

# Updating Fatigue Failure Probability

Considering Monitoring and Inspection Data

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Quang Mai

December 7, 2018

Faculty of Applied Sciences



Strain  
Measurement Data



Crack Inspection  
& Repair  
Data



Wind  
Measurement Data



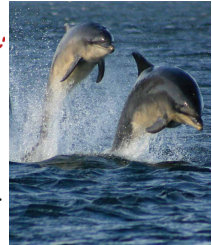
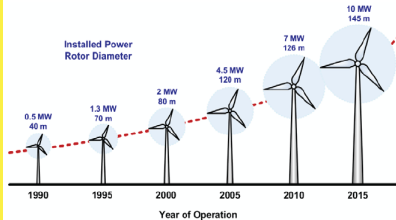
Wave  
Measurement Data

# Introduction

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# Wind Energy – the fastest growing energy source

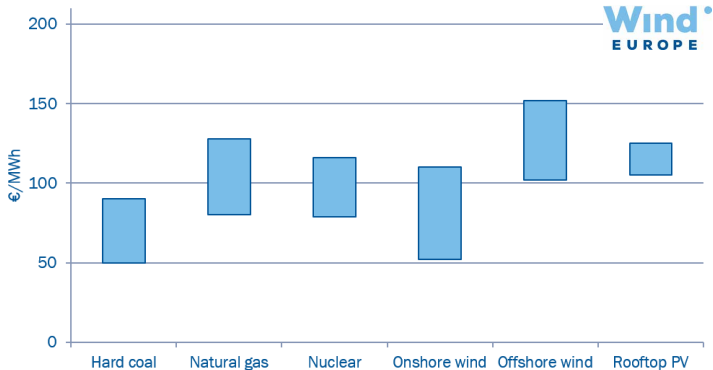
- Less environmental impact
- Fast installation
- Fast development!



# Levelized Cost of Electricity

High LCoE

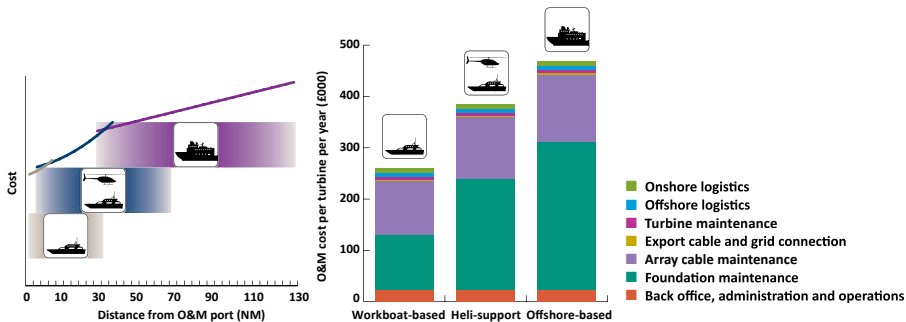
(lifetime costs per unit of electricity – MWh)



# Levelized Cost of Electricity

High LCoE

O&M Cost

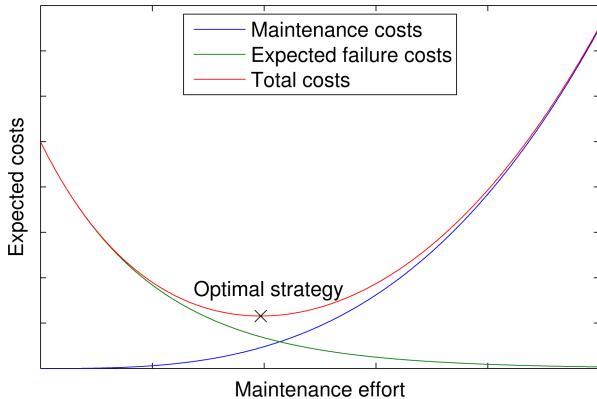


# Levelized Cost of Electricity

High LCoE

O&M Cost

Risk-based Inspection



# Levelized Cost of Electricity

High LCoE

O&M Cost

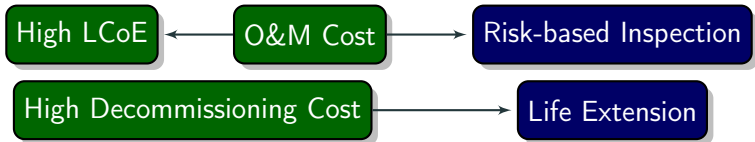
Risk-based Inspection

High Decommissioning Cost

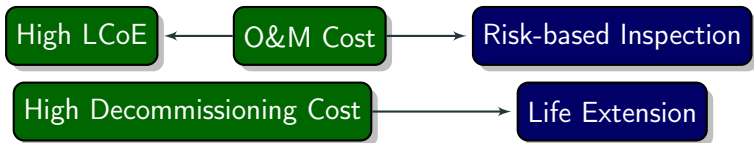




# Levelized Cost of Electricity



## Levelized Cost of Electricity



Majority of reported offshore failures are fatigue failures!

⇒ Update Failure Probability in **Fatigue Failure Mode!**

# Message Objective

Using **Failure Assessment Diagram** and **Occurrence of Weather Conditions** in the Limit State Function improves the accuracy of the **Updated Failure Probability**.



# Road Map

Introduction

Background

FAD in Updating Considering Inspection

Occurrence of Weather Conditions in Updating Considering Monitoring

Conclusion

# Background

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# Road Map

Introduction

Background

Fatigue Assessment

Failure Probability

Updating Principle

FAD in Updating Considering Inspection

Occurrence of Weather Conditions in Updating Considering  
Monitoring

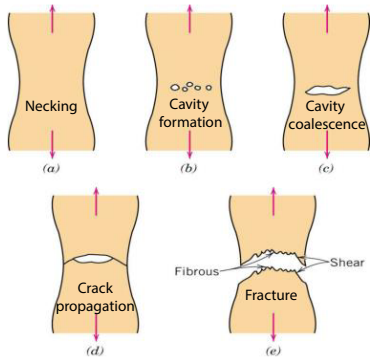
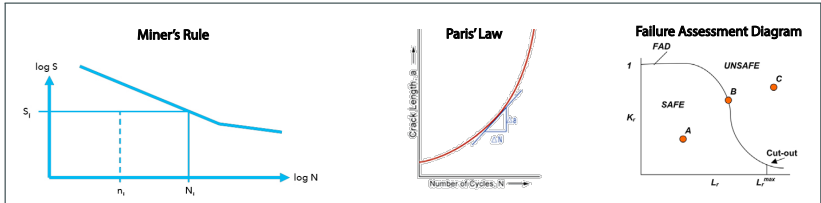
Conclusion

# Background

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## Fatigue Assessment

# Fatigue Assessment





# Fatigue Assessment: Miner's Rule

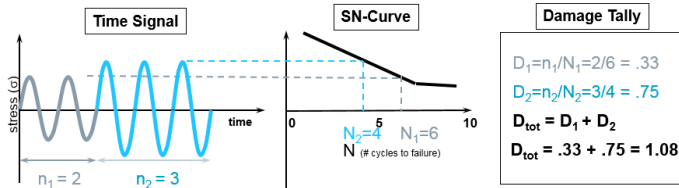
Fatigue Damage:

$$D = \sum_{i=1}^k \frac{n_i}{N_i}$$

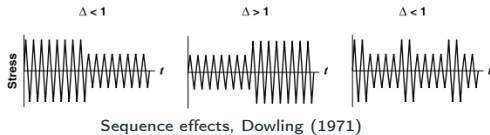
Safety condition:  $D \leq \Delta$ , or

Limit State Function:

$$g = \Delta - D \geq 0$$

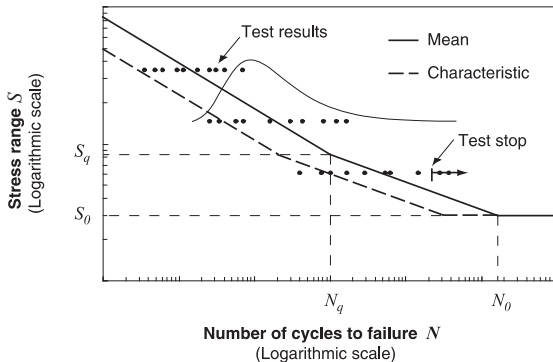


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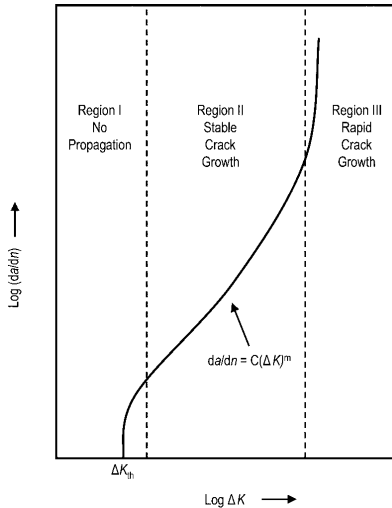


Limit State Function:

$$g = \Delta - D$$

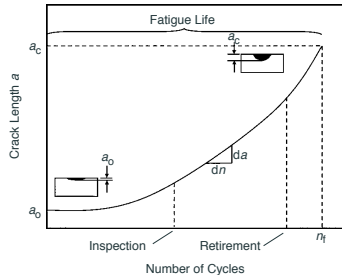


# Fatigue Assessment: Paris' Law

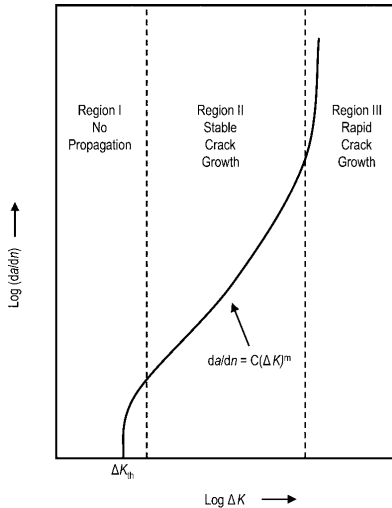


$$\frac{da}{dN} = C (\Delta K)^m$$

$$\Delta K = Y \Delta \sigma \sqrt{\pi a}$$



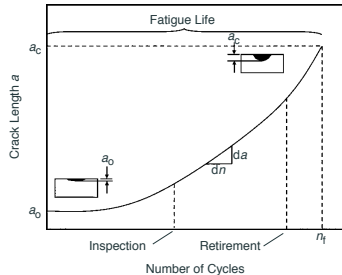
# Fatigue Assessment: Paris' Law



Limit State Functions:

$$g_1 = a_c - a$$

$$g_2 = K_{mat} - K$$



# Fatigue Assessment: Failure Assessment Diagram

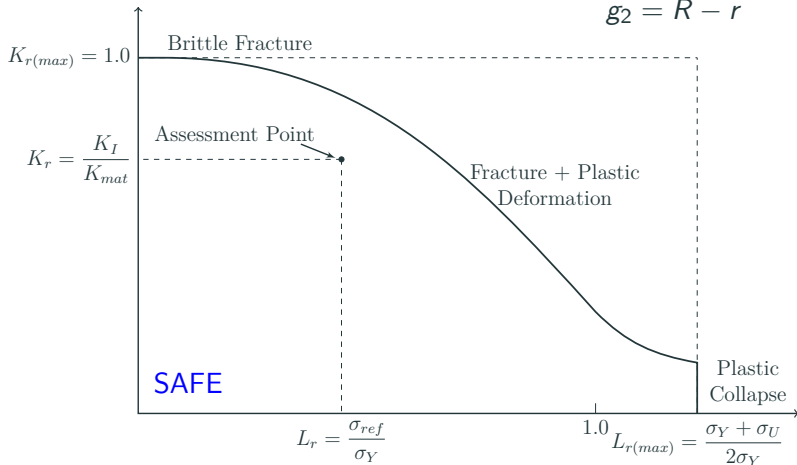
Assessing acceptability of cracks

Limit State Function:

$$g_1 = K_{rFAD} - K_r$$

or:

$$g_2 = R - r$$



# Background

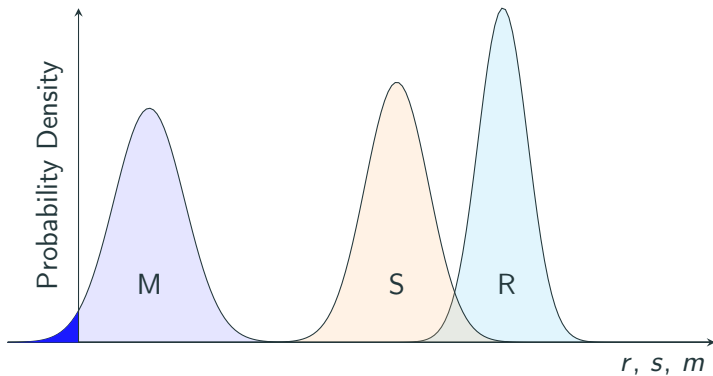
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## Failure Probability

# Failure Probability

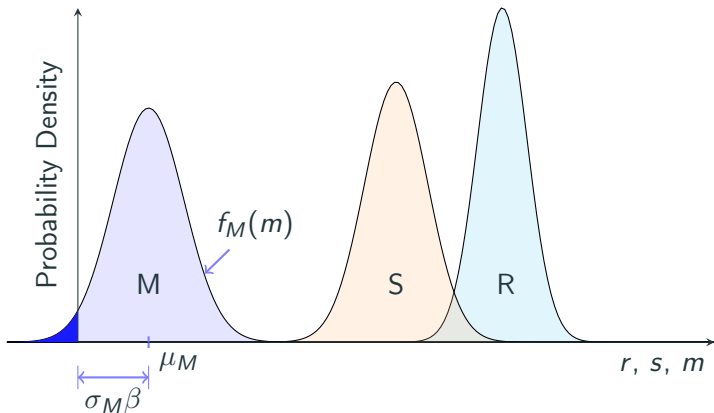
Safety margin (Limit State Function):

$$M = R - S$$



# Failure Probability

$$P_F = \int_{-\infty}^0 f_M(m) dm = \Phi\left(\frac{0 - \mu_M}{\sigma_M}\right) = \Phi(-\beta)$$





# Background

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## Updating Principle

$$\text{Bayes Theorem: } P(A|B) = \frac{P(A, B)}{P(B)}$$

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Event updating

Variable updating

# Updating Principle

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Variable updating

# Updating Principle

$$M = R - S$$

Crack inspection:

$$P(M < 0 | I) = \frac{P(M < 0 \cap I)}{P(I)}$$

Load Monitoring data:

$$f_{S|I}(s|I) = C \cdot P(I|s) f_S(s)$$

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## FAD in Updating Considering Inspection

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# Road Map

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FAD in Updating Considering Inspection

Motivation & Literature Review

FAD Gives Higher Failure Probability Values

FAD in Updating

Occurrence of Weather Conditions in Updating Considering  
Monitoring

Conclusion

# FAD in Updating Considering Inspection

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Motivation & Literature Review

# Motivation

LSF 1:  $g = a_c - a$

$a_c$ : critical crack size

$a$ : crack size

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$\Rightarrow$  ambiguous, for ships & pipelines!

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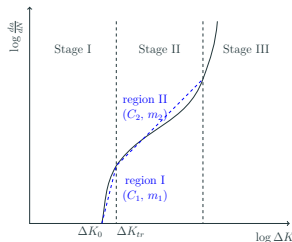
$a$ : crack size

$\Rightarrow$  ambiguous, for ships & pipelines!

$$\text{LSF 2: } \begin{cases} g_1 &= a_c - a \\ g_2 &= K_{mat} - K_{max} \end{cases}$$

$K_{mat}$ : fracture toughness

$K_{max}$ : maximum stress intensity factor



# Motivation

LSF 1:  $g = a_c - a$

$a_c$ : critical crack size

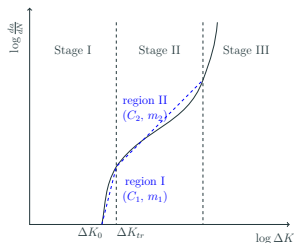
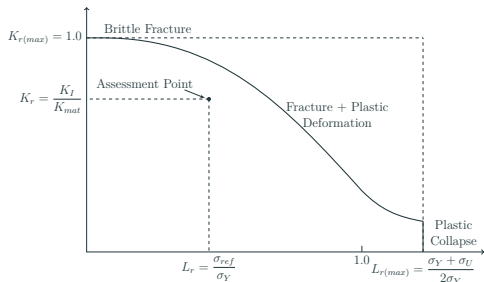
$\Rightarrow$  ambiguous, for ships & pipelines!

$a$ : crack size

LSF 2:  $\begin{cases} g_1 = a_c - a \\ g_2 = K_{mat} - K_{max} \end{cases} \Rightarrow$  fracture+plastic deformation?

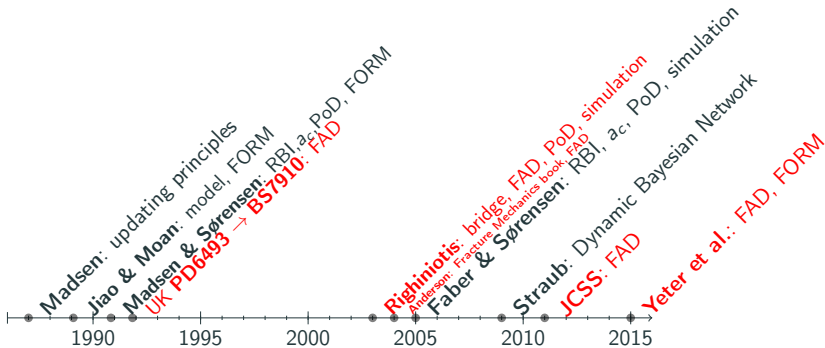
$K_{mat}$ : fracture toughness

$K_{max}$ : maximum stress intensity factor



# Literature Review

Evolution of updating considering crack inspection data:



## Gap:

Advantages and disadvantages of using Failure Assessment Diagram in updating failure probability considering crack inspection data for existing OWT support structures?



# FAD in Updating Considering Inspection

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FAD Gives Higher Failure Probability Values

# Road Map

FAD in Updating Considering Inspection

Motivation & Literature Review

**FAD Gives Higher Failure Probability Values**

Limit State Functions to Compare

Method to Calculate Failure Probabilities

Results & Discussions

FAD in Updating

# Limit State Functions to Compare

LSF 1:

$$g = a_c - a$$

LSF 2:

$$\begin{cases} g_1 = a_c - a \\ g_2 = K_{mat} - K_{max} \end{cases}$$

LSF 3:

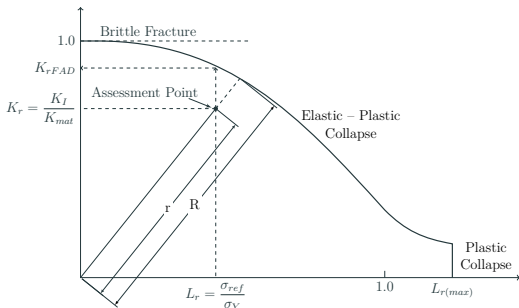
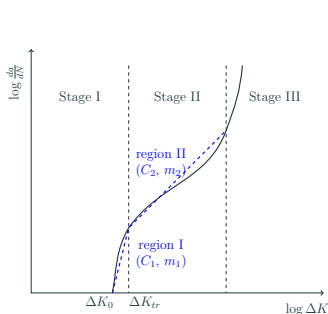
$$g = K_{rFAD} - K_r$$

or:

$$g = R - r$$

Considered Uncertainties:  $C$ ,  $a_0$ ,  $a_0/c_0$ , and FAD

FAD uncertainty: from Offshore Technology Report (HSE, 2000)



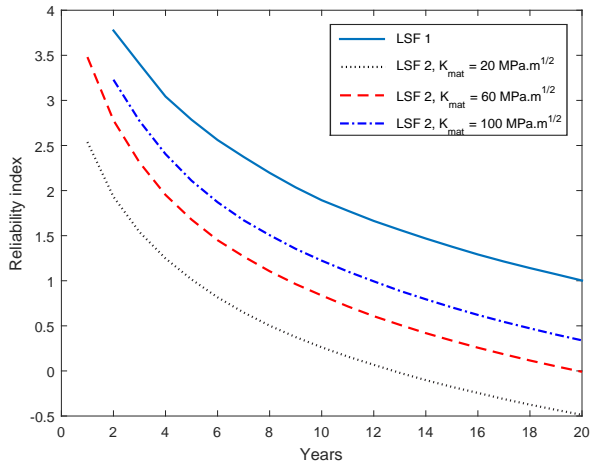
# Method to Calculate Failure Probabilities

- Monte Carlo Simulation:  $10^5$  samples of crack propagations
- Crack depth and crack length are coupled
- Constant amplitude stress history
- Failure Probability:

$$P_f = \frac{1}{N} \sum_{j=1}^N I[g]$$

$$I[g] = \begin{cases} 0 & \text{if } g > 0 \\ 1 & \text{if } g \leq 0 \end{cases}$$

# Results & Discussions: LSF-2 vs. LSF-1



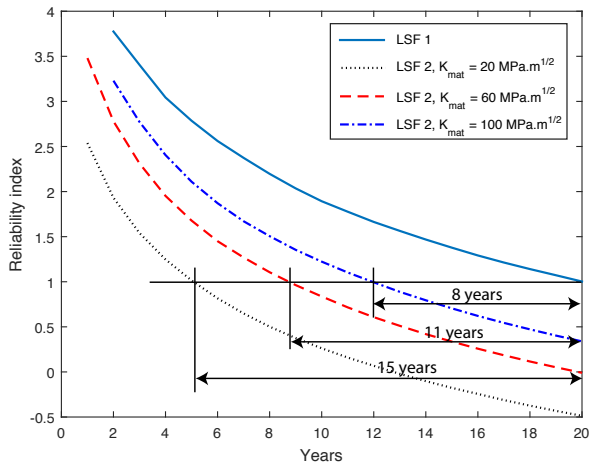
LSF 1:

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# Results & Discussions: LSF-2 vs. LSF-1



LSF-1:

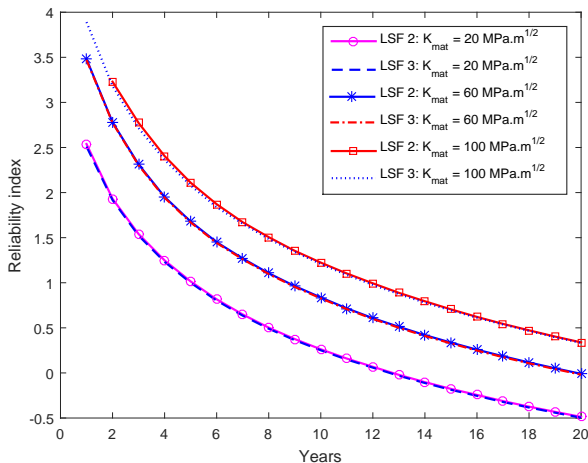
$$g = a_c - a$$

⇒ Under-estimate  $P_f$

LSF-2:

$$\begin{cases} g_1 = a_c - a \\ g_2 = K_{mat} - K_{max} \end{cases}$$

# Results & Discussions: LSF-3 vs. LSF-2



Use of  $S_{max}$ :

$$K_{max} = S_{max} Y \sqrt{\pi a}$$

$$\sigma_{ref} = \frac{S_{max}^*}{S_Y}$$

$$K_r = \frac{K_{max}}{K_{mat}}$$

$$\frac{S_{max}}{S_Y} = 0.32$$

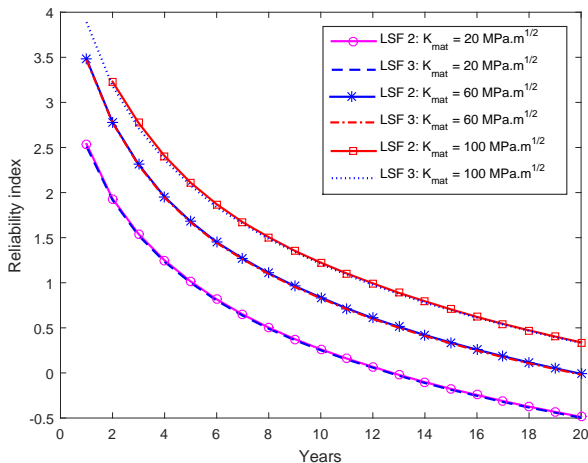
LSF-3:

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LSF-2:

$$\begin{cases} g_1 = a_c - a \\ g_2 = K_{mat} - K_{max} \end{cases} \quad 26$$

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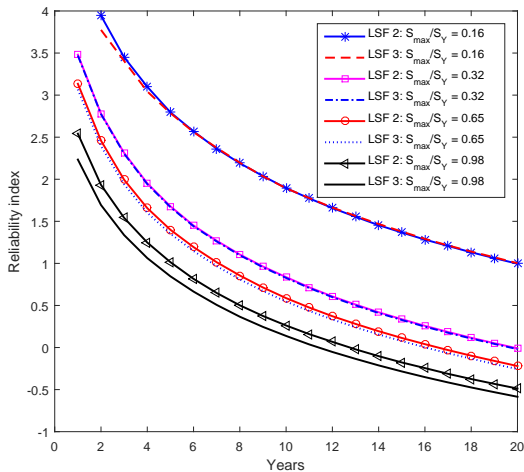
$$S_{max} \ll S_Y$$

LSF-2:

$$\begin{cases} g_1 = a_c - a \\ g_2 = K_{mat} - K_{max} \end{cases} \quad 27$$



# Results & Discussions: LSF-3 vs. LSF-2



Use of  $S_{max}$ :

$$K_{max} = S_{max} Y \sqrt{\pi a}$$

$$\sigma_{ref} = \frac{S_{max}^*}{\sigma_Y}$$

$$K_r = \frac{K_{max}}{K_{mat}}$$

$$K_{mat} = 60 \text{ MPa m}^{1/2}$$

$$S_Y = 368.75 \text{ MPa}$$

$$S_U = 750.00 \text{ MPa}$$

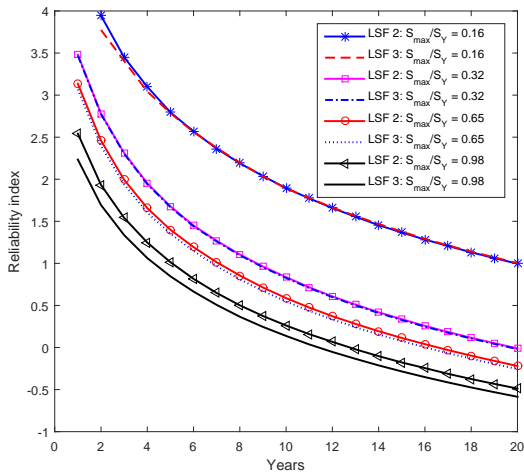
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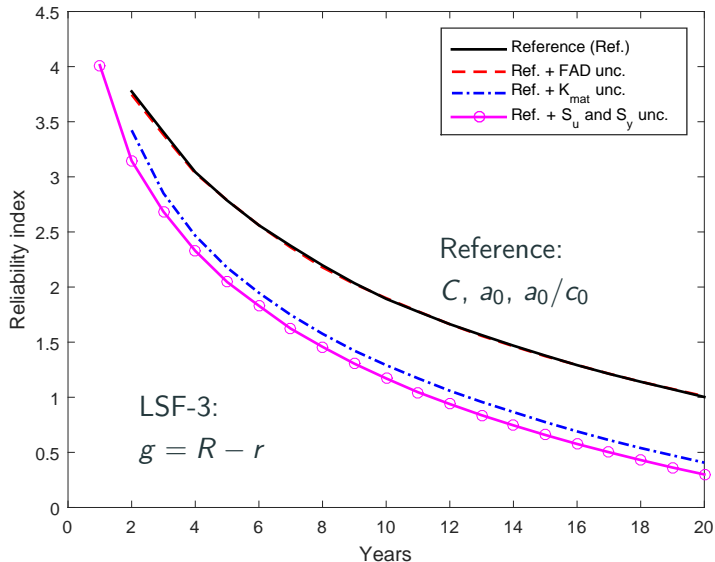
$$S_{max} > 65\% S_Y$$

$$\neq$$

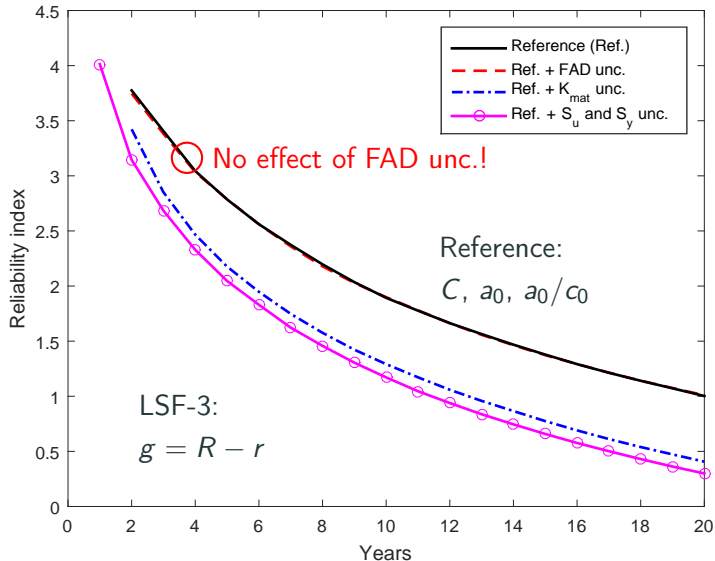
LSF-2:

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# Results & Discussions: FAD Uncertainty



# Results & Discussions: FAD Uncertainty



# FAD in Updating Considering Inspection

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FAD in Updating

FAD in Updating Considering Inspection

Motivation & Literature Review

FAD Gives Higher Failure Probability Values

FAD in Updating

The Updating Problems

Method to Solve

Results & Discussions

# The Updating Problems

Safety Margin using FAD:

$$g = K_{rFAD} - K_r$$

# The Updating Problems

Safety Margin using FAD:

$$g = K_{rFAD} - K_r$$

Crack Detection Event:

$$I_d = c - c_d$$

$c$ : crack length,

$c_d$ : detectable length



# The Updating Problems

Safety Margin using FAD:

$$g = K_{rFAD} - K_r$$

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$$I_d = c - c_d$$

Probability of Detection:

$$P(c_d) = 1 - \frac{1}{1 + \left(\frac{c_d}{x_0}\right)^b}$$

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Update  $P_F$  when **No crack is detected**:

$$P[g \leq 0 | I_d < 0] = ?$$

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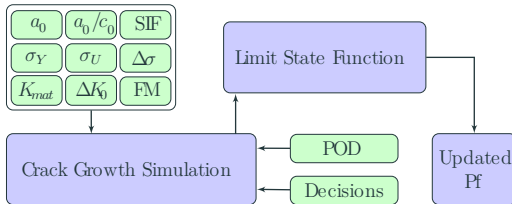
Update  $P_F$  when **Crack is detected and repaired imperfectly**:

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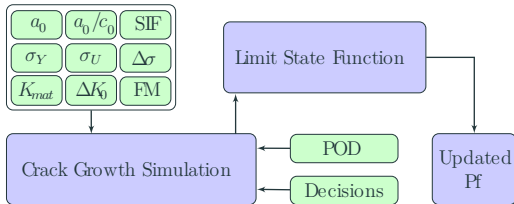
Update  $P_F$  when **Crack is detected and repaired perfectly**:

$$P[g \leq 0 | I_d \geq 0 \cap R_p] = ?$$

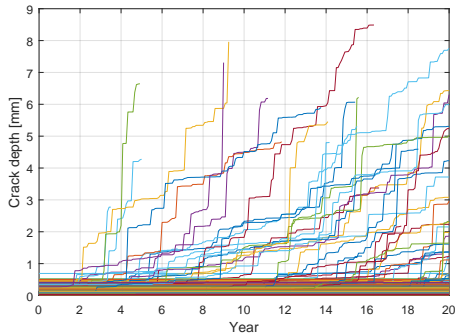
# Method to Solve: Procedure



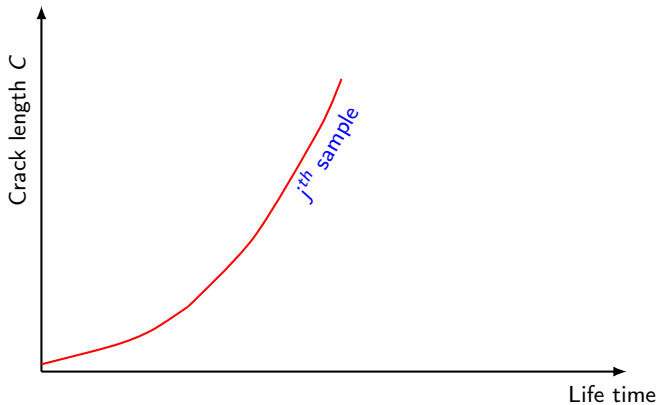
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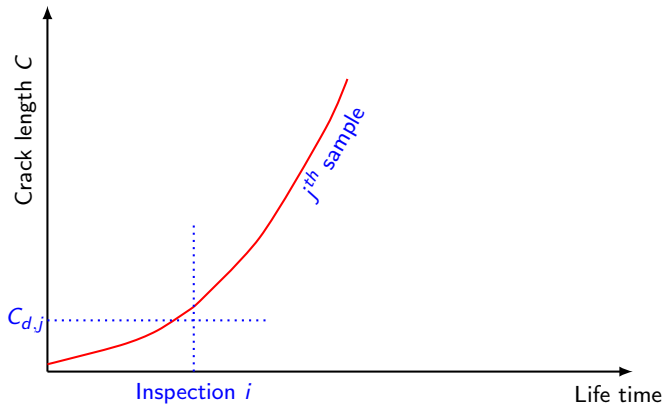
constant amplitude  
during 1 month!



## Method to Solve: Updating

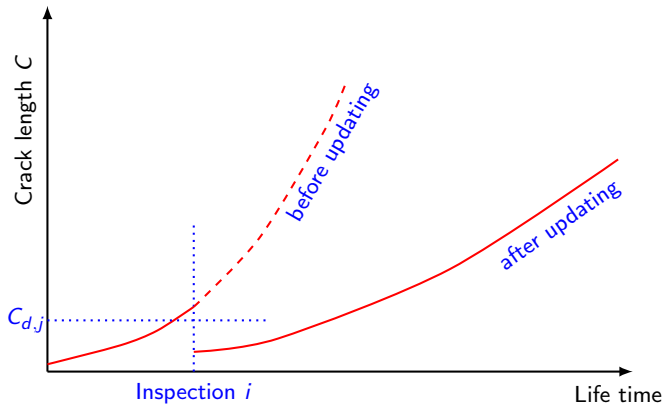


## Method to Solve: Updating

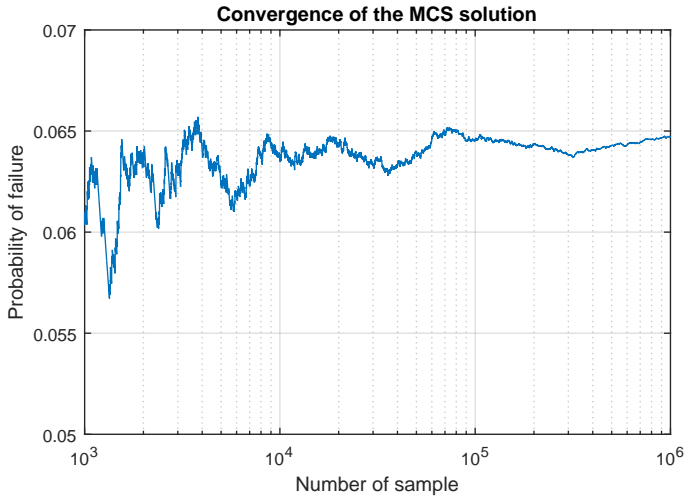




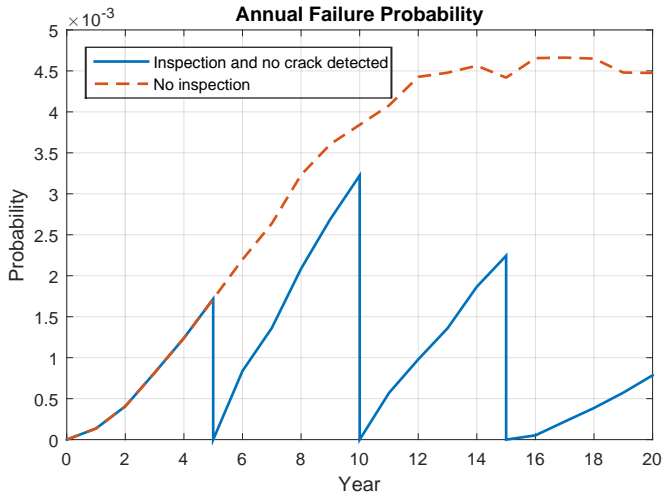
## Method to Solve: Updating



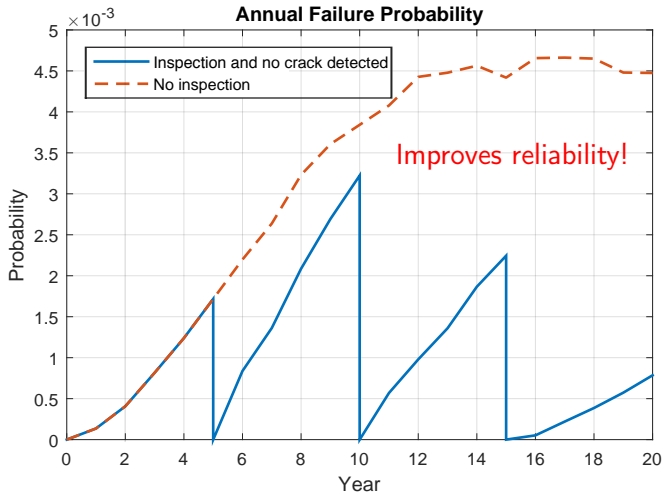
# Results & Discussions



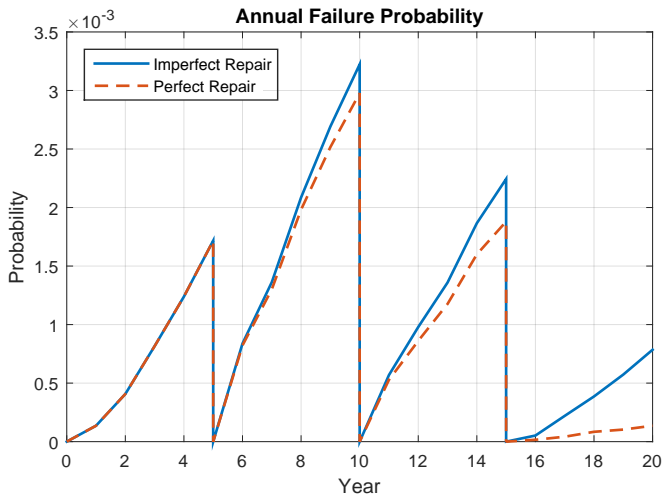
# Results & Discussions



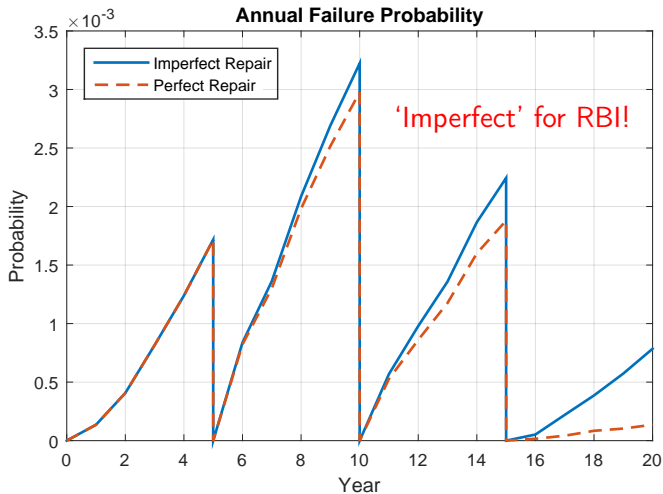
# Results & Discussions



## Results & Discussions



## Results & Discussions



# Road Map

## FAD in Updating Considering Inspection

- Motivation & Literature Review

- FAD Gives Higher Failure Probability Values

  - Limit State Functions to Compare

  - Method to Calculate Failure Probabilities

  - Results & Discussions

## FAD in Updating

- The Updating Problems

- Method to Solve

- Results & Discussions

### Gap:

Advantages and disadvantages of using Failure Assessment Diagram in updating failure probability considering crack inspection data for existing OWT support structures?



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## Disadvantages

- Time consuming.
- Fails to find very small failure probability such as 'detected & not repaired'

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## Advantages

- Releases the assumption about  $a_c$
- more conservative  $P_F$  results  $\Rightarrow$  better for inspection planning!

# Occurrence of Weather Conditions in Updating Considering Monitoring

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# Road Map

Introduction

Background

FAD in Updating Considering Inspection

Occurrence of Weather Conditions in Updating Considering  
Monitoring

Motivation & Literature Review

Methodology

Application

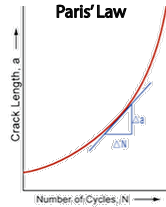
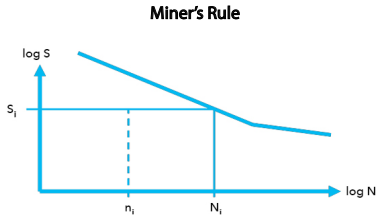
Conclusion

# Occurrence of Weather Conditions in Updating Considering Monitoring

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Motivation & Literature Review

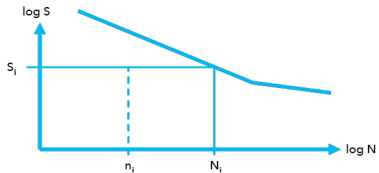
# Motivation



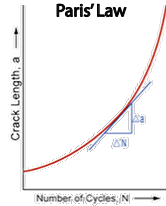
What to do with  
Load monitoring data?

# Motivation

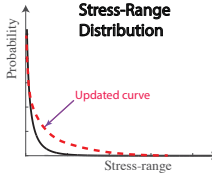
**Miner's Rule**



**Paris' Law**



**Stress-Range Distribution**

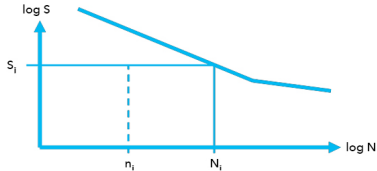


Measure strain

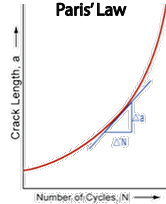
Calculate stress

# Motivation

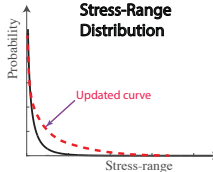
**Miner's Rule**



**Paris' Law**



**Stress-Range Distribution**

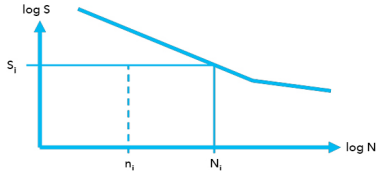


Measure strain:  
not everywhere!  
long-term!

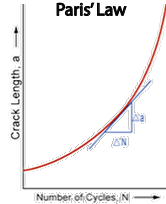
Calculate stress

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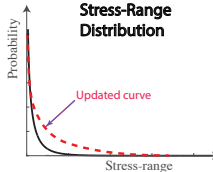
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Stress-Range Distribution

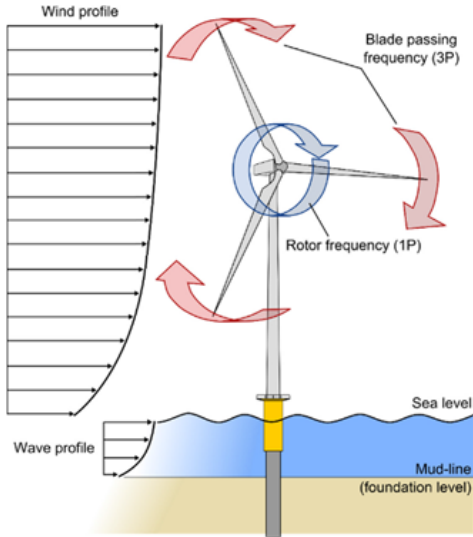


Measure strain:  
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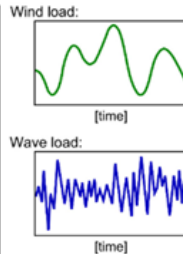
Calculate stress:  
time consuming!  
too much uncertainties!



# Motivation: Uncertainties in FEM



Typical waveforms for the different type of loads:



Sources of Uncertainties:  
Type 1: load calculation  
Type 2: calibrated FEM

## Motivation: The Idea

- Use measured data:  $\Rightarrow$  No load calculation
- Use FEM to extrapolate stress:  $\Rightarrow$  No need to measure everywhere
- Use Occurrence of Weather Conditions in LSF:  
 $\Rightarrow$  Wind & Wave instead of strain

# Motivation: The Idea

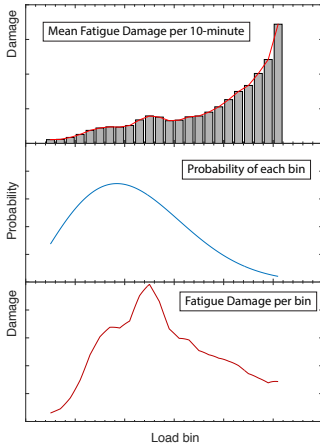
Use measured data:

⇒ No load calculation

Use FEM to extrapolate stress: ⇒ No need to measure everywhere

Use Occurrence of Weather Conditions in LSF:

⇒ Wind & Wave instead of strain



# Motivation: The Idea

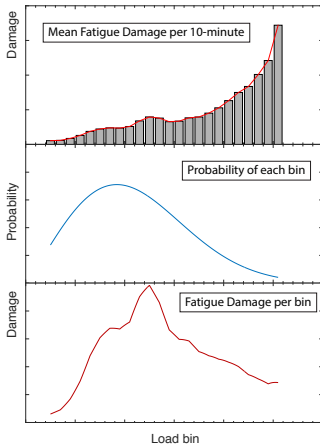
Use measured data:

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Use FEM to extrapolate stress: ⇒ No need to measure everywhere

Use Occurrence of Weather Conditions in LSF:

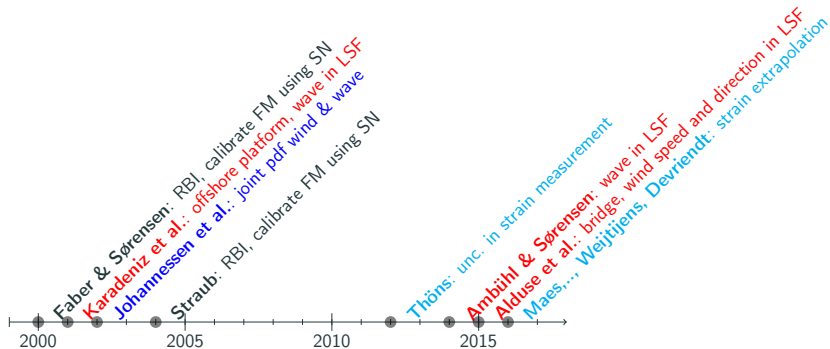
⇒ Wind & Wave instead of strain



$$\begin{aligned} g &= \Delta - D_{total} \\ &= \Delta - \sum_{e=1}^n D_{bin_e} \\ &= \Delta - \sum_{i=e}^n P_{bin_e} \cdot D_{10m,bin_e} \cdot n_{10m,yr} \end{aligned}$$



# Literature Review



## Gap:

How to perform reliability assessment of existing offshore wind turbine support structures using directly the Occurrence of Weather Conditions (wind and wave)?

# Occurrence of Weather Conditions in Updating Considering Monitoring

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Methodology

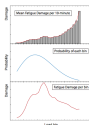
$$g = \Delta - \sum_{e=1}^n P_{bin_e} \cdot D_{mod,bin_e}$$

## Motivation: The Idea

Use measured data:  $\Rightarrow$  **No load calculation**

Use FEM to extrapolate stress:  $\Rightarrow$  **No need to measure everywhere**

Use Load Combination in LSF:  $\Rightarrow$  **Not lifetime strain measurement**



$$\begin{aligned} g &= \Delta - D_{total} \\ &= \Delta - \sum_{e=1}^n D_{bin_e} \\ &= \Delta - \sum_{e=1}^n P_{bin_e} \cdot D_{1(D_{min}, D_{bin_e})} \end{aligned}$$

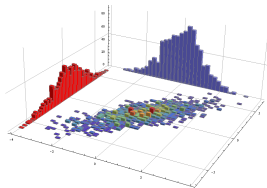


40



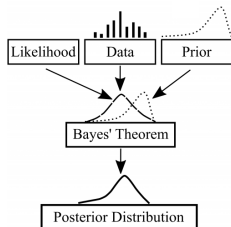
$$g = \Delta - \sum_{e=1}^n \underbrace{P_{bin_e}} \cdot D_{mod, bin_e}$$

$$P(U_{10}, H_s, T_p) = \int \int \int f(U_{10}, H_s, T_p) du \, dh \, dt$$



$$g = \Delta - \sum_{e=1}^n \underbrace{P_{bin_e}} \cdot D_{mod, bin_e}$$

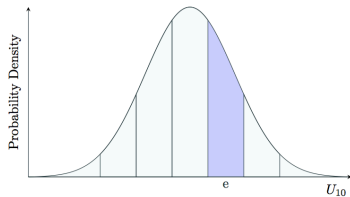
$$P(U_{10}, H_s, T_p) = \int \int \int f \left( \underbrace{U_{10}}, H_s, T_p \right) du \, dh \, dt$$



$$f_{K_w}(k_w \mid \hat{\mathbf{k}}_w)$$

$$g = \Delta - \sum_{e=1}^n \underbrace{P_{bin_e}}_{\substack{D_{1cyc,bin_e} \cdot n_{cyc,bin_e} \\ n_{10min,bin_e}}} \cdot \overbrace{D_{mod,bin_e}}^{n_{10min,total}}$$

$$P(U_{10}, H_s, T_p) = \int \int \int f(\underbrace{U_{10}}, H_s, T_p) du dh dt$$



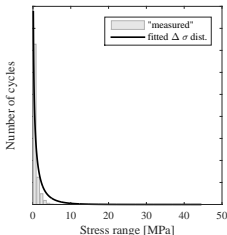
$$f_{K_w}(k_w | \hat{k}_w)$$

$$\frac{(\alpha \cdot X_m \cdot X_{SCF})^m}{K} \underbrace{k_{s,bin_e}^m}_{D_{1cyc,bin_e} \cdot n_{cyc,bin_e}} \Gamma\left(\frac{m}{\lambda_{s,bin_e}} + 1\right) \cdot \frac{n_{10min,total}}{n_{10min,bin_e}}$$

$$g = \Delta - \sum_{e=1}^n \underbrace{P_{bin_e}}_{D_{mod,bin_e}}$$

$$P(U_{10}, H_s, T_p) = \int \int \int f\left(\underbrace{U_{10}}_{}, H_s, T_p\right) du \, dh \, dt$$

$$f_{K_w}(k_w \mid \hat{k}_w)$$



## Methodology: Limit State Function

$$g = \Delta - \sum_{i=1}^T \sum_{j=1}^{n_{U_{10}}} \sum_{k=1}^{n_{H_s}} \sum_{l=1}^{n_{T_p}} \frac{(\alpha_f X_m X_{SCF})^m}{K} k_{s,jkl}^m \Gamma \left( \frac{m}{\lambda_{s,jkl}} + 1 \right) \times \dots$$

$$P(U_{10,j}, H_{s,k}, T_{p,l} | k_{w,i}) \frac{n_{c,jkl}}{n_{m,jkl}} n_m^*$$

## Update Wind Speed Distribution

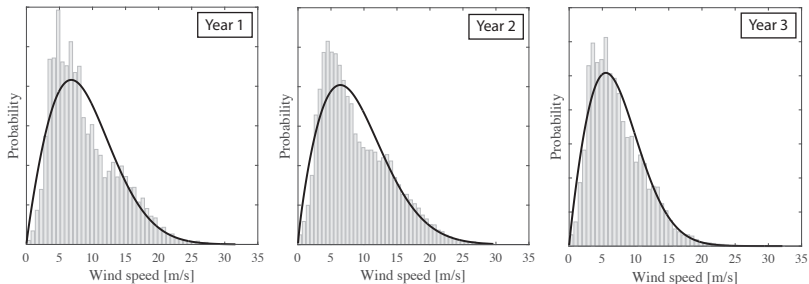
Given three years (or more) of measured wind speed, how to update the design wind speed distribution?

$$\begin{aligned} f_{K_w}(k_w|\mu, \sigma) &= f_N(k_w|\mu, \sigma) \\ &= \frac{1}{\sigma\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{k_w - \mu}{\sigma}\right)^2\right) \end{aligned}$$

The predictive density function of  $k_w$  given measured data becomes a Student's t-distribution.

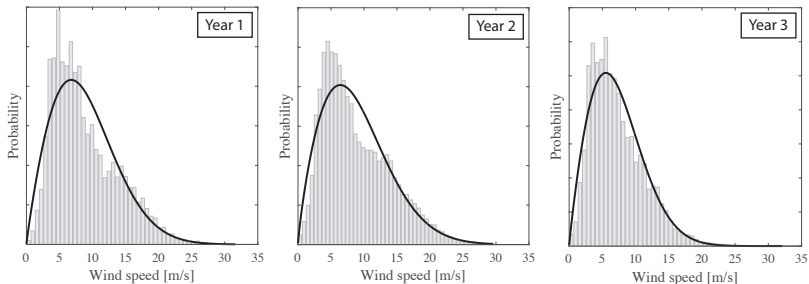
# Methodology: Update Wind Speed Distribution

Keep the 'design' shape parameter:



# Methodology: Update Wind Speed Distribution

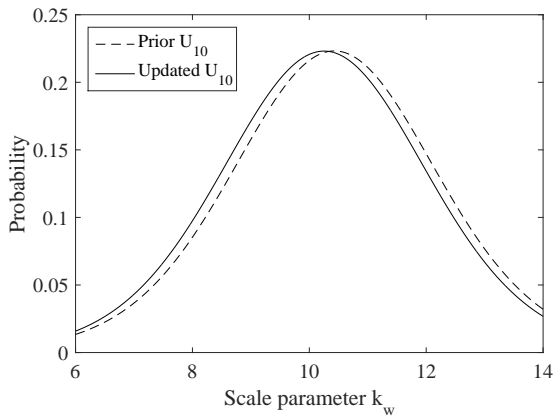
Keep the 'design' shape parameter:



$$\hat{k}_w = [10.005 \quad 9.993 \quad 8.176] \text{ m/s}$$



## Methodology: Update Wind Speed Distribution



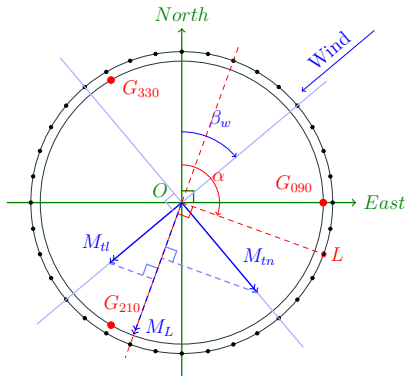
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Application

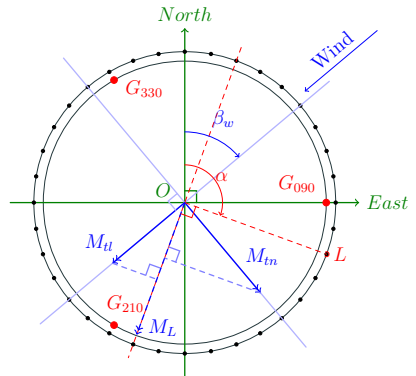
# Application

- 3 MW offshore wind turbine



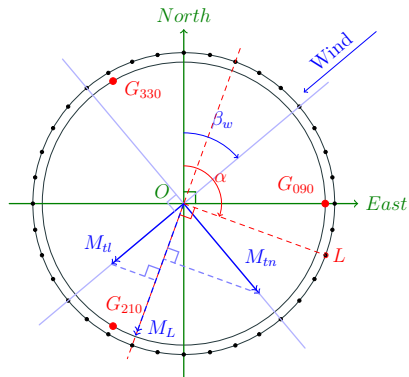
# Application

- 3 MW offshore wind turbine
- Monopile, diameter of 5.2 m



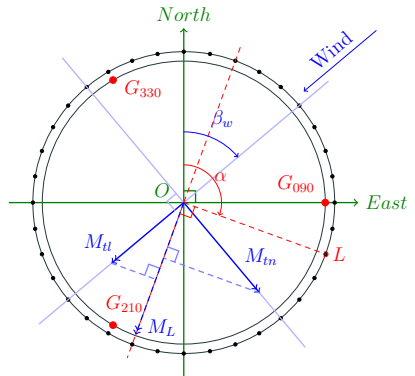
# Application

- 3 MW offshore wind turbine
- Monopile, diameter of 5.2 m
- Optical strain sensors



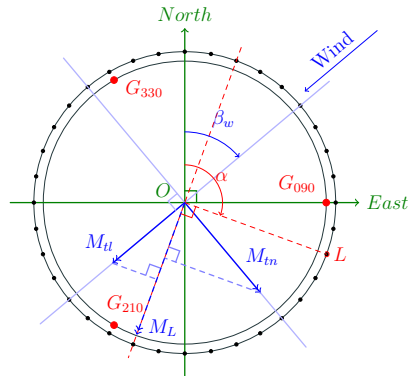
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- 3 MW offshore wind turbine
- Monopile, diameter of 5.2 m
- Optical strain sensors
- Before construction: 15 years of wind data



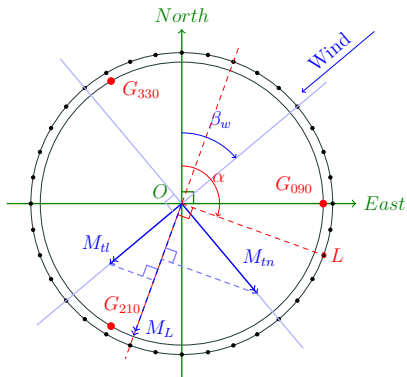
# Application

- 3 MW offshore wind turbine
- Monopile, diameter of 5.2 m
- Optical strain sensors
- Before construction: 15 years of wind data
- After construction: 3 years of wind and wave data + 1 year strain data (concurrently measured with the wind)



# Application

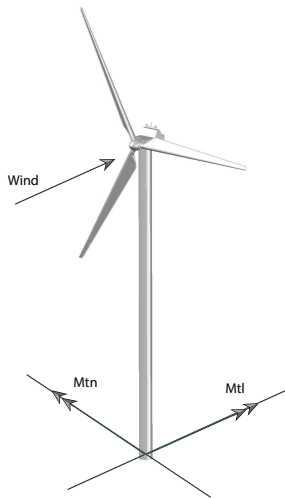
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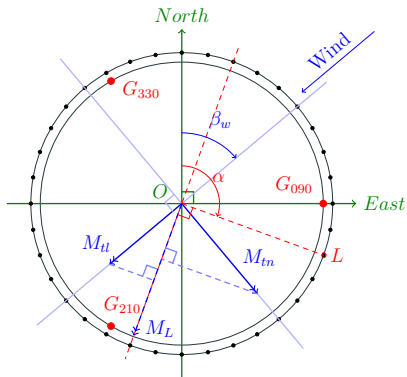
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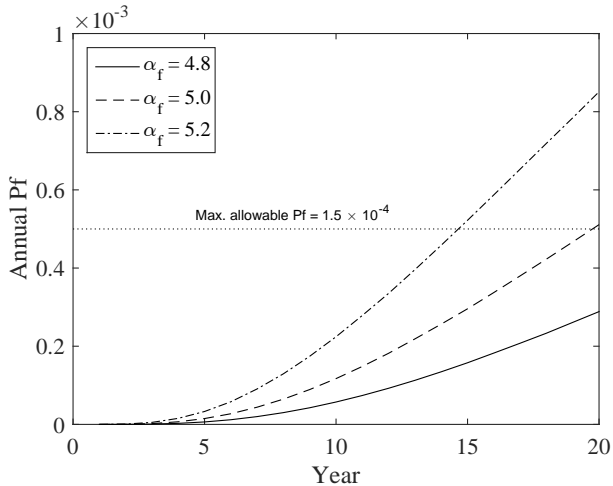


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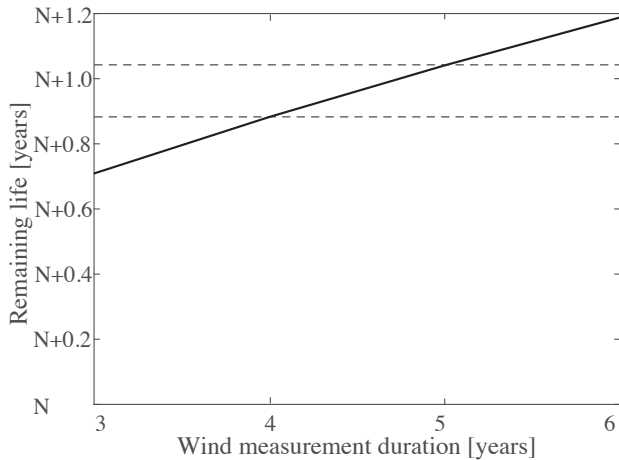
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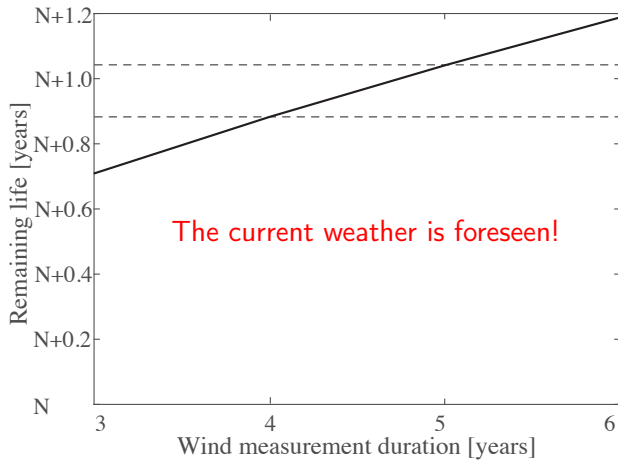
## Application: Estimating Remaining Fatigue Life



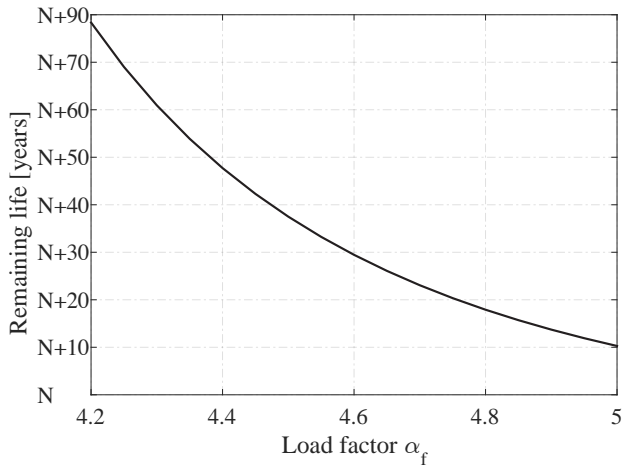
## Application: Results & Discussion



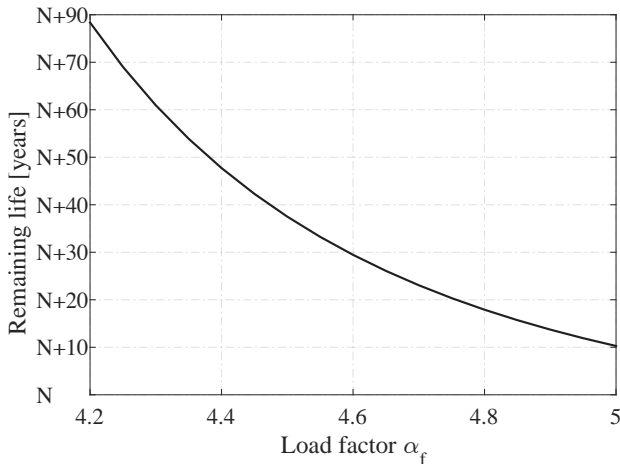
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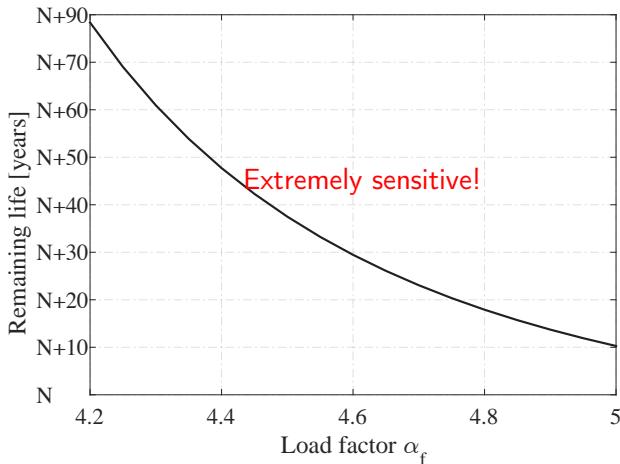
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$\alpha_f$  is deterministic!

represents: Stress Concentration Factor (SCF) at hot-spots, or  
Stress Extrapolating Factor for unmeasured locations

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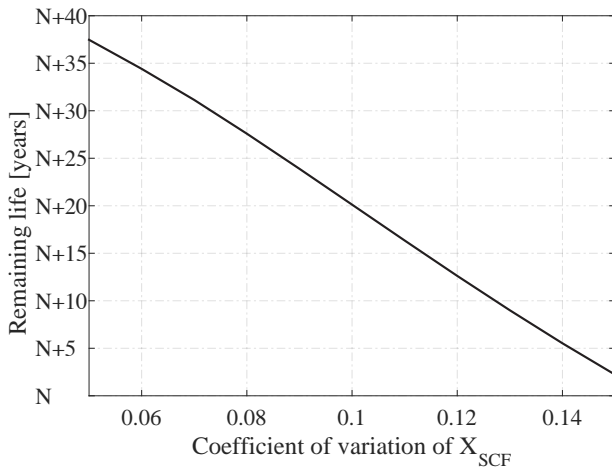


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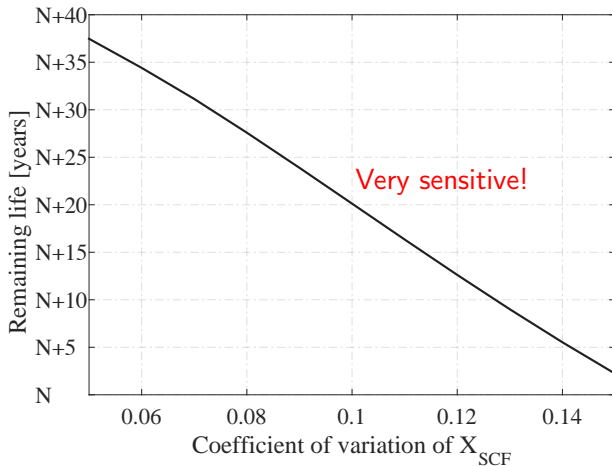
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$X_{SCF}$  is stochastic part of SCF!

Coefficient of variation = std./mean

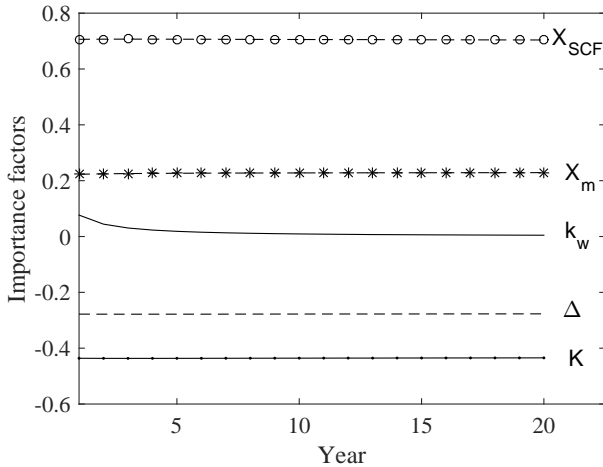
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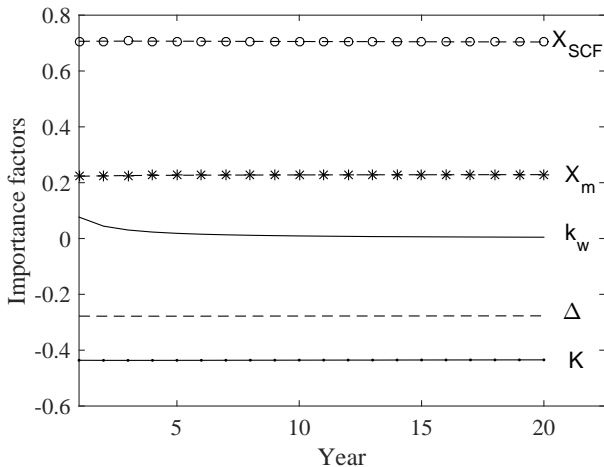
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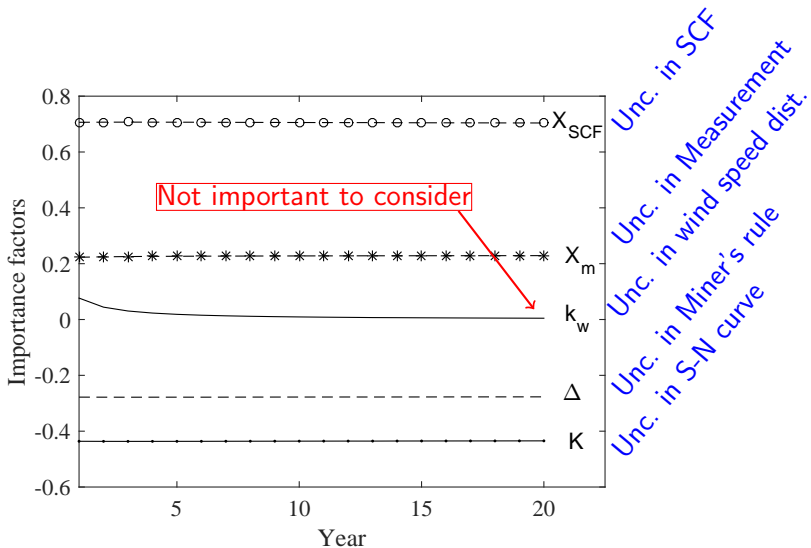


## Application: Results & Discussion



Unc. in SCF  
Unc. in Measurement  
Unc. in wind speed dist.  
Unc. in Miner's rule  
Unc. in S-N curve

## Application: Results & Discussion



Occurrence of Weather Conditions in Updating Considering  
Monitoring

Motivation & Literature Review

Methodology

Application

# Conclusion on Using Occurrence of Weather Conditions in LSF

## Gap:

How to perform reliability assessment of existing offshore wind turbine support structures using directly the Occurrence of Weather Conditions (wind and wave)?

# Conclusion on Using Occurrence of Weather Conditions in LSF

---

## Disadvantages

- Assumed that fatigue damage caused by each Weather Condition is constant.
- Depends on the stress extrapolation method to derive stress for locations that is not measured.

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## Advantages

- Fast
- Less uncertainty than a time domain analysis.



## Conclusion

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FAD in Updating Considering Inspection

Motivation & Literature Review

FAD Gives Higher Failure Probability Values

FAD in Updating

Occurrence of Weather Conditions in Updating Considering Monitoring

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## Conclusion: Future Works

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## Conclusion: Future Works

- Combining two types of new information in RBI,
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- Load extrapolation for other types of OWT support structures,
- Quantifying uncertainty of load extrapolation methods,
- Considering the random process of the peak tensile stress in calculating failure probability

# Message Objective

Using **Failure Assessment Diagram** and **Occurrence of Weather Conditions** in the Limit State Function improves the accuracy of the **Updated Failure Probability**.





## Practical Implications

$P_f$  results are higher when  $K_{mat}$  is included in the LSF of  $a_c \Rightarrow$  it needs to consider to be conservative.

## Practical Implications

The peak tensile stress affects the safety state of any crack size  $\Rightarrow$  the time when a high peak tensile stress occurs is important. This is a **first passage time** problem where the random process of the peak tensile stress first encounters a threshold.

This is a challenge of considering the fracture toughness criterion.

FAD approach predicts higher  $P_f$  values when the applied peak tensile stress is larger than 65% the yield strength, in comparison to the LSF using  $(a_c, K_{mat}) \Rightarrow$  the use of FAD should be recommended for reliability assessment of existing offshore structures with high stress (designed to the limit, corroded, damage tolerant design)

## Practical Implications

When FAD approach is utilized, the uncertainties in yield and ultimate strengths are important because they define the region of plastic collapse  $\Rightarrow$  they should be investigated to improve the reliability of the structure.

## Practical Implications

The information about cracks and intervention actions helps to improve our belief in the structural safety (reducing the probability of failure). It is the basis to optimizing inspection plans to reduce the O&M costs of offshore wind turbines.

## Practical Implications

An imperfect repair leads to a higher failure probability than a perfect repair.  $\Rightarrow$  an imperfect repair should be considered in the decision tree for a conservative inspection plan.

Updating using Monitoring data: the impact of the year-to-year variation of the annual mean wind speed becomes negligible after 4 years.  
⇒ it can be ignored in the LSF to reduce significantly calculation time and give a chance to consider a finer discretized Occurrence of Weather Conditions.

## Practical Implications

The value of the predicted remaining fatigue life obtained from the present methodology can be useful for decision making to down-rate, curtail, or extend the lifetime of the wind turbine support structures.



## Practical Implications

To apply the proposed method for locations where strain gauges cannot be installed, a load extrapolation method is needed, which in turn requires a good calibrated finite element model. A model uncertainty is also needed in the LSF.

## Practical Implications

vbox Histogram of measured strain is distorted by high frequencies of small strain cycles, by considering the corresponding accumulated fatigue damage during fitting process, the weighting factor of each bin can be modified to preserve total fatigue damage.

## The use of Miner's Rule

Fatigue damage accumulated by one load cycle is calculated as:

$$D_i = \frac{1}{N_i} = \frac{1}{K_c} S_i^m$$

For a large number of stress cycle, the expected fatigue damage can be estimated as:

$$\begin{aligned} E[D_i] &= \frac{1}{K_c} \sum_0^{\infty} S_i^m P(S_i) \\ &= \frac{1}{K_c} \int_0^{\infty} S^m f(s) ds \end{aligned}$$

If the stress-range is Weibull distributed  $(k, \lambda)$ , the expected fatigue damage per cycle becomes:

$$E[D_i] = \frac{1}{K_c} k^m \Gamma\left(\frac{m}{\lambda} + 1\right)$$

# Why Link Strain with Wind and Wave?

People may ask:

- Why not to use measured strain directly?

# Why Link Strain with Wind and Wave?

People may ask:

- Why not to use measured strain directly?
- Why not to use wind and wave to get strain from a Finite Element Model and then quantify the model uncertainty using the measured strain?

## Why Link Strain with Wind and Wave?

Why not to use measured strain directly?

- You may need to measure strain for the whole lifetime.

# Why Link Strain with Wind and Wave?

Why not to use measured strain directly?

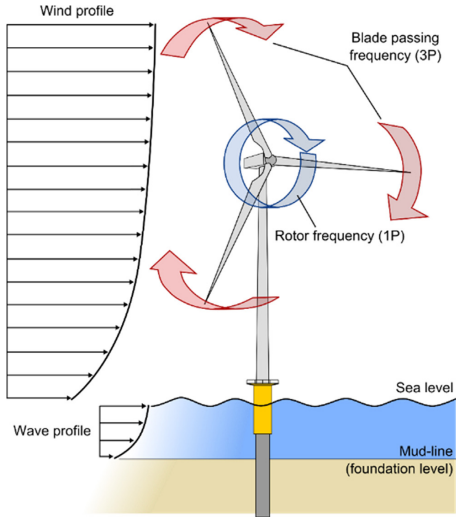
- You may need to measure strain for the whole lifetime.
- In offshore structures, there are locations where you cannot install strain gauges.

## Why Link Strain with Wind and Wave?

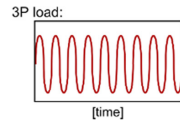
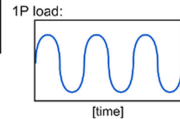
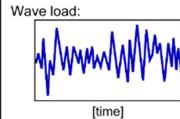
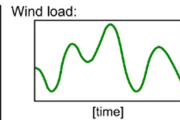
Why not to use wind and wave to get strain from a Finite Element Model and then quantify the model uncertainty using the measured strain?



# Why Link Strain with Wind and Wave?



Typical waveforms for the different type of loads:



## Why Link Strain with Wind and Wave?

Why not to use wind and wave to get strain from a Finite Element Model and then quantify the model uncertainty using the measured strain?

- You consider one irrelevant uncertainty more than the method proposed in this thesis.
- You take a lot of time to perform time domain analyses.

# Joint Distribution of Wind and Wave

The probability of occurrence of  $jk l^{th}$  bin which is used to link to fatigue damage is:

$$P(U_{10,j}, H_{s,k}, T_{z,l}) = \int \int \int f(U_{10}, H_s, T_z) dw \, dh \, dt$$

this integration need to be calculated numerically.

If only  $U_{10}$  is considered in the bin, the probability of  $j^{th}$  bin becomes:

$$\begin{aligned} P(U_{10,j}) &= F_W(a_j \leq U_{10} < b_j; k_w, \lambda_w) \\ &= \exp\left(-\left(\frac{a_j}{k_w}\right)^{\lambda_w}\right) - \exp\left(-\left(\frac{b_j}{k_w}\right)^{\lambda_w}\right) \end{aligned}$$

## Joint Distribution of Wind and Wave

$$f(U_{10}, H_s, T_z) = f(U_{10}) \times f(H|U_{10}) \times f(T_z|H_s, U_{10})$$

where:

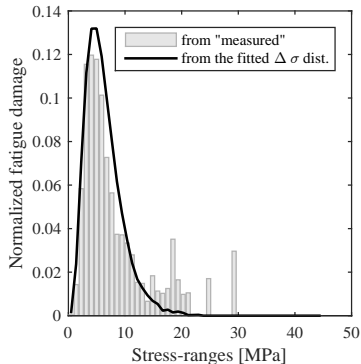
$f(U_{10})$  marginal distribution of the 10-minute mean wind speed, Weibull ( $k_w, \lambda_w$ ),

$f(H_s|U_{10})$  conditional distribution of significant wave height given  $U_{10}$ , Weibull (scale =  $func(U_{10})$ , shape =  $func(U_{10})$ ),

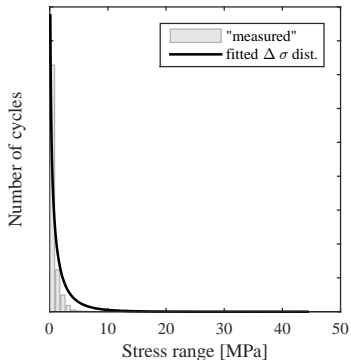
$f(T_z|H_s, U_{10})$  conditional distribution of mean wave period given  $H_s$  and  $U_{10}$ , Lognormal (mean =  $func(H_s, U_{10})$ , std =  $func(H_s, U_{10})$ ).

# The use of Miner's Rule

The assumption that stress-ranges follow a Weibull distribution is not perfect!



**(a)** Distribution of fatigue damage



**(b)** Distribution of stress-range

**Figure 1:** Fitting stress-range in wind class [0 to 5 m/s]

# Total Fatigue Damage

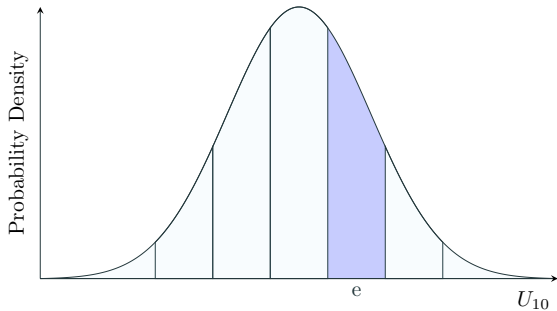
$$D = \sum_{i=1}^T \sum_{j=1}^{n_{U_{10}}} \sum_{k=1}^{n_{H_s}} \sum_{l=1}^{n_{T_p}} \frac{\alpha_f^m}{K_c} k_{s,jkl}^m \Gamma \left( \frac{m}{\lambda_{s,jkl}} + 1 \right) \times \dots$$

$$P(U_{10,j}, H_{s,k}, T_{p,l} | k_{w,i}) \frac{n_{c,jkl}}{n_{m,jkl}} n_m^*$$

- $n_{cj} = n_{U_{10}} \times n_{H_s} \times n_{T_z}$  is total number of bins;
- $n_{c,jkl}$  is number of stress cycles in the bin number  $ijkl$ ;
- $n_{m,jkl}$  is number of oceanographic records in the bin number  $ijkl$ ;
- $n_m^* = \sum_{j=1}^{n_{cl}} n_{m,j}$  is total of observed oceanographic data per year;
- $P(U_{10,j}, H_{s,k}, T_{p,l} | k_{w,i})$  is the probability of the bin  $ijkl$  given the scale parameter of the wind speed distribution  $k_{w,i}$  in the  $i^{th}$  year.

# Total Fatigue Damage

$$D = T \sum_{e=1}^{n_b} \frac{\alpha_f^m}{K_c} k_{s,e}^m \Gamma \left( \frac{m}{\lambda_{s,e}} + 1 \right) n_{c,e}$$



## Equality vs. Inequality Events

Equality: when crack is measured a certain value. Not considered here because it is a very small failure probability problem, MCS is not suitable.



## Importance Factors

The 'importance factor' of a random variable is a measure of the sensitivity of the reliability index to randomness of that random variable at the design point.

The 'importance factors' offer a way to rank the importance of the input variables with respect to the failure event of the welded joint.

The vector of 'importance factors' is denoted as  $\alpha$ ,

$$\alpha = -\frac{\nabla g(\mathbf{x})}{|\nabla g(\mathbf{x})|} \quad (1)$$

where  $\nabla g(\mathbf{x})$  is the gradient vector of the limit state function at the design point  $\mathbf{x}$ , which is assumed to exist, as shown in Eq.(2):

$$\nabla g(\mathbf{x}) = \left( \frac{\partial g}{\partial x_1}(\mathbf{x}), \quad \dots, \quad \frac{\partial g}{\partial x_n}(\mathbf{x}) \right) \quad (2)$$

## Minimum Number of Stress Cycles

- Weibull (scale =  $k$ , shape =  $\lambda$ ) of stress-range distributions in:
  - Case 1: wind speeds in bin 1 (5-10 m/s):  $k = 1.922$ ,  
 $\lambda = 0.6172$
  - Case 2: wind speeds in bin 2 (10-15 m/s):  $k = 4.2385$ ,  
 $\lambda = 0.7793$
  - Case 3: wind speeds in bin 3 (20-30 m/s):  $k = 9.408$ ,  
 $\lambda = 1.0774$
- SN curve:  $\log a_2 = 15.606$ ;  $\log a_1 = 11.764$ ;  $m_1 = 3$ ;  $m_2 = 5$

No. of cycles ( $n$ )	Case 1	Case 2	Case 3
$10^7$	5.5%	3.4%	1.4%
$5 \times 10^6$	7.3%	4.5%	1.7%
$10^6$	19.5%	9.3%	4.3%

**Table 1:** Error in fatigue damage

## Minimum wind measurement for design

15 years is not a long data set for design because, to estimate the 50-year return period wind speed, a minimum 20 years of data is required (Coles et al. 2001)