

**Modelling the excavation damaged zone in claystone with strain localisation using coupled second gradient model and the influence of gallery ventilation**

**B. Pardoen - S. Levasseur - F. Collin**

Université de Liège – ArGEnCo

1. FRACTURING EVIDENCES
2. MATERIAL BEHAVIOUR
3. NUMERICAL RESULTS FOR GALLERY EXCAVATION
4. CONCLUSIONS AND OUTLOOKS

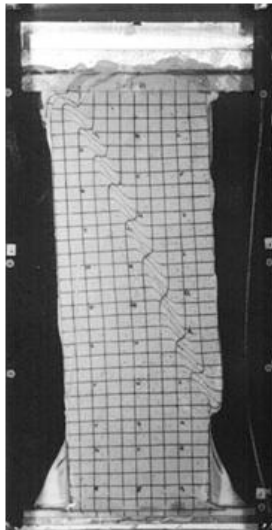
1. **FRACTURING EVIDENCES**
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# 1. Fracturing evidences

## Mechanical fracturing :

### Small scale (laboratory) :

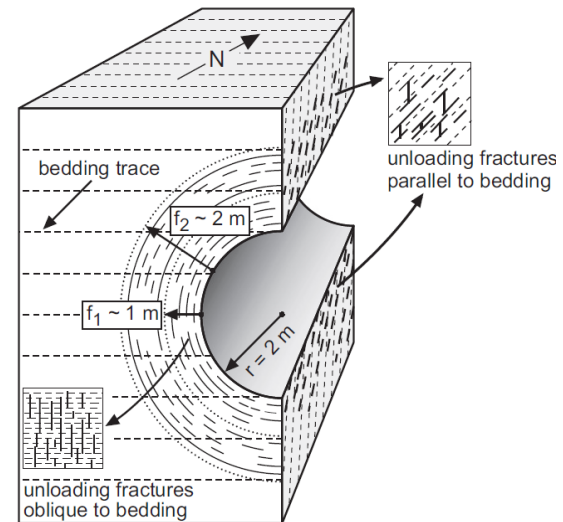
- Loading
- Strain localization in shear band mode
- Rupture



Mokni and Desrues, 1999

### Large scale (galleries) :

- Excavation
- Stress redistribution
- Fracturing / Damage zone

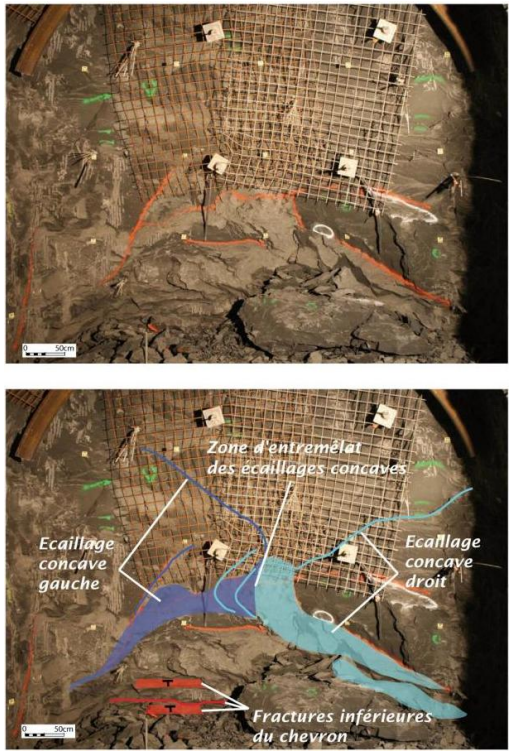
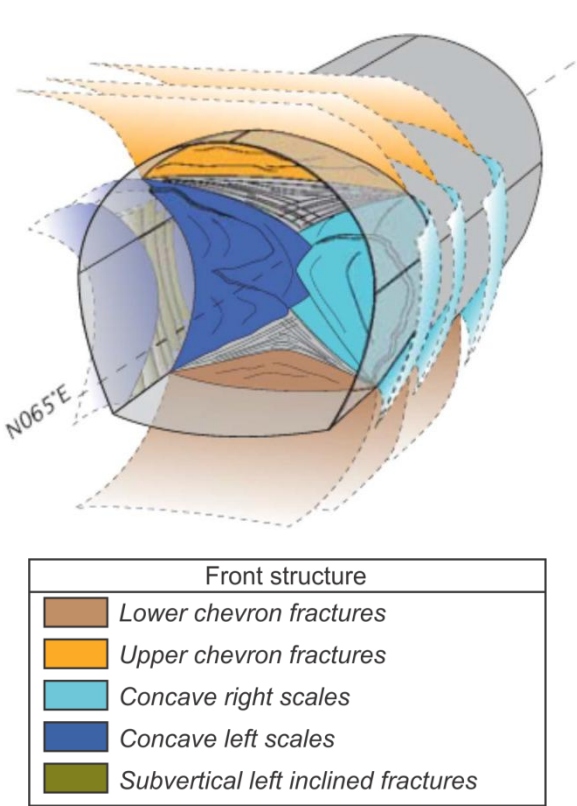


Bossart et al., 2002

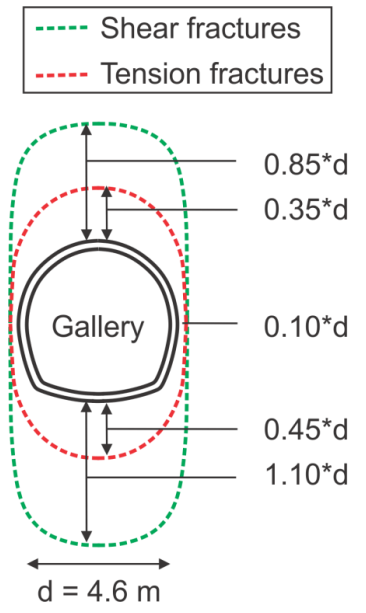
# 1. Fracturing evidences

## In situ evidences :

Observations and measurements (ANDRA URL, GED Gallery, Cruchaudet *et al.* 2010a).



Front of the section GED1002



Major issues : prediction of the extension and fracturing structure.

- study - damaged zone development with shear strain localisation,
- influence of the gallery ventilation.

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## 2. Material behaviour

### Material :

Partially saturated porous media, 2 phases : solid + water

Homogenised mixture mass density :  $\rho = \rho_s (1-n) + \rho_w n S_{r,w}$

Bishop's stress definition :  $\sigma_{ij} = \sigma'_{ij} - b S_{r,w} p_w \delta_{ij}$

### Solid phase behaviour :

Solid grain density variation :

$$\frac{\partial \rho_s}{\partial t} = \frac{\rho_s}{K_s} \left[ (b-n) S_{r,w} \frac{\partial p_w}{\partial t} - \frac{\partial \sigma'}{\partial t} \right]$$

Biot's coefficient :

$$b = 1 - \frac{K_0}{K_s}$$

Porosity variation:

$$\frac{\partial n}{\partial t} = (1-n) \left[ \frac{1}{\rho_s} \frac{\partial \rho_s}{\partial t} + \frac{\partial \varepsilon_v}{\partial t} \right]$$

### Fluid phase behaviour :

Fluid mass flow (advection, Darcy) :

$$m_{w,i} = -\rho_w \frac{k k_{r,w}}{\mu_w} \left( \frac{\partial p_w}{\partial x_i} + \rho_w g_i \right)$$

Water retention and permeability curves (Van Genuchten's model) :

$$S_{r,w} = \left[ 1 + \left( \frac{p_c}{P_r} \right)^N \right]^{-M} \quad k_{r,w} = \sqrt{S_{r,w}} \left[ 1 - (1 - S_{r,w}^{1/M})^M \right]^2$$

## 2. Material behaviour

### Mechanical model – 1<sup>st</sup> gradient model :

The constitutive mechanical law for the clayey rock is :

- a non-associated elasto-plastic internal friction model, with a Drucker-Prager yield surface

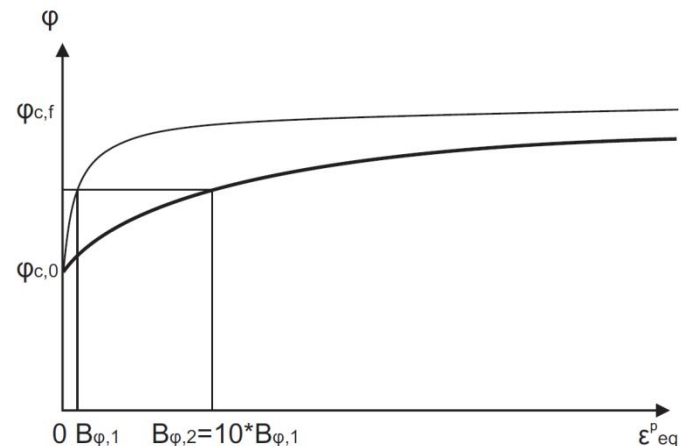
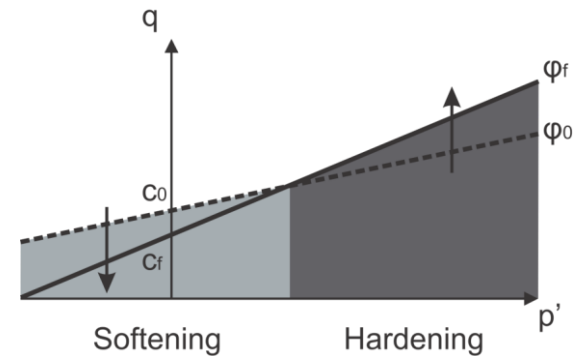
$$F \equiv \Pi_{\hat{\sigma}} - m \left( I_{\sigma} + \frac{3c}{\tan \phi_c} \right) = 0$$

$$I_{\sigma} = \sigma_{ij} \delta_{ij} \quad \Pi_{\hat{\sigma}} = \sqrt{\frac{1}{2} \hat{\sigma}_{ij} \hat{\sigma}_{ij}} \quad m = \frac{2 \sin \phi_c}{\sqrt{3}(3 - \sin \phi_c)}$$

- allowing hardening/softening of  $\phi$  and/or  $c$  as a function of the Von Mises equivalent plastic strain  $\varepsilon_{eq}^p$

$$\varepsilon_{eq}^p = \sqrt{\frac{2}{3} \hat{\varepsilon}_{ij}^p \hat{\varepsilon}_{ij}^p}$$

$$\phi_c = \phi_{c0} + \frac{(\phi_{cf} - \phi_{c0}) \varepsilon_{eq}^p}{B_{\phi} + \varepsilon_{eq}^p}$$





### Modelling of strain localisation – 2<sup>d</sup> gradient model : (Chambon *et al.*, 1998 and 2001)

The continuum is enriched with microstructure effects. The kinematics include the classical one (macro) and the microkinematics (Toupin 1962, Mindlin 1964, Germain 1973, Collin *et al.*, 2006).

$$\text{Balance equations : } \int_{\Omega} \left( \sigma_{ij} \frac{\partial u_i^*}{\partial x_j} + \underline{\Sigma_{ijk} \frac{\partial^2 u_i^*}{\partial x_j \partial x_k}} \right) d\Omega = \int_{\Omega} G_i u_i^* d\Omega + \int_{\Gamma_{\sigma}} (\bar{t}_i u_i^* + \underline{\bar{T}_i D u_i^*}) d\Gamma$$
$$\int_{\Omega} \left( \frac{\partial M}{\partial t} p_w^* - m_i \frac{\partial p_w^*}{\partial x_i} \right) d\Omega = \int_{\Omega} Q p_w^* d\Omega + \int_{\Gamma_q} \bar{q} p_w^* d\Gamma$$

$\Sigma_{ijk}$  is the double stress, which need an additional constitutive law : linear elastic law (Mindlin, 1964) defined as a function of the (micro) second gradient of displacement field  $u_i^*$  :

$$\tilde{\Sigma}_{ijk} = f \left( D, \underline{\frac{\partial^2 u_i^*}{\partial x_j \partial x_k}} \right)$$

It depends only on one elastic parameter  $D$ . The shear band width is proportional to this parameter. (Chambon *et al.*, 1998, Kotronis *et al.*, 2007).

## 2. Material behaviour

### Mechanical and hydraulic parameters :

Synthesis of Callovo-Oxfordian claystone parameters from (Charlier et al. 2012)

Symbol	Name	Value	Unit
$k_{hor}$	Horizontal intrinsic water permeability	$4 \cdot 10^{-20}$	$m^2$
$k_{vert}$	Vertical intrinsic water permeability	$1.33 \cdot 10^{-20}$	$m^2$
$n_0$	Porosity	0.18	-
$M$	Van Genuchten coefficient	0.33	-
$N$	Van Genuchten coefficient	1.49	MPa
$P_r$	Van Genuchten parameter	15	$Pa^{-1}$

Symbol	Name	Value	Unit
$E$	Young's modulus	4000	MPa
$\nu$	Poisson's ratio	0.3	-
$b$	Biot's coefficient	0.6	-
$\rho_s$	Specific mass	2300	$kg/m^3$
$\psi$	Dilatancy angle	0.5	$^\circ$
$\varphi_0$	Initial friction angle	10	$^\circ$
$\varphi_f$	Final friction angle	20	$^\circ$
$B_\varphi$	Friction angle hardening coefficient	0.002	-
$c_0$	Initial cohesion	3	MPa
$c_f$	Final cohesion	0.3	MPa
$B_c$	Cohesion softening coefficient	0.003	-
$D$	Second gradient elastic parameter	5000	N

$\longrightarrow$  Friction angle hardening  
 $\longrightarrow$  Cohesion softening

1. FRACTURING EVIDENCES
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  - **2D**
  - 3D
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### 3. Numerical results for gallery excavation – 2D

#### Numerical modelling (LAGAMINE-ULg) :

By symmetry: quarter of the gallery

HM modelling in 2D plane strain state.  
Gallery radius = 2.3 m.

Anisotropy (Andra URL) :

→ hydraulic permeability anisotropy

$$k_{\text{hor/vert}} = 4 \cdot 10^{-20} / 1.33 \cdot 10^{-20} [\text{m}^2]$$

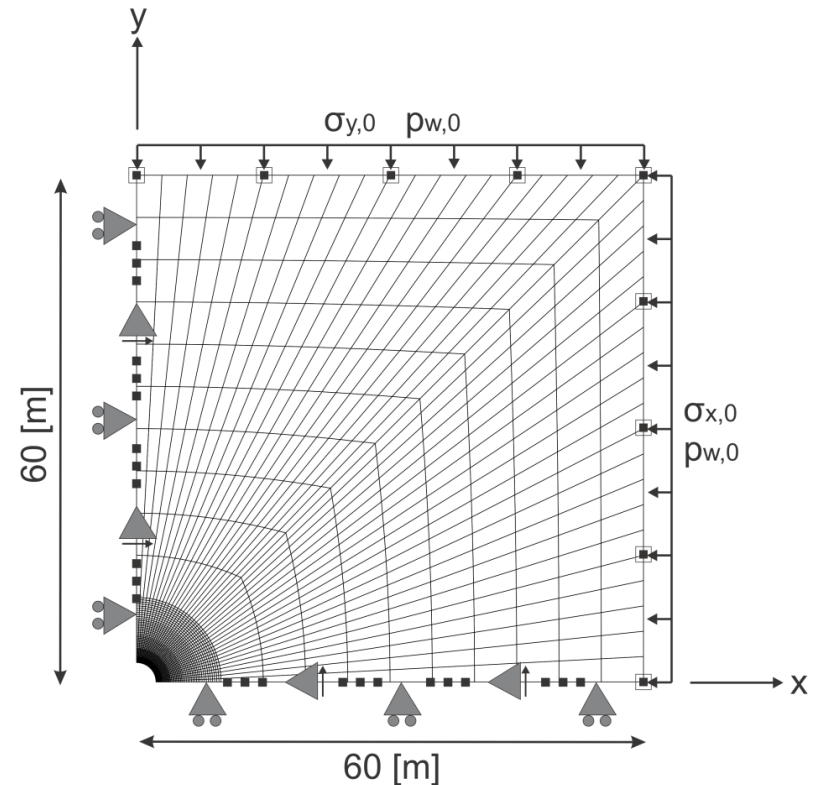
→ initial anisotropic stress state

$$p_{w,0} = 4.5 [\text{Mpa}]$$

$$\sigma_{y,0} = \sigma_{z,0} = 12 [\text{Mpa}]$$

$$\sigma_{x,0} = 15.6 [\text{MPa}]$$

- ▣ Constant pore water pressure ( $p_{w,0}$ )
- ← Constant total stress ( $\sigma_{y,0} / \sigma_{x,0}$ )
- ▶ Constrained displacement perpendicular to the boundary
- ▲ Constrained normal derivative of the radial displacement (Zervos *et al.* 2001)
- Impervious boundary



### 3. Numerical results for gallery excavation – 2D

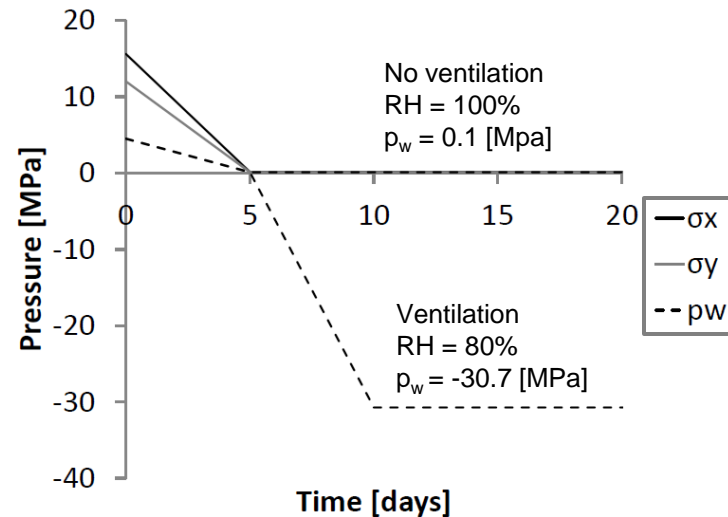
#### Numerical modelling (LAGAMINE-ULg) :

Pressure at gallery wall :

→ gallery excavation :  $\sigma_x$  and  $\sigma_y$  decrease

→ gallery ventilation : water phases equilibrium (Kelvin's law)

$$RH = \frac{P_v}{P_{v,0}} = \exp\left(\frac{-s M_v}{RT \rho_w}\right)$$

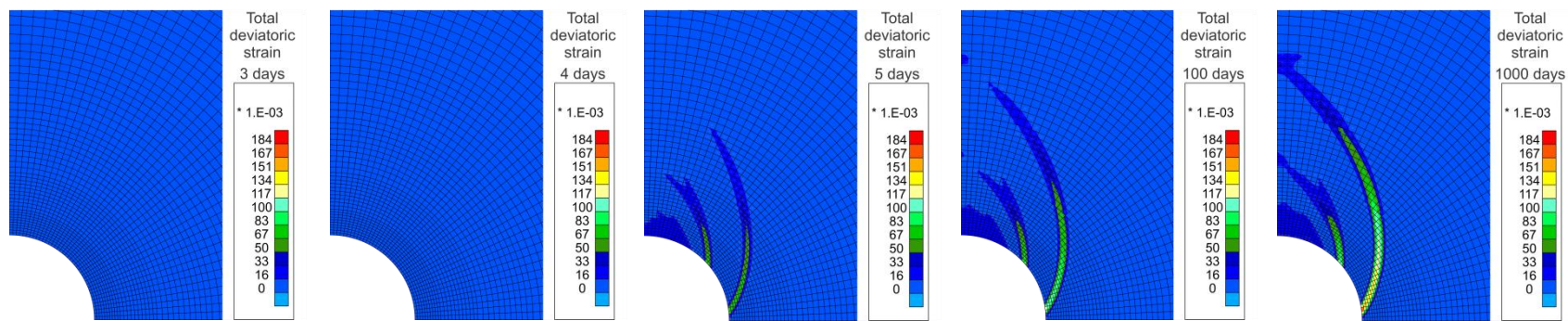


# 3. Numerical results for gallery excavation – 2D

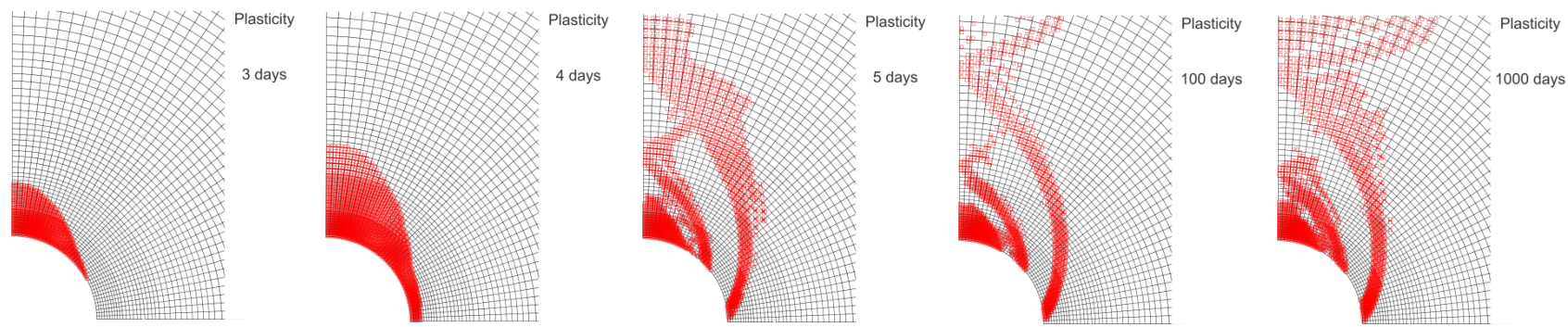
**Localisation zone :** no ventilation

End of excavation  
↓

Total deviatoric strain



Plasticity

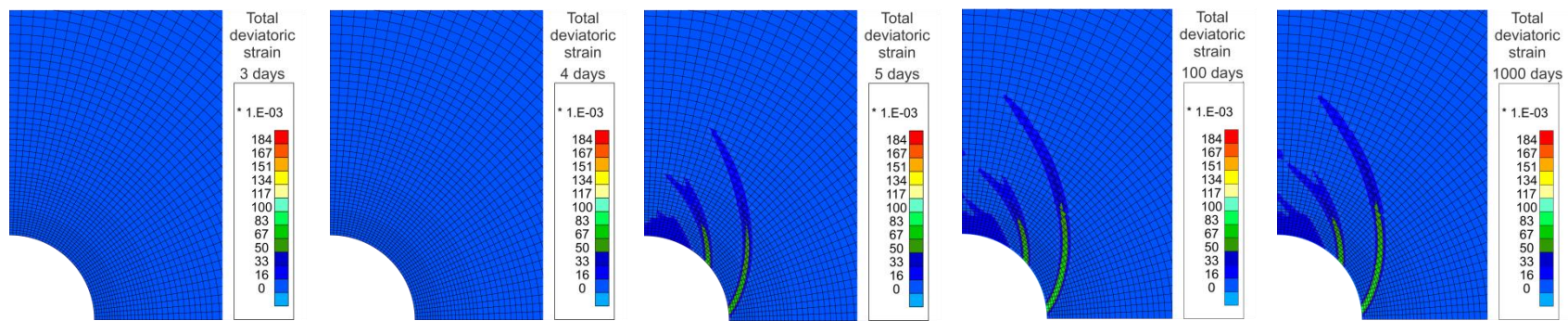


# 3. Numerical results for gallery excavation – 2D

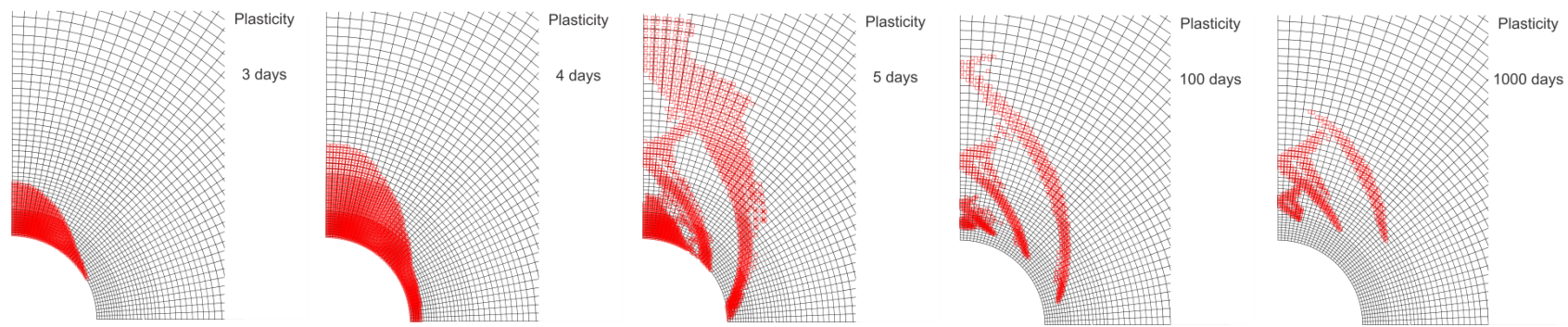
Localisation zone : ventilation

End of excavation  
↓

Total deviatoric strain



Plasticity





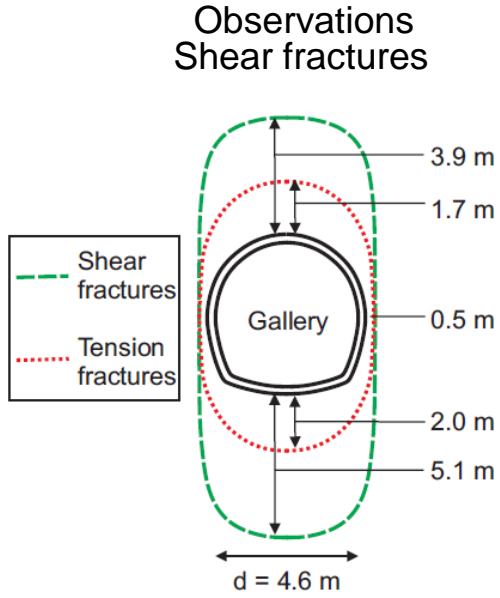
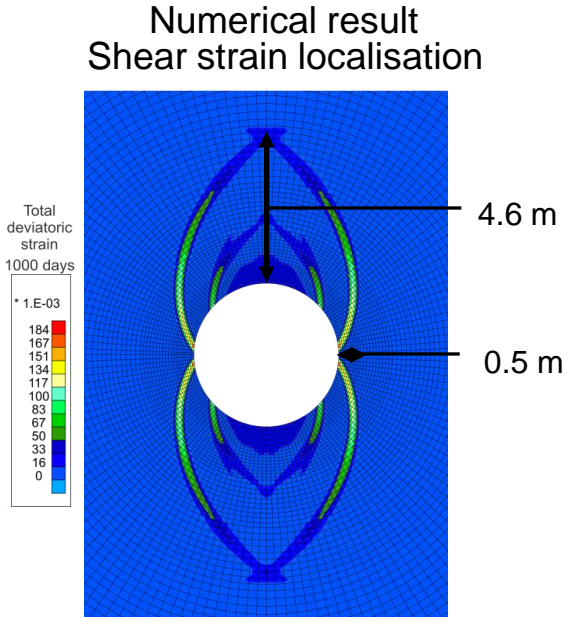
# 3. Numerical results for gallery excavation – 2D

## Localisation zone :

Chevron fracture pattern corresponding to in situ observations (Cruchaudet *et al.*, 2010b).

Chevron fractures concentrated above the gallery because of the anisotropic stress state.

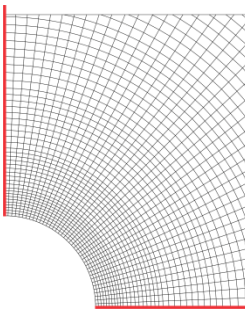
The extension of the excavation damaged zone obtained numerically corresponds fairly well to the *in situ* experimental measurements of shear fractures.



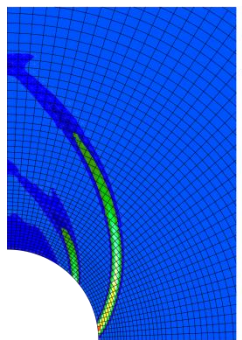
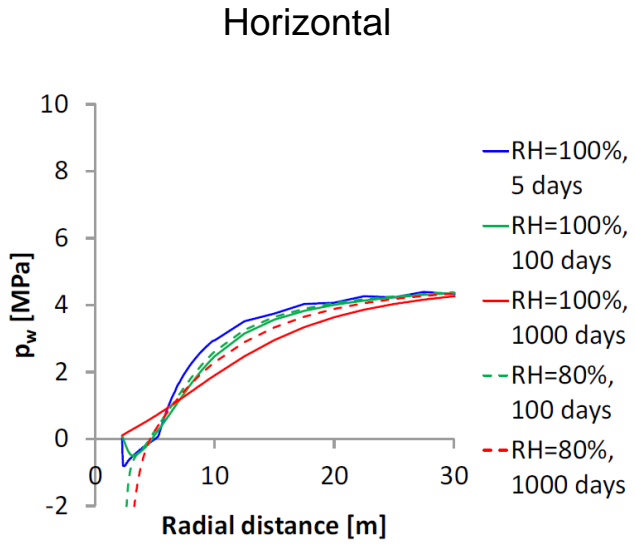
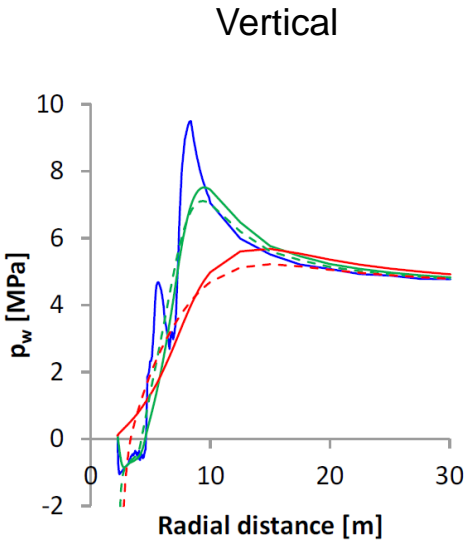


# 3. Numerical results for gallery excavation – 2D

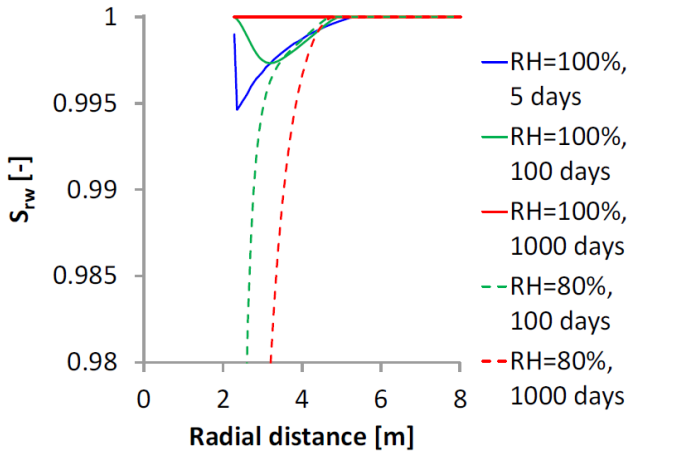
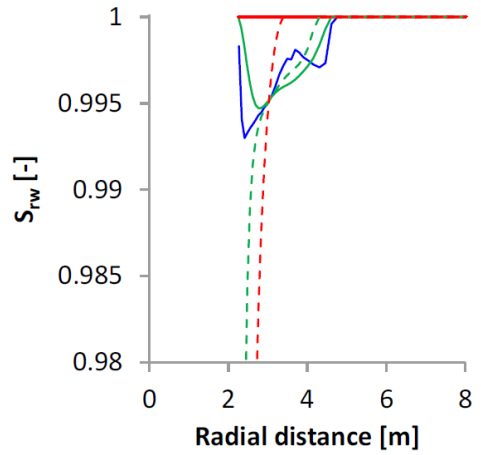
## Cross sections :



Pore water pressure

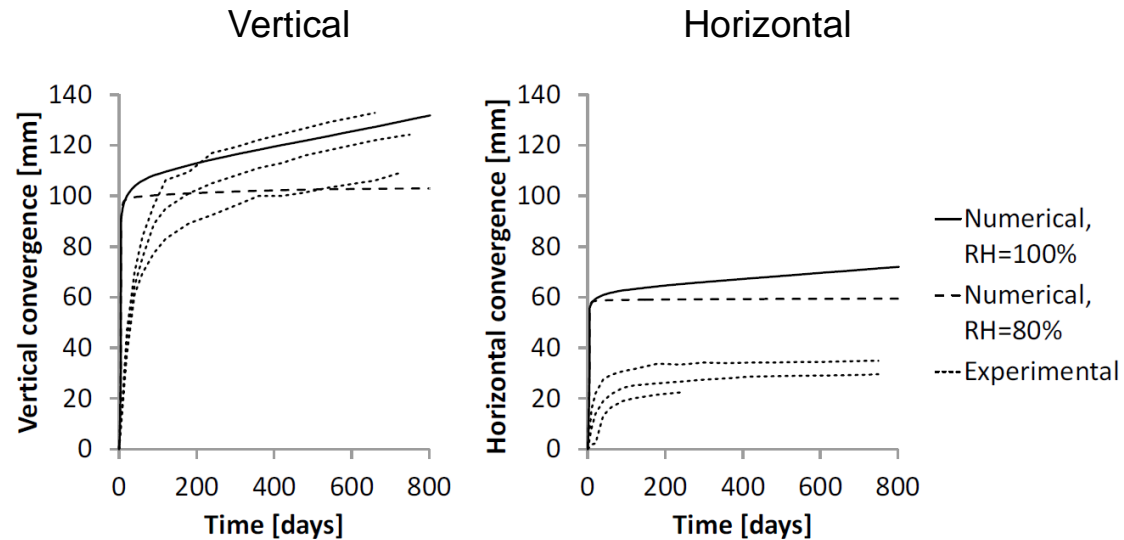


Degree of saturation



### 3. Numerical results for gallery excavation – 2D

#### Convergence :



Important during the excavation and keeps increasing afterwards.

Anisotropic convergence because of the shear strain localisation bands located above the gallery.

Experimental results from a gallery of the Andra URL (Cruchaudet *et al.*, 2010b).  
Good matching in the vertical direction for the modelling without ventilation.

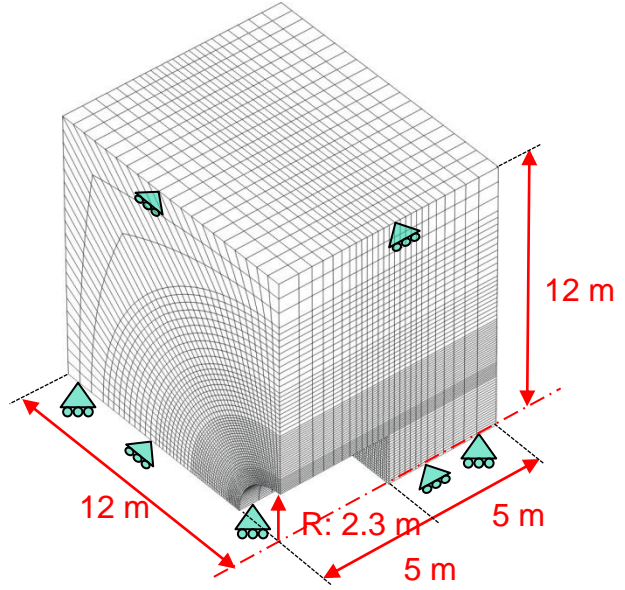
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  - **3D**
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# 3. Numerical results for gallery excavation – 3D

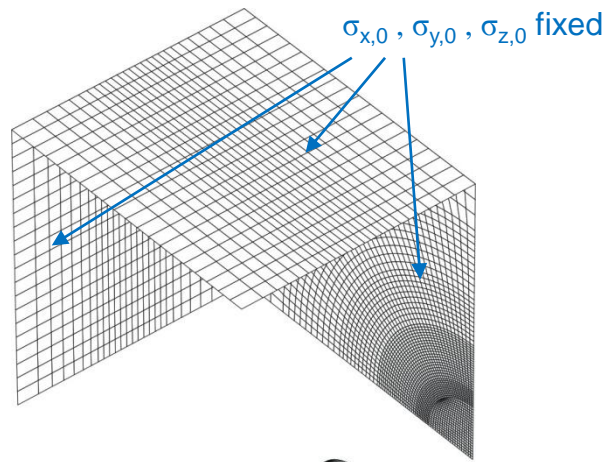
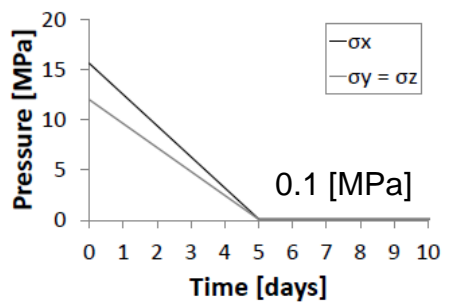
## Numerical modelling (LAGAMINE-ULg) :

Mechanical modelling in 3D state.  
 Classical FE, no second gradient !

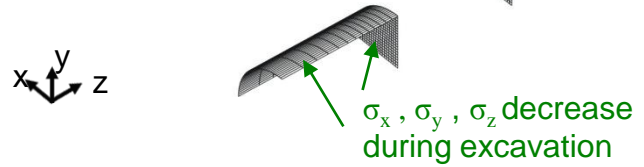
Initial anisotropic stress state (Andra URL) :  
 $\sigma_{y,0} = \sigma_{z,0} = 12$  [MPa]  
 $\sigma_{x,0} = 15.6$  [MPa]



Identical excavation :



Mesh : 410 591 nodes  
 60 320 volume elements with 20 nodes  
 4 480 elements of stress imposition  
 752 226 equations  
 8 days of calculation



# 3. Numerical results for gallery excavation – 3D

## Equivalent deformation $\epsilon_{eq}$ :

$\epsilon_{eq}$  during boring :

3 days  
 $\sigma/\sigma_0 = 0.40$

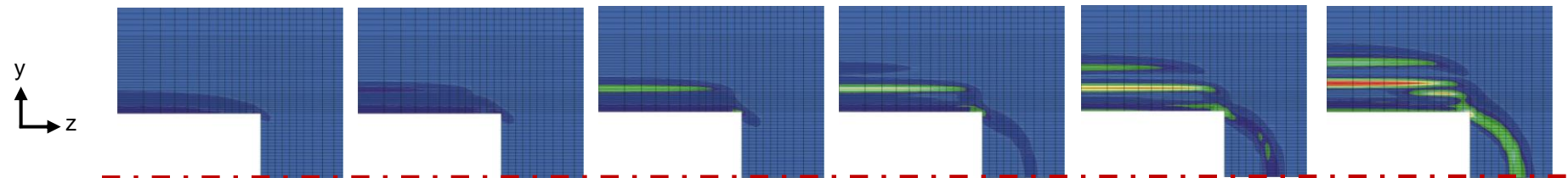
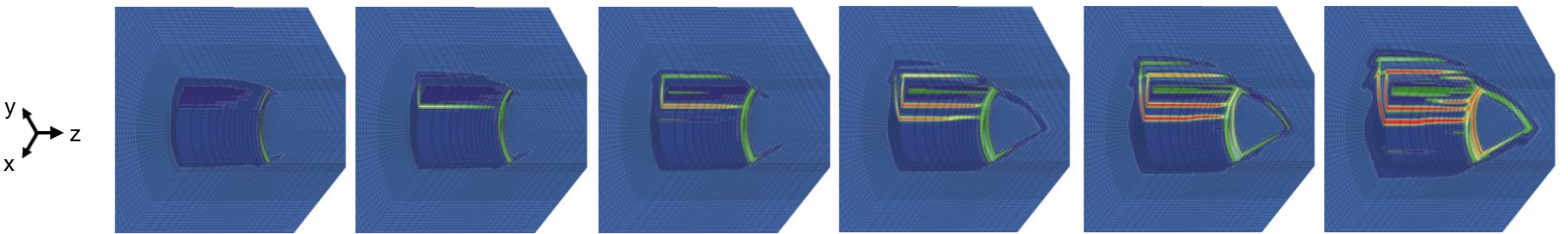
3.25 days  
 $\sigma/\sigma_0 = 0.35$

3.5 days  
 $\sigma/\sigma_0 = 0.30$

3.75 days  
 $\sigma/\sigma_0 = 0.25$

4 days  
 $\sigma/\sigma_0 = 0.20$

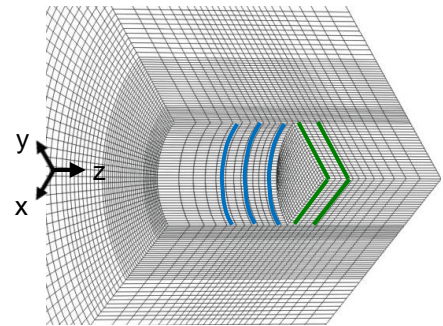
4.25 days  
 $\sigma/\sigma_0 = 0.15$



# 3. Numerical results for gallery excavation – 3D

## Equivalent deformation $\epsilon_{eq}$ :

$\epsilon_{eq}$  for 4.25 days of excavation ( $\sigma/\sigma_0 = 0.15$ ) :



$z < 0$  : excavation zone  
 $z = 0$  : gallery end  
 $z > 0$  : rock mass

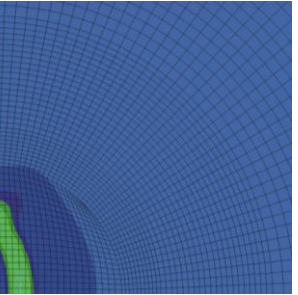
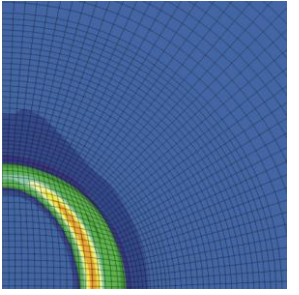
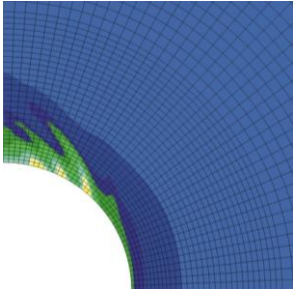
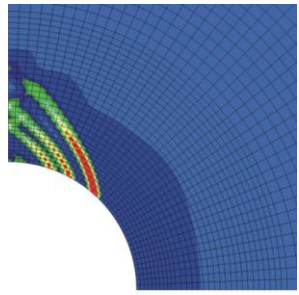
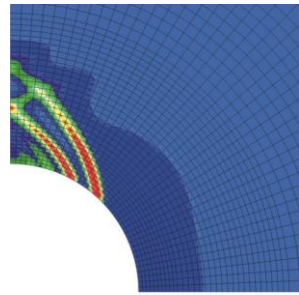
$z = -2.25m$

$z = -1.25m$

$z = -0.25m$

$z = +0.25m$

$z = +1.25m$



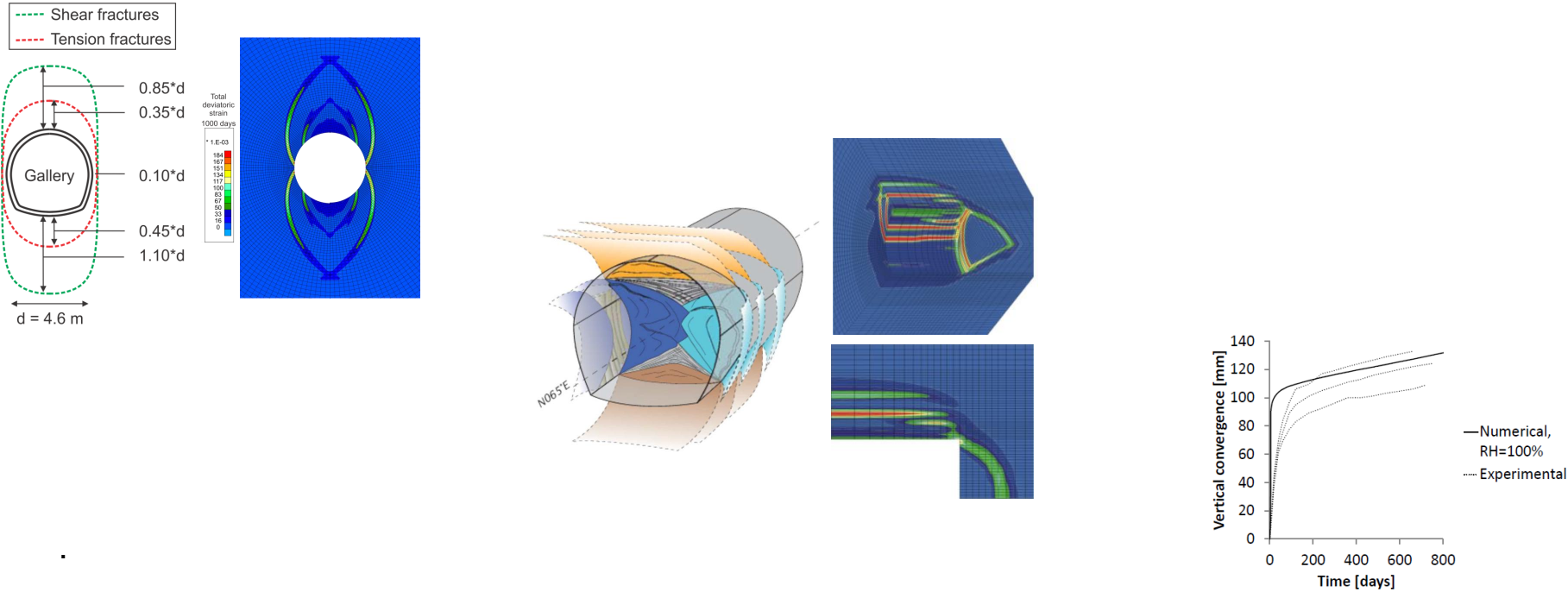
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# 4. Conclusions and outlooks

Damaged zone → strain localisation zone similar to *in situ* measurements

→ modelling provide information about the rock structure and evolution within this zone, as observed *in situ*.



→ need for a better definition of :

- the rock anisotropy
- the properties changes
- the hydromechanical coupling



- Bossart P., Meier P. M., Moeri A., Trick T., Mayor J.C., "Geological and hydraulic characterisation of the excavation disturbed zone in the Opalinus Clay of the Mont Terri Rock Laboratory", *Engineering Geology*, vol. 66, n° 1-2, 2002, p. 19–38.
- Chambon R., Caillerie D., El Hassan N., "One-dimensional localisation studied with a second grade model", *European Journal of Mechanics - A/Solids*, vol. 17, n° 4, 1998, p. 637–656.
- Chambon R., Caillerie D., Matsushima T., "Plastic continuum with microstructure, local second gradient theories for geomaterials : localization studies", *International Journal of Solids and Structures*, vol. 38, 2001, p. 8503-8527.
- Chambon R., Crochepeyre S., Charlier R., "An algorithm and a method to search bifurcation points in non-linear problems", *International Journal for Numerical Methods in Engineering*, vol. 51, 2001, p. 315–332.
- Charlier R., Collin F., Pardoën B., Talandier J., Radu J.P., Gerard P., "An unsaturated hydro-mechanical modelling of two in-situ experiments in callovo-oxfordian argillite". *Engineering Geology*, 2012, submitted.
- Collin F., Chambon R., Charlier R., " A finite element method for poro mechanical modelling of geotechnical problems using local second gradient models", *International Journal for Numerical Methods in Engineering*, vol. 65, n° 11, 2006, p. 1749–1772.
- Cruchaudet M., Noiret A., Talandier J., Armand G., "Expérimentation SDZ – Bilan de la mise en place de l'instrumentation et des premières mesures à fin mars 2010 – Centre de Meuse/Haute-Marne", rapport interne n° D.RP.AMFS.09.0087, 2010a, ANDRA.
- Cruchaudet M., Noiret A., Talandier J., Gatmiri B., Armand G., "OHZ en GED : EDZ initiale et évolution", rapport interne n° D.RP.AMFS.11.0016, 2010b, ANDRA.
- Germain P., "The method of virtual power in continuum mechanics. Part 2 Microstructure", *SIAM Journal on Applied Mathematics*, vol. 25, 1973, p. 556-575.
- Mindlin R.D., "Micro-structure in linear elasticity", *Archive for Rational Mechanics and Analysis*, vol. 16, 1964, p. 51–78.
- Sieffert Y., Al Holo S., Chambon R., "Loss of uniqueness of numerical solutions of the borehole problem modelled with enhanced media", *International Journal of Solids and Structures*, vol. 46, 2009, p. 3173–3197.
- Toupin R., "Elastic materials with couple–stresses", *Archive for Rational Mechanics and Analysis*, vol. 11, 1962, p. 385–414.
- Zervos A., Papanastasiou P., Vardoulakis I., "Modelling of localisation and scale effect in thick-walled cylinders with gradient elastoplasticity", *International Journal of Solids and Structures*, vol. 38, 2001, p. 5081-5095.

