mainly influenced by advection, dispersion and matrix diffusion) is completely another
- 441 problem then assessing efficiency of an ATES geothermal system that will be mainly
- 442 dependent on advection and conduction). Each process should be investigated and calculated
- 443 with its corresponding specific behaviour in the multi-scale heterogeneity context of the
- 444 chalk. This is requiring a lot of characterisation work involving well designed in situ solute
- and heat tracer tests.

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# 455 Data Availability

The data set of the joint heat-solute experiment is stored on the H+ Network database (http://hplus.ore.fr/en/hoffmann-et-al-2020-hydrogeology-of-the-chalk-data). The access will be free 6 months after publication. The authors express their special thanks to N. Guihéneuf by assisting the data upload to the H+ database

assisting the data upload to the H+ database.

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