



Hydro-mechanical behaviour of a pellets based bentonite seal: Numerical modelling of lab scale experiments

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Outline:

- 1. Introduction;
- 2. Materials and method;
- 3. Experimental results;
- 4. Coupled hydro-mechanical model;
- 5. Numerical results and analysis;
- 6. Conclusions.





BENTONITE = clay material that primarily consists of montmorillonite:









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BENTONITE = clay material that primarily consists of montmorillonite:

• MECHANICS

HYDRAULICS

 significant swelling upon hydration (swelling deformation, swelling pressure dry density dependent);

very low permeability (~10⁻²⁰ - 10⁻²¹ m² in saturated conditions);







BENTONITE = clay material that primarily consists of montmorillonite:



CEA - BEACON - Deliverable D5.1.1



Seiphoori et Al. 2014









BENTONITE = clay material that primarily consists of montmorillonite:



2. Materials and method



Second layer with crushed pellets during installation





32 mm pellets and crushed pellets grain size distribution





3. Experimental results



Swelling pressure in radial and axial directions function of time



Swelling pressure in axial direction and water intake function of time.



Suction function of time



Swelling pressure in axial direction and suction measurement at z=100 mm from the bottom face function of time.

4. Coupled hydro-mechanical model



4.1 Mechanical model

Suction – Mechanics

Modified CamClay - Barcelona Basic Model

(Alonso et al 1990)

$$d\varepsilon_v^e = \frac{\kappa}{1+e} \frac{dp}{p} + \frac{\kappa_s}{1+e} \frac{ds}{s+u_{atm}}$$

 $\sigma = \sigma_{-} - \eta I$

Plastic yield surface





 $\lambda(s) = \lambda(0)[(1-r)\exp(-\omega s) + r]$

For constant volume conditions:

$$p(s) = p_A \left(\frac{s_A + u_{atm}}{s_B + u_{atm}}\right)^{\frac{\kappa_s}{\kappa}}$$

$$\kappa_s(p) = \kappa_{s0} * \exp\left(-\alpha_p * p\right)$$

4. Coupled hydro-mechanical model



4.2 Hydraulic model: Water retention behaviour-double-porosity structure (Dieudonne' 2016)



4. Coupled hydro-mechanical model



4.2 Hydraulic model: Permeability - double-porosity structure

$$K_w = K_{w0} \frac{e_M^{expn}}{(1 - e_M)^{expm}} \frac{(1 - e_{M0})^{expm}}{e_{M0}^{expn}}$$

Fo-Ca-clay powder and pellets



(Van Geet et al 2005) –X-ray tomography on pellet mixture during hydration test (Dry density)-

The permeability evolution affects the velocity of the swelling pressure development, but in pellet-mixture is a difficult process to evaluate.

5.1 Geometric configuration of the simulation

- Monodimensional problem;
- Homogeneous medium (same mechanical and hydraulic properties);
- Homogeneous initial state (suction=171 MPa and confining stress σ_a = 0.02 MPa and σ_r = 0.2 MPa).



Boundary condition of the model





5.2 Permeability evolution and water intake



Evolution through time of permeability over the height of the sample during water injection (numerical results)



Water mass injected from the bottom end. Comparison between experimental data and model predictions

$$K_{w} = K_{w0} \frac{e_{M}^{expn}}{(1 - e_{M})^{expm}} \frac{(1 - e_{M0})^{expm}}{e_{M0}^{expn}}$$





5.3 Swelling pressure



Swelling pressure in axial direction. Comparison between experimental data and model predictions



Swelling pressures in radial direction. Comparison between experimental data and model predictions





5.4 Post mortem analysis





Dry density distribution over the height of the sample at the end of the test. Comparison between experimental data and model predictions



Macrovoid ratio distribution over the height of the sample at the end of the test. Comparison between experimental data and model predictions

Water content distribution over the height of the sample at the end of the test. Comparison between experimental data and model predictions

6. Conclusions



- The analysed sample presented a prominent initial <u>heterogeneous pore structure</u> <u>distribution</u> which is <u>not considered</u> in the numerical strategy...
- Nevertheless the numerical model is able to <u>predict remarkably well</u> the experimental results in terms of:
- **<u>swelling pressure</u>** (especially its non-monotonic evolution);
- <u>water intake</u> (direct consequence of the selected permeability law evolution);
- <u>final dry density and water intake distribution.</u>



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References



1. A. C. Dieudonné, Hydromechanical behaviour of compacted bentonite: from micro-scale analysis to macro-scale modelling, (University of Liege, 2016)

- 2. Y. J. Cui, JRMGE 9, 565-574, (2017)
- 3. Q. Wang et al., Eng. Geol. 164, 67-76 (2013)
- 4. A. Molinero Guerra, Experimental and numerical characterizations of the hydro-mechanical behavior of a heterogeneous material: pellet/powder bentonite mixture. (Ecole des Ponts ParisTech, 2019)

5. F. Bernachy and W. Guillot, Bentonite mechanical evolution - Experimental work for the support of model development and validation, D.4.1/2, (BEACON project, 2019)

- 6. M. Van Geet, G. Volckaert, S. Roels, Appl. Clay Sci. 29, 73-87 (2005)
- 7. A. Molinero Guerra et al., Appl. Clay Sci. 135, 164-169 (2017)
- 8. C. Imbert and M. Villar, V. Appl. Clay Sci. 32, 197-209 (2006)

9. J. Talandier, Specifications for BEACON WP5: testing, verification and validation of models step 1- Verification cases,

D5.1.1. (BEACON project, 2018)

10. E. E. Alonso, A. Gens, A. Josa, Geotechnique 40, 405-430 (1990)

11. A. Dueck and U. Nilsson, Thermo-Hydro-Mechanical properties of MX-80 - Results from advanced laboratory tests, TR-10-55, (SKB, 2010)

12. A. C. Dieudonné et al., A water retention model for compacted clayey soils, Third International Symposium on

Computational Geomechanics, (Krakow, Polland, 2013)

- 13. E. Romero, Eng. Geol. 165, 3-19 (2013)
- 14. A. Seiphoori, A. Ferrari, L. Laloui, Géotechnique 64, 721-734 (2014)