

1 **Physico-chemical and mineralogical characterization of clay materials**
2 **suitable for production of stabilized compressed earth blocks**

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14 **Abstract**

15 The main objective of this study is to characterize the physico-chemical and mineral properties of
16 clay materials from Burkina Faso to produce stabilized compressed earth blocks (CEBs). The
17 reactivity of the clay materials was tested based on the electrical conductivity of solutions and the
18 compressive strength of CEBs stabilized with 0-20 wt% CCR (calcium carbide residue) and cured
19 for 45 days at 40±2 °C. Pabre and Kossodo respectively contain the highest fractions of clay (20-
20 30%) and gravel (40%). Saaba and Pabre contain the highest content of kaolinite (60-70%) and
21 quartz (45-60%) and recorded the highest and lowest reactivity, respectively. The compressive
22 strength of CEBs stabilized with 20% CCR improved tenfold (0.8 to 8.3 MPa) for Saaba and only

23 2.6 (2 to 7.1 MPa) for Pabre. The clay materials in the present study are suitable to produce CCR-
24 stabilized CEBs for load-bearing construction.

25 **Keywords:** Calcium carbide residue; clay material; compressed earth block; compressive strength;
26 kaolinite; reactivity.

27 **1. Introduction and background**

28 Clay materials for application in construction of modern buildings have been mostly studied for
29 production of fired products such as tiles and bricks [1-5]. The firing process is energy consuming
30 and therefore not environmental-friendly, resulting in relative increase of the cost and embodied
31 energy of the products [6-8]. Today, the awareness of the global warming and necessity for energy
32 saving on the one hand and improved indoor comfort provided by unfired clay material on the
33 other hand give this material the potential for construction of sustainable buildings [7,9-12]. For
34 centuries, unfired clay materials have been used in building construction, in various forms such as
35 adobe (sun dried brick), rammed earth and recently compressed earth block (CEBs), and proved
36 promising to provide affordable and comfortable housing [7-10].

37 Burkina Faso is experiencing drastic demographic increase mainly in Ouagadougou, the capital
38 city of 1.9 million inhabitants [13]. The valorization of local and non-conventional materials in
39 modern construction can potentially contribute to providing the shelter to this population.
40 Historically, the city of Ouagadougou is known to have been constructed using adobe and other
41 techniques based on unfired clays. Clay materials lost much interest and acceptance with the arrival
42 of so called “modern” materials such as cement and steel [14]. Clay materials from this region are
43 basically used for road pavement and in some cases for the artisanal production of adobe; although
44 some small enterprises such as Zi-Matérial and CC3D offer services of production,
45 commercialization and construction with CEBs in the major cities. In fact, current research on

46 earth for building construction in Burkina Faso mainly reports on adobes and only a limited
 47 number of studies are reported on CEBs [10, 11, 14, 24-26].

48 CEBs are the most accepted form of unfired clay materials for the construction of modern building
 49 [7,10,15,]. This is basically related to their low (ambient) temperature of curing and other
 50 advantages such as improved mechanical and hygrothermal performances and durability, regular
 51 shape and size which allow to achieve various architectural designs contrary to the adobe and
 52 cement blocks [10, 15, 16, 18]. Clay material used for production of CEBs should have a certain
 53 fraction of fine particles and plasticity for binding cohesion of coarser particles in order to ensure
 54 physical and mechanical stability of the matrix, i.e. 10-50% of clay particles and plasticity index
 55 of 10-30, depending on the presence, the type and the content of stabilizer [7,9,12,16]. Table 1
 56 presents the texture, plasticity, and mineralogy of earthen materials recommended for the
 57 production of stabilized CEBs. The CEBs should generally have dry compressive strength of at
 58 least 2 MPa for usage in non-load bearing walls, as prescribed by various standards and normative
 59 documents applicable in different parts of the world [15,18]. Nevertheless, the performances of
 60 the CEBs become critical when it comes to construction of load bearing walls and usage in water
 61 saturated conditions [7,12].

62 **Table 1.** Some recommendations for production of stabilized CEBs: texture, plasticity,
 63 mineralogy, type and content of stabilizer and physico-mechanical performances.

Parameters	Chemical stabilizers (mass percent)	
	Cement (5-10 %)*, a, c, f	Lime (6-12 %)*, a, c
Composition of the clay materials		
Clay particle (%) ^{c, f}	10-30	30-50
Plasticity index (%) ^c	10-20	20-30
Mineralogy ^{**} , c, e	Inactive	Active
Production and curing of stabilized CEBs		
Moisture of production (%) ^{***} , a, b	10-15	15-20
Curing time in ambient condition (days) ^a	28	>28 (up to many months)
Physico-mechanical performances of cured CEBs		
Bulk density (kg/m ³) ^b	1600-2200	1400-2000
Dry compressive strength (MPa) ^{b, c, d}	4-12	2-7

Structural categories: Compressive strength of ordinary CEBs used in dry environment (CEB O D) according to ARS 673:1996 & ARS 674:1996 ^g.

CEB O 1 D: ≥ 2 MPa for no load-bearing applications (single-storey building).

CEB O 2 D: ≥ 4 MPa for load-bearing applications in two-storey building.

CEB O 3 D: ≥ 6 MPa for load-bearing applications in three-storey building.

References | ^a [6-7], ^b [9,12], ^c [16], ^d [17], ^e [19], ^f [20], ^g [21], [22-23]

* Mass percentage of the dry mass of the clay materials.

** Inactive: no swelling minerals such as kaolinite, quartz.

** Active: swelling/ reactive minerals such as smectite (montmorillonite), poorly ordered kaolinite.

*** Mass percentage of the dry mass of clay material + stabilizer, at normal compaction pressure (1-2 MPa).

The moisture content of production of CEBs can widely vary around the optimum moisture content dependent on the type of the clay material and stabilizer, thus need to be determined accordingly.

ARS(O): African regional standard (organization).

64 Many efforts have been made to improve the physico-mechanical performances of CEBs in dry
65 and saturated conditions as well as their long term durability. This is mainly achieved by
66 stabilization with chemical binders such cement, lime, geopolymer as well as by-product materials
67 [6-7,15,18,24-26]. The stabilization of CEBs with by-product materials in partial or full
68 substitution to classic binders not only improves the technical performance but also can potentially
69 improve the sustainability and economic benefits of stabilized CEBs [8,24,27-28]. The
70 compressive strength of CEBs, considered as an appropriate indicator of the mechanical
71 performance of stabilized CEBs, should be at least 4 MPa and 2 MPa, respectively, in dry and
72 saturated conditions for usage in the construction of load-bearing walls [7,15,21].

73 Prior studies used cement (5-10%) and lime (6-12%) to stabilize sandy (10-30 % clay particles,
74 10-20 of PI) and clayey (30-50 % clay particles, 20-30 of PI) earthen materials for production of
75 CEBs, respectively [6,17,20,17,23]. The stabilization resulted in substantial improvement of the
76 compressive strength of CEBs in dry and saturated conditions (Table 1). The improvement of the
77 compressive strength was related to the formation of cementitious products such as calcium silicate
78 hydrates (CSH) and calcium aluminate hydrates (CAH), either from the hydration reaction of
79 cement or pozzolanic reaction between lime and aluminosilicate minerals such as kaolinite and
80 fine particles of quartz ($<50 \mu\text{m}$) in the clay material [6,19,29,30-31]. More recently, calcium
81 carbide residue (CCR), an industrial by-product rich in lime (Ca(OH)_2), showed the potential to

82 improve the compressive strength of CEBs through the chemical interactions with the clayey
83 earthen material [24,26]. Therefore the suitability of clay materials should be characterized prior
84 to the production of stabilized CEBs given that their performance is largely influenced by the
85 characteristics of the earthen material, type and content of stabilizer as well as the production and
86 curing process. Most of the studies focused on the textural and plasticity properties for selecting
87 the appropriate clay materials for production of stabilized CEBs. Nevertheless, the latter chemical
88 interactions prompted a further need for understanding the contribution of chemical/mineral
89 composition of the clay materials on the performance of stabilized CEBs.

90 The present study investigates the suitability of clay materials from four different sites for the
91 production of CCR-stabilized CEBs. This is achieved through the characterization of the physico-
92 chemical and mineral properties of the clay materials and their reactivity. The effect of the mineral
93 composition of the clay materials on their reactivity with CCR was evaluated based on the rate of
94 consumption of calcium ions. The bulk density and compressive strength of CCR-stabilized CEBs
95 were considered as the main criteria for selecting the most appropriate materials for construction
96 of at least two-storey buildings.

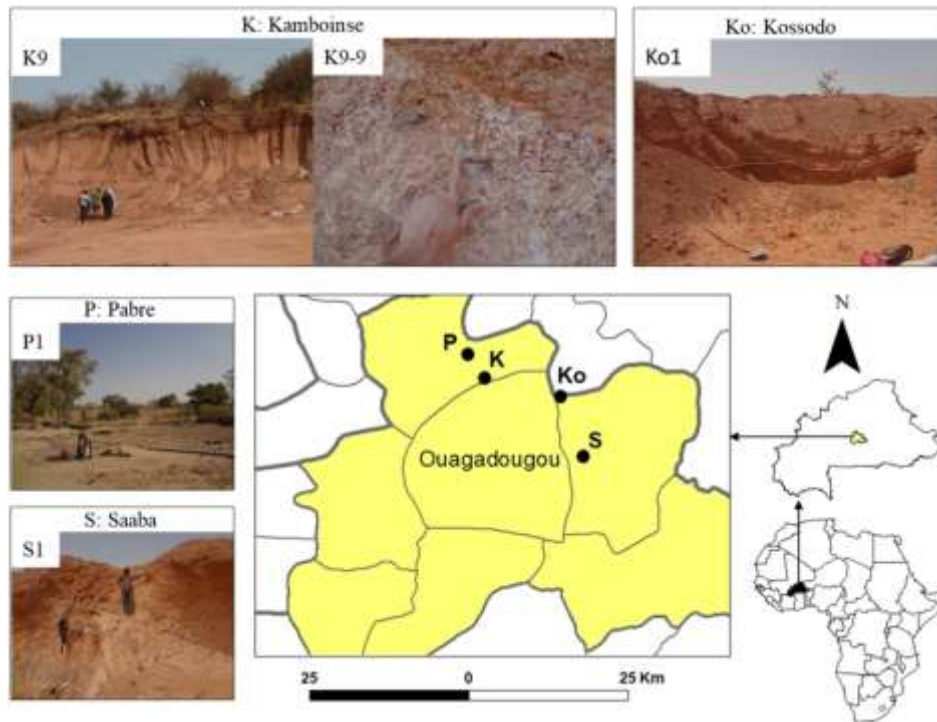
97 **2. Material and methods**

98 **2.1. Clay material**

99 The study was carried out on clay materials from four different sites in the vicinity of
100 Ouagadougou-Burkina Faso, i.e. Kamboinse, Pabre, Kossodo and Saaba (Fig. 1). The number of
101 samples collected from each site depended on its size, exploitability and apparent variability.

102 The site of Kamboinse (K) is located 15 km in the North of Ouagadougou (12°29'24.48" N;
103 1°32'59.28" W; altitude 326 m). It has an average area of 700x200 m² and average thickness of
104 exploitable layer of 6 m. The clay material from Kamboinse has a reddish-brown color, with some

105 whitish irregularities. The apparent texture of the particles varies depending on the depth: gravelly
106 on the surface layer (< 2 m of depth), clay in the middle (2-6 m) and sandy in the depth (> 6 m)
107 (Fig. 1). Pabre (P) is located 20 km in the North of Ouagadougou (12°31'23.46" N; 1°34'22.8" W;
108 Alt. 297 m). It has an average area of 600x100 m² and thickness of exploitable layer estimated at
109 2 m. The clay material from Pabre was collected 50 cm under the top layer and has a silty clay
110 texture (Fig. 1). This material has a grey color and is used for artisanal production of adobe bricks.
111 Kossodo (Ko) is located 10 km in the North-East of Ouagadougou (12°27'49.32" N; 1°26'34.02";
112 W Alt. 303 m) and extends over an average area of 350x150 m² and has an average thickness of
113 exploitable layer of 5 m. The clay material from Kossodo has a reddish-brown color, gravel-like
114 texture on the top (depth ~3 m), silty-sandy texture in the depth (Fig. 1). The clay material from
115 this site is used for production of cement stabilized CEBs by a local enterprise. Saaba (S) is located
116 15 km in the East of Ouagadougou (12°22'48.12" N; 1°24'2.22" W Alt. 310 m) and extends over
117 an average area of 350x70 m² and has an average thickness of exploitable layer of 4 m. It is
118 characterized by whitish a purple-brown clay material, which has sandy silt texture (Fig. 1). The
119 clay materials from Saaba are exploited by the local population for artisanal production of
120 construction materials and pottery.



121
 122 **Fig. 1.** Geographical setting of the Centre region of Burkina Faso and some views of the studied
 123 outcrops.

124 **Table 2** presents the quantity of exploitable clay materials given the total volume of the deposit
 125 estimated from average dimensions (area and depth) of the deposit and volume of material already
 126 excavated from the deposit. Kamboinse contains the largest quantity of exploitable clay materials
 127 (700 000 m³) while Saaba contains the smallest quantity (49 000 m³).

128 **Table 2.** Average resources and samples of the clay materials from the four investigated sites.

Sites	Average length (m)	Average width (m)	Average depth (m)	Total volume (m ³)	Excavated volume (m ³)	Exploitable volume (m ³)
Kamboinse (K)	700	200	6	840 000	140 000	700 000
Pabre (P)	600	100	2	120 000	-	120 000
Kossodo (K)	350	150	5	262 500	131 250	131 250
Saaba (S)	350	70	4	98 000	49 000	49 000
Samples	Geographical coordinates on the surface		Depth (m)	Sample description (Rock color chart)		
K1-3	12°29'24.48" N; 1°32'59.28" W; 326 m		3.5	Reddish brown (10R 4/6)		
K3-1	12°29'32.76" N; 1°32'55.62" W; 315 m		2.5	Reddish brown (10R 4/6)		
K3-2			3.5	Reddish brown (10R 4/6)		
K3-4			3.5	Whitish orange (10 YR 8/2)		
K3-5			5	Yellowish orange (10YR 8/6)		
K4-1	12°29'32.4" N ; 1°32'52.44" W; 313 m		6.5	Reddish grey (5YR 8/1)		
K4-2			6.5	Reddish brown (10R 4/6)		
K4-3			5.5	Reddish brown (10R 4/6)		
K4-4			7.5	Rose-grey (5YR 8/1)		
K5-1	12°29'23.46" N ; 1°32'58.8" W; 321 m		5	Reddish brown (10R 4/6)		
K5-2			4	Reddish brown (10R 4/6)		

K5-3		3.5	Greyish orange (10YR 7/4)
K5-4		6	Greyish orange (10YR 7/4)
K6	12°29'17.88"N; 1°32'59.28"W; 310 m	1	Greyish orange (10YR 7/4)
K8	12°29'15.54" N; 1°33'2.4"W; 318 m	4	Reddish brown (10R 4/6)
K9-1	12°29'23.46" N; 1°33'2.88"W; 328 m	1.5	Reddish brown (10R 4/6)
K9-2		3	Pale brown (5YR 6/4)
K9-3		3.5	Pale brown (5YR 6/4)
K9-4		4.5	Pale brown (5YR 6/4)
K9-5		5.5	Pale brown (5YR 6/4)
K9-6		6.5	Pale brown (5YR 6/4)
K9-7		7.5	Dark red-brown (10R 3/4)
K9-8		8.5	Pale orange (10 YR 8/2)
K9-9		9	Red (5R 5/4)
K9-10		3.5	Pale rose grey (5YR 8/1)
K9-11		5.5	Pale orange (10 YR 8/2)
P1	12°31'35.7" N; 1°34'31.5"W; 299 m	0.5	Yellowish brown (10YR 6/2)
P2	12°31'23.46" N; 1°34'22.8"W; 297 m	0.5	Yellowish brown (10YR 6/2)
P3	12°31'18.36" N; 1°34'18"W; 293 m	0.5	Yellowish brown (10YR 6/2)
Ko1-1	12°27'49.32" N; 1°26'34.02" W; 303 m	2.9	Brown gravel-like
Ko1-2		3.1	Pale brown silty clayey (5YR 5/6)
Ko1-3		3.9	Pale brown silty clayey (5YR 5/6)
Ko1-4		4.9	Dark yellowish orange silty clayey (10YR 6/6)
S1-1	12°22'44.88" N; 1°24'38.1" W; 315 m	2.3	Reddish brown (10R 4/6)
S1-2		3.9	Rose orange (10R 7/4)
S1-3		4	Pale red (5R 6/2)
S1-4		4.5	Rose grey (5YR 8/1)
S1-5		5	Pale red (5R 6/2)
S2-1	12°22'48.12" N; 1°24'2.22"W; 310 m	0.75	Greyish orange (10R 8/2)
S3-1	12°22'44.04" N; 1°24'34.08"W; 312 m	4	Greyish orange (10R 8/2)
S3-2		4.5	Reddish brown (10R 5/4)

129 2.2. Calcium carbide residue

130 Calcium carbide residue (CCR) is whitish-grey by-product material formed from the hydrolysis of
131 calcium carbide (CaC_2) during the production of acetylene gas. It was collected from Burkina
132 Industrial Gas (BIG) in Ouagadougou, ground and sieved to a particle size of 125 μm . Prior study
133 reported that the CCR contains up to 43 % lime ($\text{Ca}(\text{OH})_2$), determined by mean of X-ray powder
134 diffraction and thermogravimetric analysis, has a median particle size (D_{50}) of 20.5 μm and can
135 improve the compressive strength of CEBs [24].

136 2.3. Physico-chemical and mineral characterization of the clay material

137 Particle size distribution (PSD), clay: <0.002 mm, silt: 0.002-0.08 mm, sand: 0.08-2 mm and
138 gravel: >2 mm, was determined referring to NF EN ISO 17892-4 standards [32]. Atterberg's
139 plasticity limits (liquidity and plasticity) were determined using a Casagrande apparatus in

140 accordance with NF P 94-051 [33]. The specific density of particles was determined according to
141 NF EN 1097-6 [34]. The optimum water content required for achieving the maximum dry density
142 was determined by standard proctor test according to NF P 94-093 [35]. These physical and
143 geotechnical properties were characterized at the laboratory LEMHaD, 2iE, Burkina Faso.

144 Chemical composition was analyzed by means of X-Ray Fluorescence (XRF) technique using
145 ARL Perform'X Sequential XRF equipment (Department of geology, PGEP, ULiege, Belgium).
146 After determination of loss-on-ignition at 1000 °C for 2 hours (LOI 1000), about 340–450 mg (Ms)
147 of the sample were fused in lithium borate (11xMs) to form pellets. The major chemical elements
148 were determined on the pellets against an internal standard calibrated from sodium to uranium.

149 Mineral composition was analyzed by X-Ray diffraction (XRD) performed using the Bruker
150 D8-Advance Eco 1.5 kW diffractometer equipped with copper anticathode ($\text{Cu K}\alpha \lambda=1.54060 \text{ \AA}$
151 at 40 kV and 25 mA) and Lynxeye xe detector (Department of geology, AGEs, ULiege, Belgium).
152 The bulk mineralogy was identified by powder diffraction on samples ground to less than 150 μm ,
153 while the clay minerals were identified on oriented $<2 \mu\text{m}$ aggregates throughout treatments (air-
154 drying, glycolated and heat treatment). The XRD patterns were acquired from 2 to 70° 2 θ , at step
155 size of 0.02° 2 θ , scan time of 2 s per step for bulk samples and 2 to 30° 2 θ , step size of 0.009° 2 θ ,
156 scan time of 0.5 s per step for oriented aggregates. The acquired patterns were analyzed using
157 Diffrac.Eva V4.11 software of Bruker. The mineral composition was further confirmed by
158 thermogravimetric analysis (TGA) and Fourier Transform Infrared Spectroscopy (FTIR). The
159 TGA was carried out using automatic multiple sample thermogravimetric analyzer TGA-2000 of
160 the Las Navas Instruments (Department of Geology, AGEs, ULiège, Belgium). The analysis was
161 carried out in 25-1000 °C range at a heating rate of 5 °C/min in dry air environment. FTIR was
162 recorded using FTIR spectrometer in the range of 4000–400 cm^{-1} at scan size of 1 cm^{-1} (Department
163 of ArGenCo, UUE, ULiège, Belgium). The FTIR was carried out on pellets formed using 2 g of
164 the clay materials (ground to $<250 \mu\text{m}$) and 148 g of potassium bromide.

165 Semi-quantitative estimation of the mineral composition was achieved based on the measurement
 166 of the intensity of diagnostic peaks of minerals identified by XRD on which corrective factors
 167 were applied, i.e. “XRD: Method A”, according to [36-38]. The content of kaolinite and goethite
 168 were confirmed based on the mass loss on the TGA due to their respective dehydroxylation
 169 compared to that of pure kaolinite (13.97% at 530-590°C) and pure goethite (10.13% at
 170 290-330°C), i.e. “TGA: Method B” [2,39]. Additionally, the mineral composition was estimated
 171 based on chemical analysis by XRF and applying the formula proposed by [40] in [2]. This
 172 so-called “XRF: Method C” allows assigning different oxides to minerals identified by XRD and
 173 TGA. The following assignments were made: (1) K₂O to illite or K-feldspar, (2) CaO to
 174 plagioclase, (3) Al₂O₃ to kaolinite after subtracting the contribution to illite or K-feldspar, (4) SiO₂
 175 to quartz after subtracting the contribution to illite or K-feldspar and kaolinite, (5) Fe₂O₃ to
 176 hematite after subtracting the contribution to goethite (determined from the TGA). The values of
 177 the mineral composition obtained from “Method A” and “Method C” were normalized with respect
 178 to 100 %.

$$\% \text{illite} = \frac{\text{K}_2\text{O}}{94} 814 \quad \text{or} \quad \% \text{K-feldspar} = \frac{\text{K}_2\text{O}}{94} 557 \quad (1)$$

$$\% \text{plagioclase} = \frac{\text{CaO}}{56} 278 \quad (2)$$

$$\% \text{kaolinite} = \frac{\% \text{Al}_2\text{O}_3 - \frac{\% \text{illite} \times 102 \times 3}{814} - \frac{\% \text{K-feldspar} \times 102}{557} - \frac{\% \text{plagioclase} \times 102}{278}}{102} 258 \quad (3)$$

$$\% \text{quartz} = \frac{\% \text{SiO}_2 - \frac{\% \text{illite} \times 60 \times 6}{814} - \frac{\% \text{K-feldspar} \times 60 \times 6}{557} - \frac{\% \text{plagioclase} \times 60 \times 2}{278} - \frac{\% \text{kaolinite} \times 60 \times 2}{258}}{60} 60 \quad (4)$$

$$\% \text{hematite} = \frac{\% \text{Fe}_2\text{O}_3 - \frac{\% \text{goethite} \times 160}{178}}{160} 160 \quad (5)$$

814= molar mass of illite (K₂O)(SiO₂)₆(Al₂O₃)₃(H₂O)₃ in g/mol 94= molar mass of K₂O in g/mol
 557= molar mass of K-feldspar (K₂O)(SiO₂)₆(Al₂O₃) in g/mol 56= molar mass of CaO in g/mol
 278= molar mass of plagioclase (CaO)(SiO₂)₂(Al₂O₃) in g/mol 102= molar mass of Al₂O₃ in g/mol
 258= molar mass of kaolinite (SiO₂)₂(Al₂O₃)(H₂O)₂ in g/mol 60= molar mass of SiO₂ in g/mol

60= molar mass of quartz (SiO_2) in g/mol

160= molar mass of Fe_2O_3 in g/mol

178= molar mass of goethite $(\text{Fe}_2\text{O}_3)(\text{H}_2\text{O})$ in g/mol

179 **2.4. Tests of the suitability of clay material**

180 The suitability of different clay materials for the production of stabilized CEBs was tested based
181 on the rate of reactivity with the CCR and physico-mechanical performances of CEBs stabilized
182 with CCR. The evaluation of the reactivity aims to estimate the initial content of the CCR required
183 by the clay material for the pozzolanic reactivity to take place and monitor the rate of consumption
184 of that CCR over the curing time. It was assessed based on the evolution of the pH and electrical
185 conductivity (EC) in the mix solutions made of clay materials and CCR, referring to [26].

186 The representative clay materials from different sites, i.e. the mix of samples from different point
187 on each site, were washed on 400 μm to collect the undersize and dried at 40 ± 2 °C. The finer
188 fraction of clay materials, undersize on 400 μm , was considered for their binding and reactivity
189 contribution to the earthen materials contrary to the coarser fraction. Dry mixtures were prepared
190 using 5 g of dry clay material and 0, 10, 20 wt% CCR (mass percent of CCR with respect to the
191 dry mass of the clay materials). The dry mixtures were completed with 200 mL of distilled water
192 to prepare mix solutions. Mix solutions were cured for 1 to 45 days at 40 ± 2 °C in tightly closed
193 bottles to minimize carbonation. The initial pH and EC were measured after different curing times
194 (1, 14, 28 and 45 days) using pHenomenal MU 6100 L multiparameter lab conductimeter equipped
195 with CO 11 conductivity probe and pH electrode.

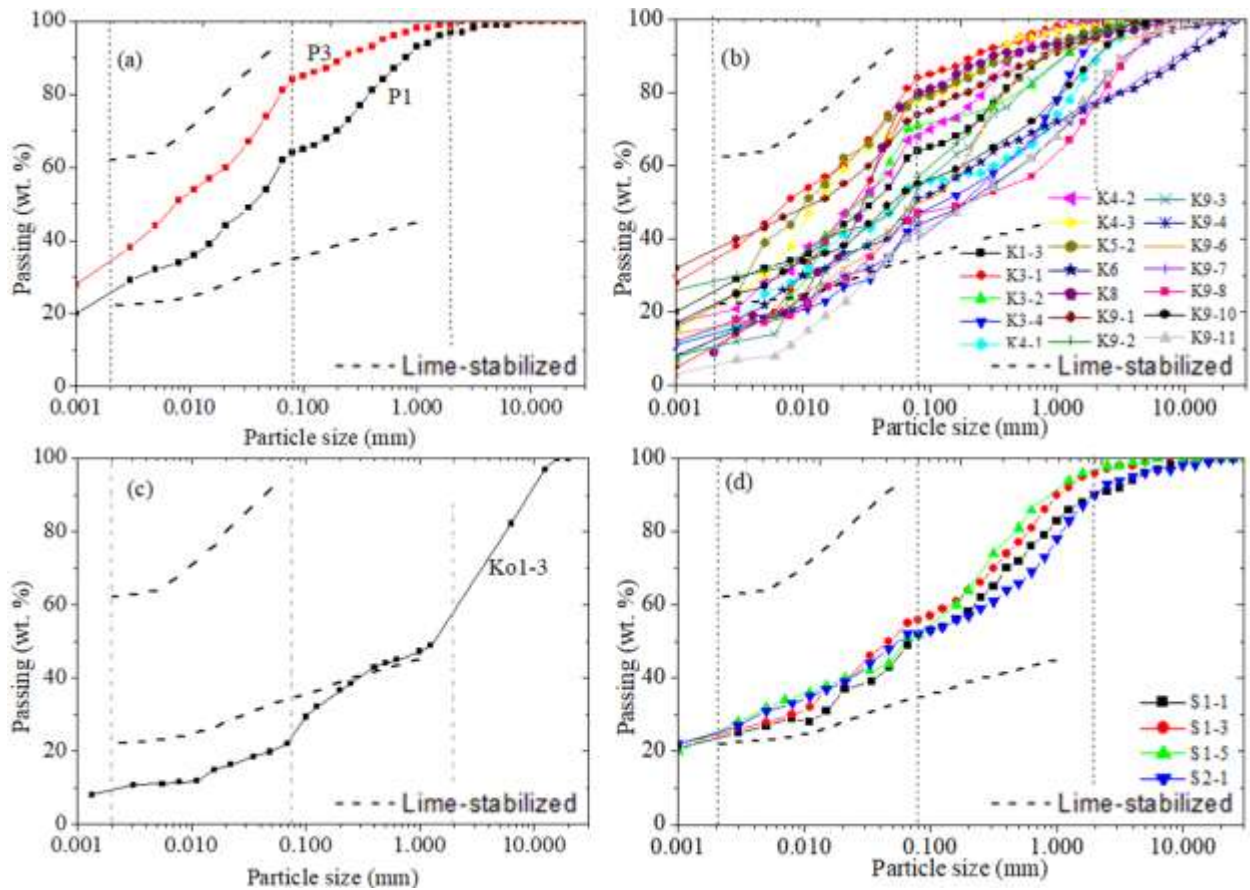
196 CEBs stabilized with CCR were produced using clay materials from different sites and 0, 10, 20%
197 CCR. The clay materials were sieved on 5 mm and dry mixed with CCR until apparent
198 homogeneity, as recommended by CRATerre [16]. The optimum amount of water, determined
199 according to the static compaction method proposed by CDE [41] was added to the dry mix and
200 mixed until homogeneous moisture distribution. An appropriate amount of humid mix was

201 manually compressed in a prismatic mold (295x140x95 mm³) of a Terstaram press machine to
202 mold the CEBs. The Terstaram machine allow to reach the compaction pressure up to 35 bars [25].
203 The CEBs were stored in polyethylene bags, to prevent the loss of humidity and eventual
204 carbonation, and cured for 45 days in an oven at 40±2 °C. After the curing, the CEBs were dried
205 at 40±2 °C until reaching constant mass (change of weigh less than 0.1 % for 24 hour). The
206 compressive strength was tested on stack of two halves of the CEBs according to the XP P13-901
207 standard [42].

208 **3. Results and discussion**

209 **3.1. Particle size distribution of the clay material**

210 The particle size distribution (PSD) curves of the clay materials from different sites are presented
211 in Fig. 2. These curves show continuous PSD for all samples, basically clay particles (<0.002 mm),
212 silt (0.002-0.08 mm), sand (0.08-2 mm) and gravel (> 2 mm). The shape of the PSD curve varies
213 for different sites and within the same site, for the case of Kamboinse. The clay materials from
214 Pabre (Fig. 2a) and Saaba (Fig. 2d) have PSD fitting in the boundaries recommended by the
215 CRATerre [16] for production of CEBs stabilized with lime. By contrast, Kossodo (Fig. 2c) does
216 not fit within these boundaries, as well as some points from Kamboinse (Fig. 2b). However, it is
217 noteworthy that these boundaries are just guidelines which do not have to be fully satisfied by the
218 earthen materials [16].



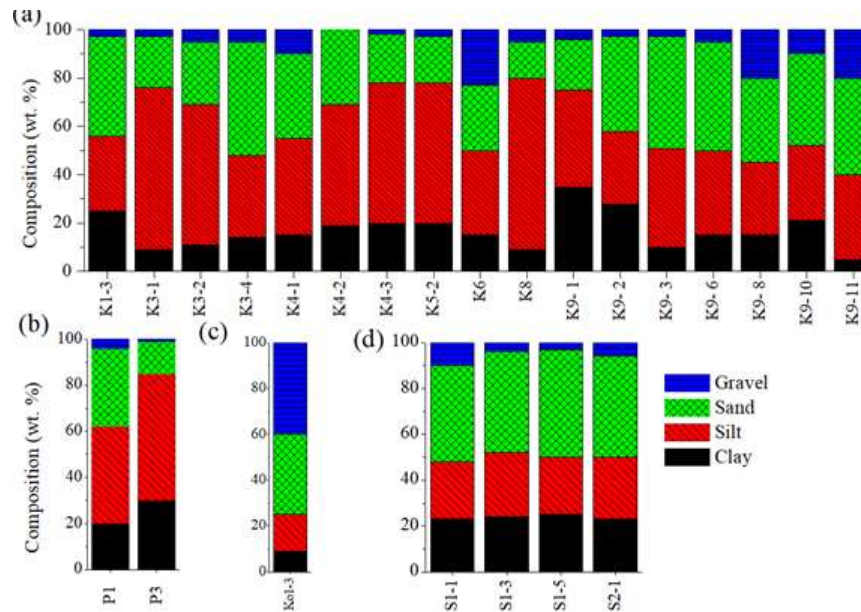
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220 Fig. 2. Particle size distribution of the bulk samples from the four investigated sites: (a) P=Pabre,
 221 (b) K=Kamboinse, (c) Ko=Kossodo, (d) S=Saaba. Comparison with CRATERre boundaries
 222 recommended for lime stabilized earthen materials [16].

223 The variations of PSD of the clay materials from Kamboinse range in 10-25 % of clay particles
 224 except K3-1, K8, K11 and K9-1, K9-2 which respectively contain <10% and >25% of clay
 225 particles, 30-60 % of silt, 15-45 % of sand and 0-20 % of gravel (Fig. 3a). The PSD of the clay
 226 materials from Pabre is characterized by 20-30 % of clay particles, 40-55 % of silt, 15-35% of
 227 sand and 0-5 % of gravel (Fig. 3b).The sample analyzed from Kossodo contained less than 10%
 228 of clay particles, 15 % of silt, 35 % of sand and 40 % of gravel (Fig. 3c). The materials from Saaba
 229 contained 20-25 % of clay, 25-30 % of silt, 40-45 % of sand and less than 10 % of gravel (Fig. 3d).

230 Samples from Pabre contain the highest fraction of clay particles, while Kamboinse, Saaba and
 231 Kossodo respectively contain the highest fractions of silt, sand and gravel particles. In term of total
 232 fraction of fine particles (<0.08 mm: clay + silt), Pabre contains the highest fraction (60-85 %)

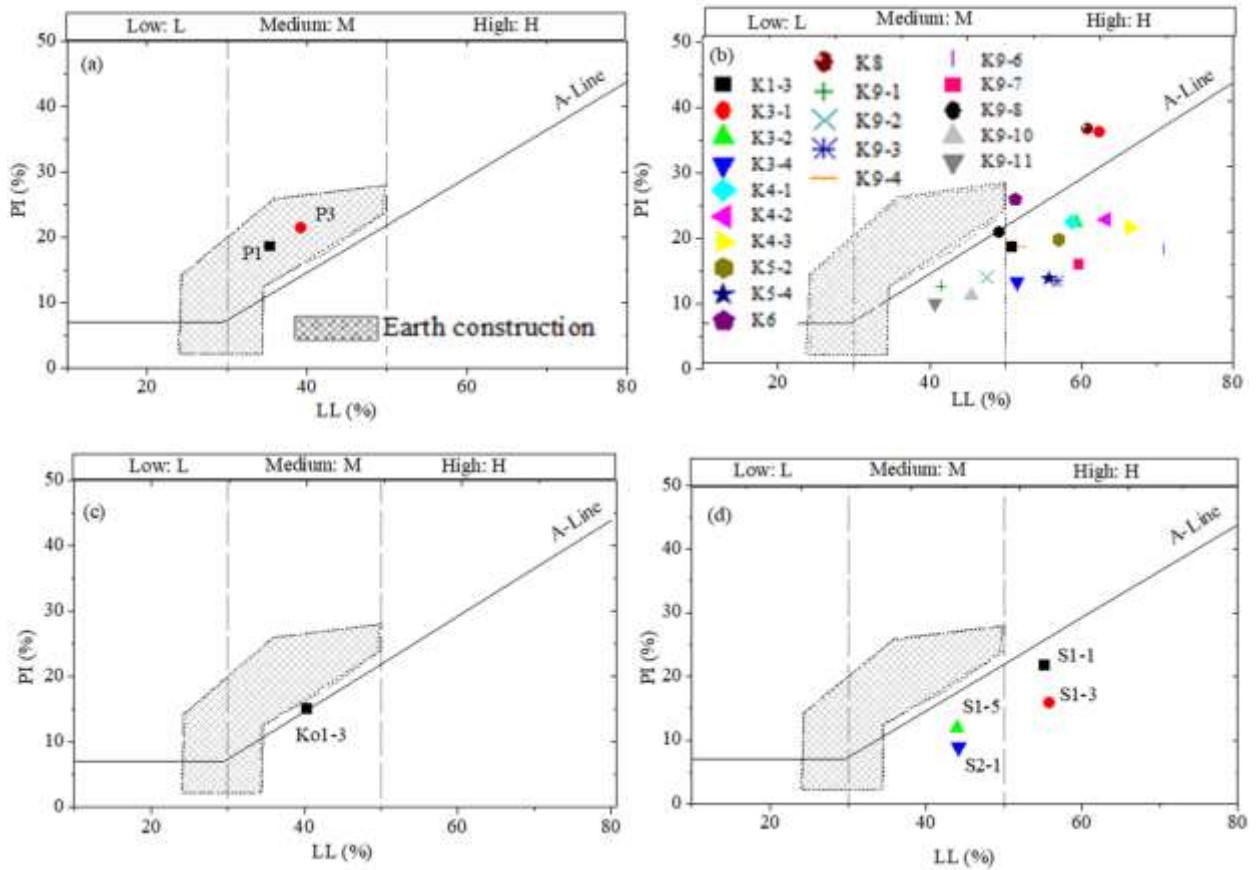
233 followed by Kamboinse (50-80 %), Saaba (~50 %) and Kossodo (~25 %). It is noteworthy that
 234 Saaba has relatively homogeneous PSD, while Kamboinse has the most heterogeneous PSD.



235
 236 Fig. 3. Fractions of particle size of the bulk samples from the four investigated sites:
 237 (a) K=Kamboinse, (b) P=Pabre, (c) Ko=Kossodo, and (d) S=Saaba.

238 **3.2. Plasticity of the clay materials**

239 Fig. 4 presents the relationship between plasticity index (PI) and limit of liquidity (LL) of all
 240 samples. The plasticity behaviors vary from medium to high plastic silt (under the A-line) and
 241 medium to high plastic clay (above the A-line). The clay materials from Pabre are characterized
 242 by a PI of 20 and LL of 35-40 (Fig. 4a). The PI and LL of the clay materials from Kamboinse
 243 range in 10-25 and 40-65, respectively, except K3-1 and K8 (PI: 35, LL: 60) (Fig. 4b). The sample
 244 from Kossodo has a PI of 15 and LL of 40 (Fig. 4c). The clay materials from Saaba have a PI of
 245 10-20 and LL of 45-55 (Fig. 4d). These results suggest that the clay materials from Pabre behave
 246 as clay of medium plasticity; those from Kamboinse and Saaba as silt-clay and silt of medium to
 247 high plasticity, respectively; Kossodo as silt-clay of medium plasticity.



248

249 Fig. 4. Plasticity of the bulk samples from the four investigated sites: (a) P=Pabre, (b)
 250 K=Kamboinse, (c) Ko=Kossodo, and (d) S=Saaba. Comparison with the boundaries recommended
 251 for earthen construction [42].

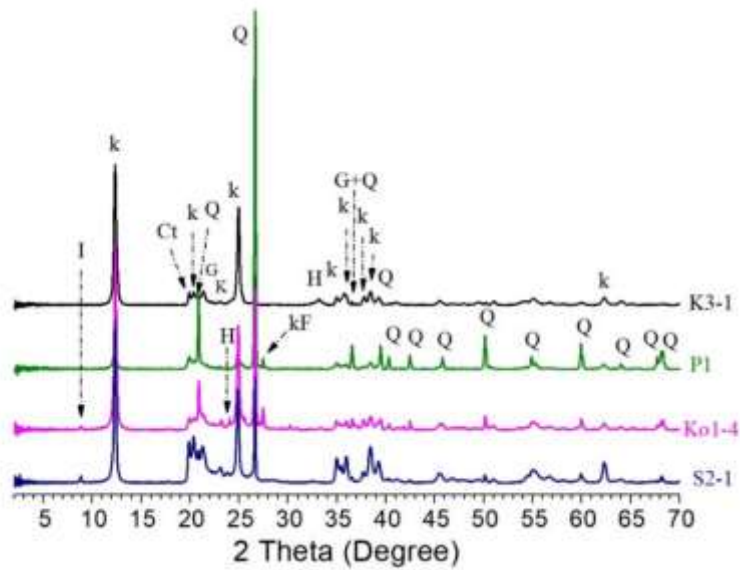
252 The clayey behavior of the materials from Pabre ($PI \geq 20$, $LL > 30$) agrees with their high content
 253 of the fine particles (60-85% of clay + silt) and give them high cohesion. Similarly, clay materials
 254 from Kamboinse have medium to high cohesion given their silt-clay behavior (PI of more or less
 255 than 20 and $LL > 30$) and high fraction of fine particles (50-80%). Kossodo and Saaba, both having
 256 $PI < 20$ and small fraction of fine particles ($< 50\%$) have medium cohesion [16]. Moreover, clay
 257 materials from Pabre and Kossodo have plasticity close to that recommended for earth construction
 258 [42], thus they would be more suitable for production of unstabilized CEBs. Clay materials with
 259 high cohesion and continuous PSD would be suitable for production of CEBs.

260 3.3. Chemical composition of the clay materials

261 The chemical composition of selected samples from different sites mainly consists of silica (SiO_2),
262 alumina (Al_2O_3), iron (III) oxide (Fe_2O_3) and traces of CaO, MgO, Na_2O , K_2O (Table 3). The
263 samples from Kamboinse, Kossodo and Saaba basically contain 40-60 % of silica, 20-30 % of
264 alumina and 0-15 % of iron (III) oxide. The sample from Pabre (P1) contains the highest fraction
265 of silica (~75 %) and the lowest fraction of alumina (~11 %) compared to other sites. The contents
266 of oxides in samples from Kamboinse, Kossodo and Saaba are consistent with the chemical
267 composition of kaolinite-rich clay minerals [5]. The occurrence of CaO, MgO, Na_2O , K_2O in trace
268 reflects the unlikely presence of high swelling clay minerals such smectite [43].

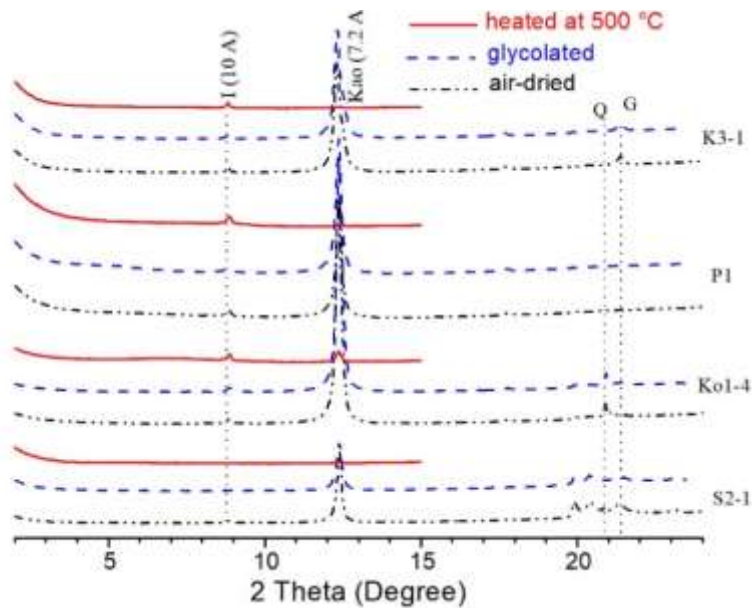
269 The content of iron (III) oxide (0-15 %) can be related to the increasing reddish brown color which
270 may suggest the presence of goethite. The ratio of $\text{SiO}_2/\text{Al}_2\text{O}_3$ generally varied between 2 to 3,
271 apart from exceptional samples such as P1 (6.6), K3-1 (1.6) (Table 3). This ratio reveals the
272 difference in mineral content, where the highest ratio can be related to the highest content of quartz
273 and *vice versa* [2]. For most samples, the range of variation of the LOI 1000 between 9-12 % (at
274 the exception of P1, LOI 1000 = 5.7 %) reveals the presence of mainly hydrated minerals such as
275 clay and goethite [2]. High ratio of $\text{SiO}_2/\text{Al}_2\text{O}_3$ (>3) and low LOI 1000 (5.7 %) in the sample from
276 Pabre (P1) suggests an important contribution of quartz and low content of clay minerals [2,4].
277 Moreover, Table 3 shows that the sum of SiO_2 , Al_2O_3 , and Fe_2O_3 is greater than 75 % which agrees
278 with some recommendations for stabilized CEBs [12].

279



280 Fig. 5. XRD patterns of selected bulk samples from the four investigated sites. K=Kamboinse,
281 P=Pabre, Ko=Kossodo, S=Saaba; Ct= total clay, k= kaolinite, I= Illite, Q= quartz, kF=K-feldspar.

282

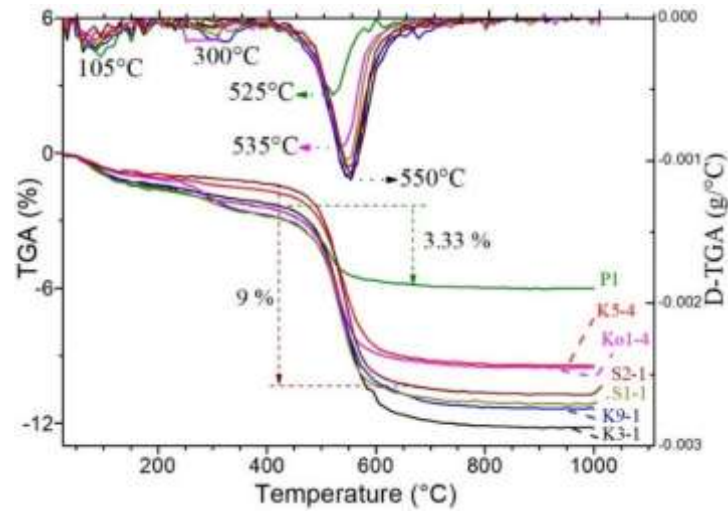


283 Fig. 6. XRD patterns of selected oriented aggregates from the four investigated sites:
284 K=Kamboinse, P=Pabre, Ko=Kossodo, S=Saaba; kao= kaolinite, I= illite, Q= quartz,
285 G= goethite.

286 3.4. Mineral composition of the clay materials

287 The XRD spectra of bulk samples selected from the four investigated sites mainly revealed the
288 diagnostic reflection of clay mineral (4.47 Å), kaolinite (7.15 Å), quartz (3.34 Å), goethite
289 (2.44 Å), hematite (2.69 Å) and traces of K-feldspars and illite (Fig. 5). The spectra of the oriented

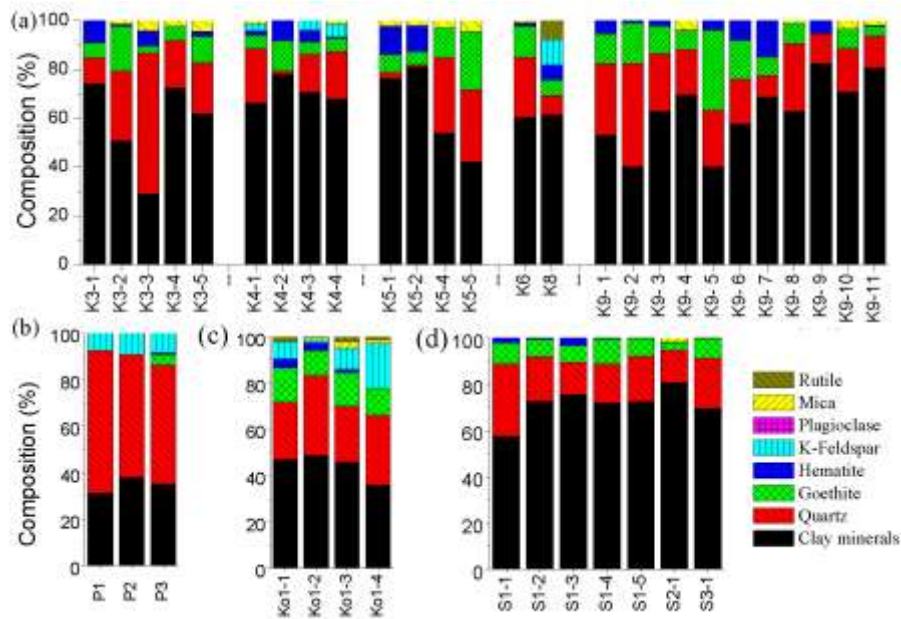
290 aggregates confirmed the predominance of kaolinite and some traces of illite (Fig. 6). The
 291 reflection of kaolinite clay appeared at 7.15 Å on air-dried and glycolated samples and disappears
 292 on heating at 500 °C for 4 hours, while that of illite appeared at 10 Å and remained throughout all
 293 treatments [38].



294

295 Fig. 7. Thermogravimetric analysis (TGA) and derivative thermogravimetric analysis (D-TGA)
 296 curves of bulk samples selected from the four investigated sites. K=Kamboinse, P=Pabre,
 297 Ko=Kossodo, S=Saaba.

298 Fig. 7 presents the thermogravimetric analysis (TGA) of samples selected from different sites. The
 299 samples recorded a loss of mass around 550 °C (i.e., between 450 and 650 °C) corresponding to
 300 the dehydroxylation of kaolinite [39]. At this temperature range, sample S2-1 recorded the highest
 301 loss of mass (9 %), while P1 recorded the lowest loss of mass (3.33 %). This confirms that S2-1
 302 and P1 contain the highest and lowest fraction of kaolinite, respectively. The thermal removing of
 303 hydroxyl groups in kaolinite was previously reported around 530-590 °C, depending on the degree
 304 of crystallinity [2,39,43]. Early onset of the dehydroxylation can be related to poorly ordered
 305 structure of kaolinite [43]. The TGA/D-TGA displays the peak of dehydroxylation of kaolinite in
 306 samples P1 and Ko1-4 at lower temperature (525-535 °C) compared to other samples (550 °C)
 307 (Fig. 7). This suggests that the sample from Pabre not only contains the lowest content of kaolinite
 308 but also, similarly to Kossodo, this kaolinite is relatively poorly ordered.



309

310 Fig. 8. Semi quantitative analysis of the bulk samples from the investigated sites estimated by
 311 XRD powder diffraction (i.e. method A). K=Kamboinse, P=Pabre, Ko=Kossodo, S=Saaba.

312 The semi-quantitative estimation of mineral composition of the bulk samples confirmed the
 313 predominance of clay minerals, quartz with minor goethite, hematite and K-feldspar and trace of
 314 mica, plagioclase and rutile (Fig. 8). The mineral composition widely varies from one site to
 315 another. The clay materials from Kamboinse contain 40-75 % of clay minerals, 5-40 % of quartz,
 316 0-20 % of goethite, <10 % of hematite (Fig. 8a). Pabre contains 30-35 % of clay mineral, 45-60 %
 317 of quartz, <5 % of goethite, <5 % of hematite, and <10 % of K-feldspar (Fig. 8b). Kossodo is
 318 characterized by 35-50 % of clay mineral, 25-35 % of quartz <15 % of goethite, <5 % of hematite
 319 and <20 % of K-feldspar (Fig. 8c). Saaba contains 60-80 % of clay mineral, 15-30 % of quartz,
 320 <10 % of goethite and <5 % of hematite (Fig. 8d). Saaba is characterized by the highest content of
 321 clay mineral (60-80 %) and lowest content of quartz (15-30 %). By contrast, Pabre contains the
 322 lowest content of clay mineral (30-35 %) and highest fraction of quartz (40-60 %). Kamboinse and
 323 Kossodo have medium composition of clay mineral. The mineral composition of Kamboinse is the
 324 most heterogeneous among the investigated sites.

325

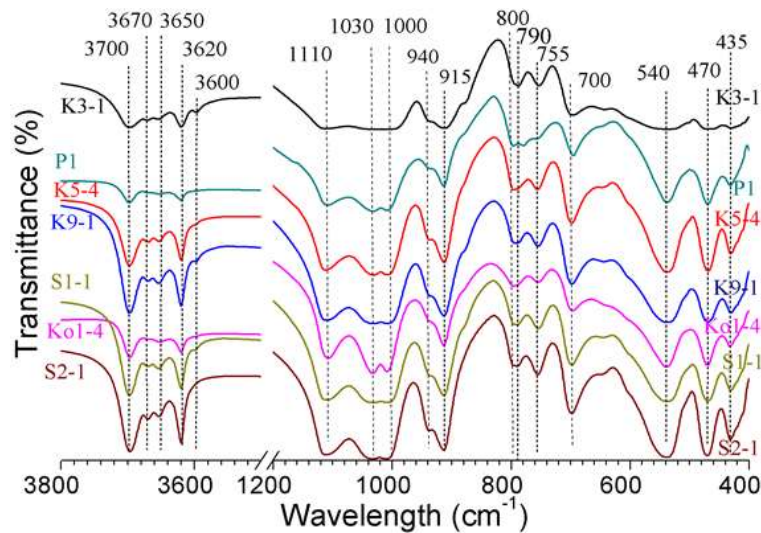
326 **Table 3.** Chemical and mineral composition of selected samples from the investigated sites.

Chemical composition of selected samples (%)									
Oxides	Kamboinse			Pabre	Kossodo	Saaba			
	K3-1	K5-4	K9-1	P1	Ko-4	S1-1	S2-1		
SiO ₂	42.9	63.7	49.7	76.9	55.0	53.5	60.9		
Al ₂ O ₃	27.3	23.3	24.0	11.7	23.4	24.7	27.6		
Fe ₂ O ₃	15.7	2.3	13.0	3.8	8.1	9.7	0.6		
CaO	0.1	0.1	0.6	0.2	0.1	0.1	0.1		
TiO ₂	1.4	0.5	0.9	0.9	1.2	0.9	0.0		
MnO	0.1	0.0	0.0	0.1	0.0	0.0	0.0		
MgO	0.1	0.0	0.1	0.0	0.0	0.0	0.0		
Na ₂ O	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
K ₂ O	0.1	0.3	0.2	0.8	2.3	0.1	0.1		
P ₂ O ₅	0.1	0.0	0.1	0.0	0.1	0.0	0.0		
Total LOI (1000 °C for 2 hours)	12.1	9.7	11.3	5.7	9.8	10.9	10.6		
Total oxides	100	100	100	100	100	100	100		
SiO ₂ /Al ₂ O ₃ ratio	1.6	2.7	2.1	6.6	2.4	2.2	2.2		
SiO ₂ + Al ₂ O ₃ + Fe ₂ O ₃	85.9	89.3	86.8	92.4	86.5	87.9	89.2		
TGA mass loss: 200-400°C	-0.8	-0.6	-1.3	-0.8	-1.3	-1.3	-0.4		
TGA mass loss: 450-650°C	-9.4	-7.3	-7.9	-3.4	-6.8	-7.9	-8.9		
Mineral composition of selected samples (%)									
Methods	Minerals	K3-1	K5-4	K9-1	P1	Ko1-4	S1-1	S2-1	
A: XRD (powder & oriented aggregate)	Quartz	11	31	29	61	30	31	14	
	Goethite	6	12	12	0	12	9	3	
	Hematite	9	0	5	0	0	2	0	
	K-feldspar	0	0	0	8	19	0	0	
	Plagioclase	0	0	0	0	0	0	0	
	Mica	0	3	0	0	2	0	2	
	Rutile	0	0	0	0	1	0	0	
	Total clay minerals	74	54	54	31	36	58	81	
	Clay minerals								
		Kaolinite	69	47	53	29	35	58	78
	Illite	5	7	1	2	1	0	3	
	Total: Method A	100	100	100	100	100	100	100	
B: TGA	Goethite (200-400°C)	8	6	13	8	12	12	4	
	Kaolinite (450-650°C)	67	53	57	24	49	56	64	
C: XRF (chemical analysis)	Illite	1	3	2			1	1	
	K-feldspar				5	13			
	Plagioclase	1	1	3	1	1			
	Kaolinite	70	55	58	26	52	61	67	
	Quartz	11	35	22	60	22	25	28	
	Goethite	8	6	13	8	12	12	4	
	Hematite	9		2		1	1		
	Total: Method C	100	100	100	100	100	100	100	

327 Table 3 comparatively summarizes the chemical and mineral composition of selected samples
328 from different sites using different methods. It shows that kaolinite is the main clay mineral
329 accompanied by traces of illite ($\pm 5\%$). The kaolinite content determined from TGA (Method B)
330 is in good agreement (within $\pm 5\%$) with XRF (Method C), while the content estimated by XRD
331 (Method A) differs up to $\pm 10\%$. Exceptionally for the sample Ko1-4 from Kossodo, the content
332 of kaolinite significantly differs for XRD (35%) and TGA/XRF methods (49/52%). This

333 difference can be related to the possible disorder in the structure of kaolinite from Ko1-4, as
334 suggested from the TGA, whereby the fraction of disordered kaolinite is not detected by XRD.

335



336

337 Fig. 9. FTIR spectra of bulk samples of clay materials selected from the investigated sites.
338 K=Kamboinse, P=Pabre, Ko=Kossodo, S=Saaba.

339 The FTIR confirmed the predominance of kaolinite and quartz mineral (Fig. 9). Intense
340 transmittance bands detected at 3700, 3670, 3652 and 3620 cm^{-1} are related to the stretching of
341 hydroxyl groups in the kaolinite, while the mild band at 3600 cm^{-1} is related to the illite [45].
342 Bands at 1110, 1030 and 1000 cm^{-1} correspond to the vibration of Si-O bonds, those at 940 and
343 915 cm^{-1} to vibration of Al-OH bonds in the kaolinite as already observed by [5,43,45]. The bands
344 at 790 and 755 cm^{-1} correspond to the vibration of Si-O-Al while those around 540 cm^{-1} and 470
345 cm^{-1} respectively correspond to the stretching of Si-O-Al and Si-O bonds of the kaolinite [45].

346 The bands in 3700-3620 cm^{-1} are useful for qualitative assessment of the structure of kaolinite in
347 a sense that well-defined bands represent well-ordered kaolinite and the disappearance of the band
348 at 3670 cm^{-1} represents poorly ordered kaolinite [5,43,45-46]. Samples K9-1 and K5-4 (from
349 Kamboinse), S1-1 and S2-1 (from Saaba) present intense and well-defined diagnostic bands of
350 kaolinite (3700-3620 cm^{-1}) while K3-1, P1 and Ko1-4 (from Kamboinse, Pabre and Kossodo,

351 respectively) recorded low intensities. The contents of kaolinite in samples K3-1, P1, and Ko1-4
352 estimated from the TGA are respectively 67 %, 24 % and 49 % (Table 3). This suggests that the
353 disappearance of the bands at 3650 and 3670 cm^{-1} can be related not only to the low content of the
354 kaolinite in P1 but also to the poor order of its structure in K3-1, P1 and Ko1-4. This is in agreement
355 with the early onset of the dehydroxylation in P1 and Ko1-4 observed during TGA/D-TGA. The
356 mild intensity of the vibration band at 940 cm^{-1} for K3-1 and Ko1-4 samples is also consistent with
357 a poorly ordered clay mineral [5,43]. Additionally, the lack of well resolved bands at
358 1110-1000 cm^{-1} and 540 cm^{-1} in sample K3-1 suggests the presence of defects in Si-O and Si-O-
359 Al bonds of the kaolinite. The occurrence of poorly ordered kaolinite would be beneficial for its
360 reactivity during the pozzolanic reaction with lime [6,43-44,46].

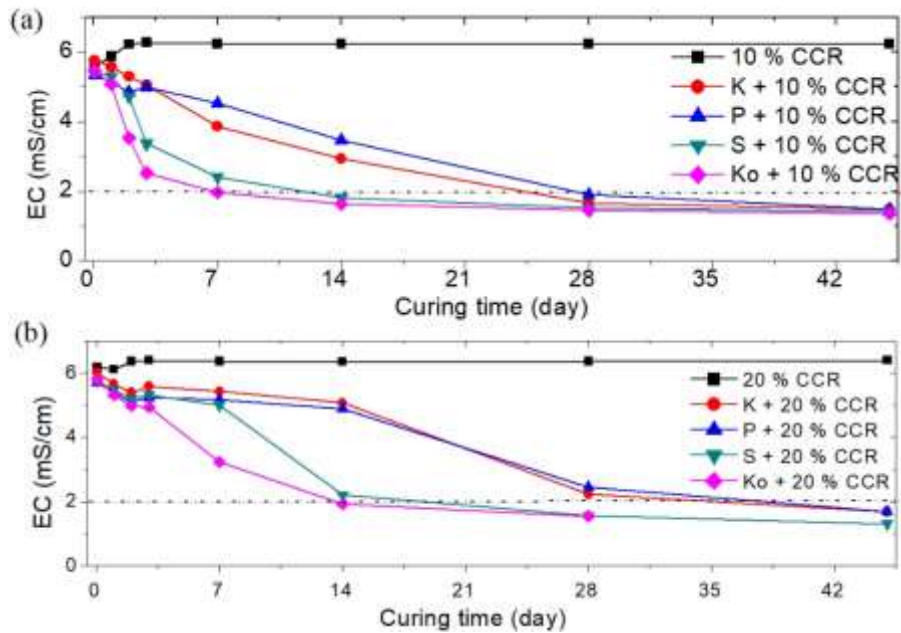
361 **3.5. Suitability of clay materials for production of stabilized compressed earth blocks**

362 **3.5.1. Initial pH and evolution of electrical conductivity of the mix solutions**

363 The initial pH of the solutions of clay materials were 8.5 for Kamboinse, 6.1 for Pabre, 5.2 for
364 Kossodo, and 5.5 for Saaba. The pH reached the apparent maximum values of 11.7 for Kamboinse,
365 11.4 for Pabre, 11.3 for Kossodo, and 11.4 for Saaba after 1 hour of addition of 2 % CCR compared
366 to the pH of 11.6 for the solution of 2 % CCR alone. This suggests that at least 2 % CCR is
367 necessary to satisfy the short term modification and increase the pH of all clay materials [26]. This
368 is related to the predominance of low activity clay mineral, such as kaolinite, in all clay materials.
369 Therefore, more than 2 % CCR would be necessary for triggering and sustaining the long term
370 pozzolanic reaction in all clay materials [47].

371 Fig. 10 shows the evolution of the electrical conductivity (EC) of the mix solutions containing the
372 clay materials and 10-20 % CCR over the curing time. Note that the EC of the solution of the clay
373 materials alone remained less than 0.1 mS/cm over time, while that of the solutions of CCR alone
374 was greater than 6.3 mS/cm. Fig. 10a and Fig. 10b show that the initial EC (after 1 hour) of the

375 clay materials mixed with 10 % and 20 % CCR respectively reached at least 5.8 mS/cm and
 376 6.0 mS/cm. The dissolution of portlandite ($\text{Ca}(\text{OH})_2$) from CCR into Ca^{2+} and OH^- was responsible
 377 for this drastic increase of the initial pH and EC [26,47].

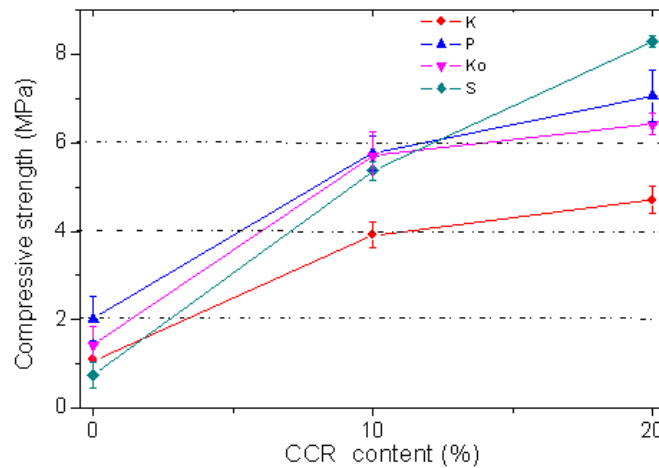


378
 379 Fig. 10. Evolution of electrical conductivity (EC) in mix solutions containing the clay materials
 380 and (a) 10 % (b) 20 % CCR at 40 ± 2 °C: K= Kamboinse, P= Pabre, Ko= Kossodo, S= Saaba.
 381 The EC of all mixtures decreased over the curing time, at similar rate in the first 3 days, after which
 382 a clear difference was recorded for different clay materials and CCR contents (Fig. 10). For
 383 mixtures containing 10 % CCR, the clay materials from Saaba (S) and Kossodo (Ko) recorded the
 384 highest rate of decrease of EC compared to Kamboinse (K) and Pabre (P) between the 3rd and 14th
 385 day. This resulted in reaching the apparent minimum EC of about 2 mS/cm after 14 days of curing
 386 for Kossodo and Saaba and 28 days for Kamboinse and Pabre (Fig. 10a). For mixtures containing
 387 20 % CCR, similar evolution of the EC was observed, the EC of Kamboinse and Pabre delayed
 388 until the 14th day before starting to decrease and reaching the apparent minimum value of about
 389 2 mS/cm after 28 days (Fig. 10b). For both CCR contents, the EC can be considered to reach the
 390 minimum value after 28 days for all clay materials, given that no substantial decrease was recorded
 391 between the 28th and 45th day.

392 The decrease of the EC is explained by the consumption of Ca^{2+} and OH^- during the pozzolanic
393 reaction of CCR with the clay materials [26,47]. In fact, linear correlation between the evolution
394 of the EC and concentration of Ca^{2+} in mix solution of clay materials and CCR was previously
395 reported [26]. The minimum EC in mix solutions can be related to the end of the consumption of
396 Ca^{2+} and the occurrence of optimum maturity of the pozzolanic reaction. Thus clay materials from
397 Kossodo and Saaba have better reactivity, given their early and high rate of consumption of Ca^{2+} ,
398 than the materials from Kamboinse and Pabre. The reactivity of clay materials from Saaba and
399 Kossodo can respectively be related to their relatively high content and poorly ordered kaolinite
400 as shown by XRD, TGA and FTIR (Section 3.4). The pozzolanic reaction involves aluminosilicate
401 minerals such as kaolinite and eventually fine quartz from the clay materials and lime ($\text{Ca}(\text{OH})_2$)
402 from the CCR [19,26,29-30]. This is highly dependent on the particle fineness and content of
403 reactive constituents and degree of order in their structure, with finer and poorly ordered materials
404 presenting better reactivity [19,46].

405 **3.5.2. Physico-mechanical performances of stabilized compressed earth blocks**

406 Fig.11 presents the evolution of the average compressive strength of CEBs produced from clay
407 materials from different sites stabilized with 0, 10, 20 % CCR. The average compressive strength
408 of CEBs containing 0 % CCR (unstabilized/control) was 1.1 MPa for Kamboinse, 2 MPa for
409 Pabre, 1.4 MPa for Kossodo and 0.8 MPa for Saaba (Table 4). The compressive strength of
410 unstabilized CEBs is basically related to the cohesion behavior as well as the granular distribution
411 of the clay materials. In fact, Pabre, which was characterized as highly cohesive material (Section
412 3.2), recorded the highest average compressive strength (2 MPa) compared to other sites which
413 have medium cohesion. This suggests that only unstabilized clay material from Pabre are likely to
414 produce ordinary CEBs for construction of single-storey (no load-bearing) building according to
415 the ARS 674:1996 [21].



416

417 Fig.11. Dry compressive strength of CEBs made of clay materials from different sites stabilized
 418 with CCR cured for 45 days at 40 ± 2 °C: K= Kamboinse, P= Pabre, Ko= Kossodo, S= Saaba.

419 The average compressive strength of the CEBs was substantially improved by stabilization with
 420 10 % and 20 % CCR (Fig.11). The addition of 20 % CCR resulted in increasing the compressive
 421 strength, with respect to the control, more than 3.3 times (1.1 to 4.7 MPa) for Kamboinse, 2.6
 422 times (2 to 7.1 MPa) for Pabre, 3.5 times (1.4 to 6.4 MPa) for Kossodo and 10 times (0.8 to
 423 8.3 MPa) for Saaba (Table 4). Saaba not only recorded the highest compressive strength (8.3 MPa)
 424 after stabilization with 20 % CCR, but also the similar rate of improvement of the strength in the
 425 ranges of 0-10 % and 10-20 % CCR, contrary to other sites which presented little improvement
 426 beyond 10 % CCR (Fig.11). This suggests that the compressive strength of clay material from
 427 Saaba can further be improved by the addition of more than 20 % CCR while it would likely get
 428 to the optimum beyond 20 % CCR for other sites. Additionally, the average compressive strength
 429 of CEBs from Kossodo, Pabre and Saaba stabilized with 10 % CCR is greater than 4.5 MPa of the
 430 CEBs stabilized with 10 % geopolymers [25]. The compressive strength of CEBs from Saaba
 431 stabilized 20 % CCR is comparable to 8.5 MPa of CEBs stabilized 20 % geopolymers or 8-10 %
 432 cement [20,25].

433 The improvement of the compressive strength with the addition of CCR is related to the pozzolanic
 434 reaction between the clay materials and CCR. This reaction was previously responsible for the

435 formation of cementitious products such as CSH and CAH for binding the matrix of stabilized
 436 CEBs [24,26,44]. The reactivity of different clay materials (Section 3.5.1) is partly responsible for
 437 the difference in the improvement of the compressive strength of CEBs. Clay materials which
 438 recorded the highest rate of reaction (Saaba and Kossodo) recorded the highest rate of
 439 improvement of the compressive strength. Therefore, clay materials which would not fit for
 440 production of unstabilized CEBs can rather be used for production of stabilized CEBs.

441 **Table 4.** Physico-mechanical properties of stabilized CEBs produced from the investigated sites.

Clay material	CCR * content (wt %)	OWC ** (wt %)	Bulk density (kg/m ³)	Total void ratio (%)	Dry compressive strength (MPa)
Kamboinse	0	17	1800 (±12)	34.5 (±0.5)	1.1 (±0.4)
	10	19	1666 (±11)	38.8 (±0.4)	3.9 (±0.3)
	20	22	1624 (±9)	39.8 (±0.3)	4.7 (±0.3)
Pabre	0	16	1805 (±10)	32.1 (±0.6)	2.0 (±0.5)
	10	18	1729 (±5)	35.4 (±0.4)	5.8 (±0.4)
	20	20	1629 (±8)	38.0 (±0.3)	7.1 (±0.6)
Kossodo	0	13	2036 (±17)	30.0 (±0.8)	1.4 (±0.4)
	10	15	1892 (±15)	34.0 (±0.5)	5.7 (±0.5)
	20	17	1849 (±26)	34.6 (±0.9)	6.4 (±0.2)
Saaba	0	18	1646 (±15)	38.1 (±0.4)	0.8 (±0.3)
	10	20	1617 (±31)	38.8 (±1.2)	5.4 (±0.2)
	20	22	1613 (±9)	38.6 (±0.3)	8.3 (±0.1)

*CCR : Mass percent of CCR with respect to the dry mass of the clay material;

**OWC: Optimum water content, with respect to the dry mass of clay material + CCR, required for achieving the maximum dry density;

(...) Values in parentheses are the standard deviation.

442 Additionally, some physical properties such as bulk density and total void ratio can have impact
 443 on the mechanical performance of CEBs. Table 4 shows that the bulk density of CEBs stabilized
 444 with 0-20 % CCR ranges in 1800-1600 kg/m³ for clay materials from other sites than Kossodo
 445 which has 2000-1800 kg/m³. This is related to the optimum water content (OWC) required for
 446 achieving appropriate consistency for production of CEBs which increased between 16-22 %,
 447 except for Kossodo (13-17 %), with the addition of 0-20 % CCR. The increasing OWC with
 448 respect to the plasticity and fraction of fine particles of clay materials and addition of CCR resulted
 449 in increasing the apparent total void ratio in range of 32-40 % for the stabilized CEBs, except for
 450 Kossodo (30-34 %). The low OWC required for the material from Kossodo can be explained by

451 its lower content of fine particles (clay+silt) and higher content of gravel compared to other sites
452 (Section 3.1). This is partly responsible for the relatively higher bulk density of CEBs produced
453 from Kossodo. This high bulk density can also be related to the higher specific density (2.9) of the
454 material from Kossodo compared to others (2.6-2.7).

455 The bulk density of CEBs stabilized with 10 % CCR is lesser than 1775 Kg/m³ of CEBs stabilized
456 with 10 % geopolymer or cement, except Kossodo [20,25]. This shows the structural benefit of
457 CCR-stabilized CEBs which can provide similar or better mechanical performance and lower dead
458 load than cement-stabilized CEBs. Furthermore, the decrease of the bulk density of CEBs
459 stabilized with CCR would potentially decrease their thermal conductivity and possibly contribute
460 to the improvement of the thermal comfort in the building [10,25]. It is noteworthy that denser and
461 lesser porous CEBs are more likely to have better durability such as the resistance to water
462 absorption and erosion [7]. These results suggest that clay materials investigated from the four
463 sites stabilized with 20 % CCR, cured for 45 days at 40±2 °C, can be used to produce ordinary
464 CEBs suitable for construction of three-storey building except Kamboinse which is suitable for
465 two-storey building [21]. This is regardless of whether the parameters of texture and plasticity of
466 these clay materials fit in the boundaries recommended by the CRATerre. Nevertheless, further
467 studies should be carried out for better understanding the long time durability and other
468 engineering properties of these CEBs.

469 **4. Conclusion**

470 The suitability of clay materials from 4 different sites located in the vicinity of Ouagadougou in
471 Burkina Faso was investigated for the production of CCR-stabilized CEBs. The characterization
472 of the clay materials showed that Pabre and Kossodo respectively contain the highest (20-30 %) and the lowest (<10 %) fraction of clay particles. The material from Kamboinse behaves as
473 medium to high plastic clay-silt, Pabre as medium plastic clay giving it high cohesion, Kossodo
474 medium to high plastic clay-silt, Pabre as medium plastic clay giving it high cohesion, Kossodo

475 as medium plastic clay-silt and Saaba as medium to high plastic silt. The material from Saaba
476 contains the highest fraction of clay minerals (60-75 %), mainly kaolinite, followed by Kamboinse
477 (45-75 %) while Pabre contains the highest fraction of quartz (45-60 %). Although Kossodo seems
478 to contain medium fraction of clay mineral (35-45%), different independent analyses suggested
479 that its kaolinite is poorly ordered than that of materials from the other sites. Moreover, the clay
480 materials from Kossodo as well as Saaba recorded the earliest and highest rate of reactivity with
481 CCR reaching the apparent maturity in 28 days of curing at 40 ± 2 °C.

482 The relatively high cohesion of clay material from Pabre allows to produce unstabilized CEBs
483 which have the highest compressive strength (2 MPa) compared to Kamboinse (1.1 MPa),
484 Kossodo (1.4 MPa) and Saaba (0.8 MPa). Additionally, the compressive strength of CEBs
485 recorded improvement when they were stabilized with 20 % CCR reaching 7.1 MPa for Pabre,
486 4.7 MPa for Kamboinse, 6.4 MPa for Kossodo and 8.3 MPa for Saaba. The highest improvement
487 of the compressive strength of stabilized CEBs from Saaba and Kossodo was related to their
488 highest rate of reactivity with CCR. This reactivity can be related to the highest content and degree
489 of disorder of kaolinite in the clay materials from Saaba and Kossodo, respectively. In addition,
490 the clay material from Kossodo recorded the highest bulk density (2000-1800 kg/m³) and void
491 ratio (30-34 %) related to its low plasticity and fraction of fine particles and high specific density
492 compared to other sites.

493 The results show that the clay materials from all studied sites stabilized with 20 % CCR are suitable
494 for production of stabilized CEBs with compressive strength of 4 MPa required for bearing load
495 in two-storey building. This suggests that clay materials which would not be suitable for
496 production of unstabilized CEBs can be suitable for production of CCR-stabilized CEBs. These
497 results also reveal that the selection of clay materials for production of CEBs based only on their
498 texture and plasticity may be misleading in leaving out the clay materials which would be suitable
499 considering their reactivity with the stabilizer. Nevertheless, more engineering and durability

500 properties, such as water absorption, compressive strength in saturated conditions and thermal
501 properties, should be tested in order to better understand the performances of these CEBs in
502 conditions of usage. Moreover, the study of technical feasibility should be carried on the viability
503 of exploitation of the largest deposit (Kamboinse) whose volumetric quantity was estimated at
504 700 000 m³.

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506 None

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518 **References**

- 519 [1] N. a. Nzeugang, M.R. Eko, N. Fagel, K. V. Kabeyene, a. Njoya, B. a. Madi, J.R. Mache, M.U. Chinje,
520 Characterization of clay deposits of Nanga-Eboko (central Cameroon): suitability for the production of building
521 materials, *Clay Miner.* 48 (2013) 655–662. doi:10.1180/claymin.2013.048.4.18.
- 522 [2] A. Nkalih Mefire, A. Njoya, R.Y. Fouateu, J.R. Mache, N.A. Tapon, A.N. Nzeugang, U.M. Chinje, P.Pilate,
523 P.Filament, S. Siniapkine, A. Ngono, N.Fagel, Occurrences of kaolin in Koutaba (west Cameroon):

- 524 Mineralogical and physicochemical characterization for use in ceramic products, *Clay Miner.* 50 (2015) 593–
525 606. doi:10.1180/claymin.2015.050.5.04.
- 526 [3] H. El Boudour El Idrissi, L. Daoudi, M. El Ouahabi, A. Balo Madi, F. Collin, N. Fagel, Suitability of soils and
527 river deposits from Marrakech for the manufacturing of earthenware, *Appl Clay Sci.* 129 (2016) 108–115.
528 doi:10.1016/j.clay.2016.05.013.
- 529 [4] B. Abdelmalek, B. Rehia, B. Youcef, B. Lakhdar, N. Fagel, Mineralogical characterization of Neogene clay
530 areas from the Jijel basin for ceramic purposes (NE Algeria -Africa), *Appl Clay Sci.* 136 (2017) 176–183.
531 doi:10.1016/j.clay.2016.11.025.
- 532 [5] D. Tsozué, A.N. Nzeugang, J.R. Mache, S. Loweh, N. Fagel, Mineralogical, physico-chemical and
533 technological characterization of clays from Maroua (Far-North, Cameroon) for use in ceramic bricks
534 production, *J Build Eng.* 11 (2017) 17–24. doi:10.1016/j.job.2017.03.008.
- 535 [6] H.B. Nagaraj, M.V. Sravan, T.G. Arun, K.S. Jagadish, Role of lime with cement in long-term strength of
536 Compressed Stabilized Earth Blocks, *Int J Sustain Built Environ.* 3 (2014) 54–61.
537 doi:10.1016/j.ijbe.2014.03.001.
- 538 [7] J.A. Bogas, M. Silva, M. da G. Gomes, Unstabilized and stabilized compressed earth blocks with partial
539 incorporation of recycled aggregates of recycled aggregates, *Int J Archit Herit.* 3058 (2018) 1–16.
540 doi:10.1080/15583058.2018.1442891.
- 541 [8] A.A. Shubbar, M. Sadique, P. Kot, W. Atherton, Future of clay-based construction materials – A review, *Constr*
542 *Build Mater.* 210 (2019) 172–187. doi:10.1016/j.conbuildmat.2019.03.206.
- 543 [9] G.M. Reeves, I. Sims, J.C. Cripps, *Clay Materials Used in Construction*, Geology Society of London, London,
544 2006.
- 545 [10] H.S. Moussa, P. Nshimiyimana, C. Hema, O. Zoungrana, A. Messan, L. Courard, Comparative Study of
546 Thermal Comfort Induced from Masonry Made of Stabilized Compressed Earth Block vs Conventional
547 Cementitious Material, *J Miner Mater Charact Eng.* 7 (2019) 385–403. doi:10.4236/jmmce.2019.76026.
- 548 [11] C.M. Hema, G. Van Moeseke, A. Evrad, L. Courard, A. Messan, Vernacular housing practices in Burkina
549 Faso: Representative models of construction in Ouagadougou and walls hygrothermal efficiency, in: *Energy*
550 *Procedia*, Elsevier, 2017: pp. 535–540. doi:10.1016/j.egypro.2017.07.398.
- 551 [12] A.L. Murmu, A. Patel, Towards sustainable bricks production: An overview, *Constr Build Mater.* 165 (2018)
552 112–125. doi:10.1016/j.conbuildmat.2018.01.038.
- 553 [13] INSD, *Annuaire statistique 2016*, Institut national de la statistique et de la démographie, Ouagadougou,
554 Burkina Faso, 2017.

- 555 [14] U. Wyss, La construction en « matériaux locaux » état d'un secteur à potentiel multiple. Direction du
556 Développement et de la Coopération Suisse, Ouagadougou, Burkina Faso, 2005.
- 557 [15] J. Cid-Falceto, R.F. Mazarron, I. Canas, Assessment of compressed earth blocks made in Spain: International
558 durability tests, *Constr Build Mater.* 37 (2012) 738–745.
- 559 [16] H. Houben, H. Guillaud, CRATerre: Traité de Construction en Terre: L'encyclopédie de la construction en
560 terre, Vol. I, Editions Parathèses, Marseille, 2006.
- 561 [17] J.C. Morel, A. Pkla, P. Walker, Compressive strength testing of compressed earth blocks, *Constr Build Mater.*
562 21 (2007) 303–309. doi:10.1016/j.conbuildmat.2005.08.021.
- 563 [18] M.C.J. Delgado, I.C. Guerrero, The selection of soils for unstabilised earth building: A normative review,
564 *Constr Build Mater.* 21 (2007) 237–251. doi:10.1016/j.conbuildmat.2005.08.006.
- 565 [19] S. Diamond, J.L. White, W.L. Dolch, Transformation of Clay Minerals by Calcium Hydroxide Attack, in:
566 *Twelfth Natl Conf Clays Clay Miner*, 1963: pp. 359–379. doi:10.1346/CCMN.1963.0120134.
- 567 [20] P. Walker, T. Stace, Properties of some cement stabilised compressed earth blocks and mortars, *Mater Struct*
568 *Constr.* 30 (1997) 545–551. doi:10.1007/BF02486398.
- 569 [21] CDI, CRATerre-EAG, Compressed Earth Blocks: Standards guide-Technology series N° 11, CDI (ARSO),
570 Brussels-Belgium, 1998.
- 571 [22] B.V. Venkatarama Reddy, S.R. Hubli, Properties of lime stabilised steam-cured blocks for masonry, *Mater*
572 *Struct.* 35 (2002) 293–300. doi:10.1007/BF02482135.
- 573 [23] P.J. Walker, Strength and Erosion Characteristics of Earth Blocks and Earth Block Masonry, *J Mater Civ Eng.*
574 16 (2004) 497–506. doi:10.1061/(ASCE)0899-1561(2004)16:5(497).
- 575 [24] P. Nshimiyimana, D. Miraucourt, A. Messan, L. Courard, Calcium Carbide Residue and Rice Husk Ash for
576 improving the Compressive Strength of Compressed Earth Blocks, *MRS Adv.* 3 (2018) 2009–2014.
577 <http://hdl.handle.net/2268/226552>.
- 578 [25] O.S. Sore, A. Messan, E. Prud'homme, G. Escadeillas, F. Tsobnang, Stabilization of compressed earth blocks
579 (CEBs) by geopolymer binder based on local materials from Burkina Faso, *Constr Build Mater.* 165 (2018)
580 333–345. doi:10.1016/j.conbuildmat.2018.01.051.
- 581 [26] P. Nshimiyimana, A. Messan, Z. Zhao, L. Courard, Chemico-microstructural changes in earthen building
582 materials containing calcium carbide residue and rice husk ash, *Constr Build Mater.* 216C (2019) 622–631.
583 doi:<https://doi.org/10.1016/j.conbuildmat.2019.05.037>.
- 584 [27] A. Al-Fakih, B.S. Mohammed, M.S. Liew, E. Nikbakht, Incorporation of waste materials in the manufacture
585 of masonry bricks: An update review, *J Build Eng.* 21 (2019) 37–54. doi:10.1016/j.job.2018.09.023.

- 586 [28] S. Masuka, W. Gwenzi, T. Rukuni, Development, engineering properties and potential applications of unfired
587 earth bricks reinforced by coal fly ash, lime and wood aggregates, *J Build Eng.* 18 (2018) 312–320.
588 doi:10.1016/j.jobbe.2018.03.010.
- 589 [29] W. Mechti, T. Mnif, B. Samet, M.J. Rouis, Effect of the secondary minerals on the pozzolanic activity of
590 calcined clay: case of quartz, *IJRRAS.* 12 (2012) 61–71. doi:10.1684/bdc.2011.1430.
- 591 [30] D. Ciancio, C.T.S. Beckett, J.A.H. Carraro, Optimum lime content identification for lime-stabilised rammed
592 earth, *Constr Build Mater.* 53 (2014) 59–65. doi:10.1016/j.conbuildmat.2013.11.077.
- 593 [31] M. Al-Mukhtar, S.A.A. Khattab, J.-F. Alcover, Micorstructure and geotechnical properties of lime treated
594 expansive soil, *Eng Geol.* 139–140 (2012) 17–27. doi:10.1061/(ASCE)0899-1561(2007)19:4(358).
- 595 [32] NF EN ISO 17892-4, Geotechnical investigation and testing - Laboratory testing of soil - Part 4 : Determination
596 of particle size distribution, AFNor, Saint-Denis La Plaine Cedex, 2018.
- 597 [33] NF P 94-051, Sols: reconnaissance et essais-Détermination des limites d'Atterberg-Limite de liquidité à la
598 coupelle-Limite de plasticité au rouleau, AFNor, Saint-Denis La Plaine Cedex, 1993.
- 599 [34] NF EN 1097-6, Essais pour déterminer les caractéristiques mécaniques et physiques des granulats - Partie 6 :
600 détermination de la masse volumique réelle et du coefficient d'absorption d'eau, Saint-Denis La Plaine Cedex,
601 2014.
- 602 [35] NF P94-093, Sols : reconnaissance et essais - Détermination des références de compactage d'un matériau -
603 Essai Proctor Normal - Essai Proctor modifié, AFNor, Saint-Denis La Plaine Cedex, 2014.
- 604 [36] H.E. Cook, P.D. Johnson, J.C. Matti, I. Zemmels, Methods of sample preparation and x-ray diffraction data
605 analysis, x-ray mineralogy laboratory, in: D.E. Hayes, L.A. Frakes (Eds.), *Init Repts DSDP, U.S. Govt. Printing*
606 *Office, Washington, 1975: pp. 999–1007.*
- 607 [37] T. Boski, J. Pessoa, P. Pedro, J. Thorez, J.M.A. Dias, Factors governing abundance of hydrolyzable amino
608 acids in the sediments from the N . W . European Continental Margin (47-50 °N), *Prog Oceanogr.* 42 (1998)
609 145–164.
- 610 [38] N. Fagel, T. Boski, L. Likhoshway, H. Oberhaensli, Late Quaternary clay mineral record in Central Lake Baikal
611 (Academician Ridge, Siberia), *Palaeogeogr Palaeoclimatol Palaeoecol.* 193 (2003) 159–179.
612 doi:10.1016/S0031-0182(02)00633-8.
- 613 [39] M. Földvári, *Handbook of the thermogravimetric system of minerals and its use in geological practice,*
614 *Hungarian Academy of Sciences, Budapest, 2011. doi:10.1556/CEuGeol.56.2013.4.6.*
- 615 [40] J. Yvon, O. Lietard, J.M. Cases, J.F. Delon, *Minéralogie des argiles kaoliniques des Charentes, Bull*
616 *Minéralogie.* 105 (1982) 431–437.

- 617 [41] CDE, CRATerre-EAG, ENTPE, Compressed earth blocks: testing procedures guide-Technology series N° 16,
618 CDE (ARSO), Brussels-Belgium, 2000.
- 619 [42] XP P 13-901, Blocs de terre comprimée pour murs et cloisons, Définitions-Spécifications-Méthodes d'essais-
620 Conditions de réception, AFNor, Saint-Denis La Plaine Cedex, 2001.
- 621 [43] B. Lorentz, N. Shanahan, Y.P. Stetsko, A. Zayed, Characterization of Florida kaolin clays using multiple-
622 technique approach, *Appl Clay Sci.* 161 (2018) 326–333. doi:10.1016/j.clay.2018.05.001.
- 623 [44] M. Al-Mukhtar, S. Khattab, J.F. Alcover, Microstructure and geotechnical properties of lime-treated expansive
624 clayey soil, *Eng Geol.* 139–140 (2012) 17–27. doi:10.1016/j.enggeo.2012.04.004.
- 625 [45] C. Bich, J. Ambroise, J. Péra, Influence of degree of dehydroxylation on the pozzolanic activity of metakaolin,
626 *Appl Clay Sci.* 44 (2009) 194–200. doi:10.1016/j.clay.2009.01.014.
- 627 [46] A. Tironi, M.A. Trezza, A.N. Scian, E.F. Irassar, Kaolinitic calcined clays: Factors affecting its performance
628 as pozzolans, *Constr Build Mater.* 28 (2012) 276–281. doi:10.1016/j.conbuildmat.2011.08.064.
- 629 [47] M. Al-Mukhtar, A. Lasledj, J.-F. Alcover, Behaviour and mineralogy changes in lime-treated expansive soil at
630 50°C, *Appl Clay Sci.* 50 (2010) 199–203. doi:10.1016/j.clay.2010.07.022.
- 631