

Transient behaviour of a suction caisson in sand: axisymmetric numerical modelling

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- 1 Context
- 2 Description of the case study
- 3 Results
- 4 Conclusions and perspectives

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Motivations

- 1 EU 2020 objectives (greenhouse gas, **renewable energy**, energy efficiency)

Motivations

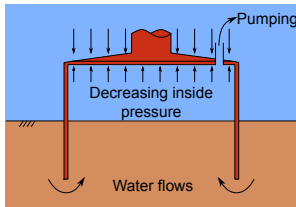
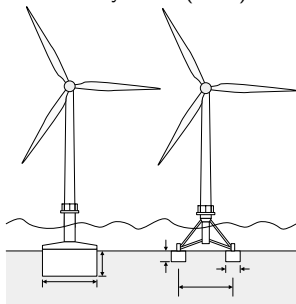
- 1 EU 2020 objectives (greenhouse gas, **renewable energy**, energy efficiency)
- 2 Basic working of soil-caisson system upon both monotonic and cyclic loading (serviceability)

Motivations

- 1 EU 2020 objectives (greenhouse gas, **renewable energy**, energy efficiency)
- 2 Basic working of soil-caisson system upon both monotonic and cyclic loading (serviceability)
- 3 Identifications of components of reaction : first step to the elaboration of a macro-element

Suction caissons for offshore foundations

Houlsby et al. (2005)



Offshore wind turbines specificities

- light structure
- high overturning moment

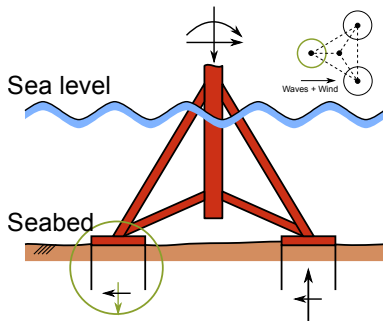
Suction caissons specificities

- hollow steel cylinder open towards the bottom
- extensively used as anchors in the North Sea
- monopod or tetra/tri-pod superstructure
- cheaply and quickly installed, reusable, Senders (2008)
- limited extension resistance by suction

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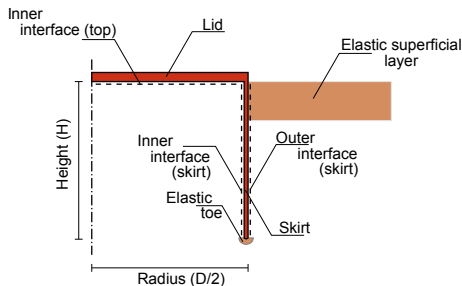
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Geometry

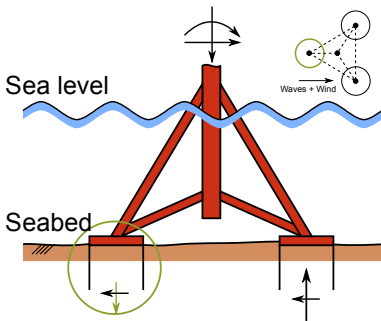


Published in Cerfontaine et al.
(2016), *Géotechnique*

Modelling (axisymmetric)

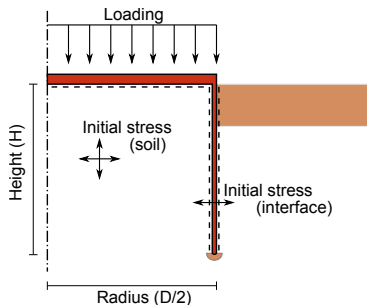


Geometry

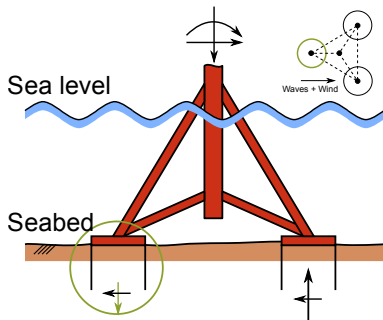


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Modelling (axisymmetric)



Geometry



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Size

$$D=7.8\text{m and } H=4\text{m}$$

Soil-steel friction coefficient

$$\mu = 0.5$$

Permeability

$$k= 5 \cdot 10^{-12}\text{m}^2$$

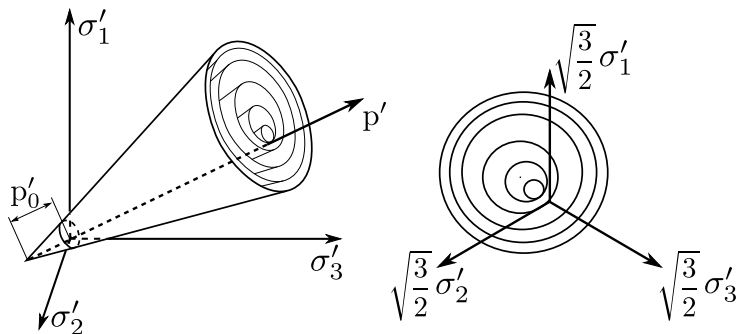
Coefficient of lateral earth pressure at rest

$$K_0 = 1.0$$

Porosity

$$n= 0.36$$

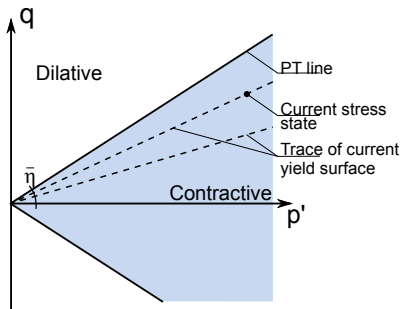
Prevost model for cohesionless soils - Kinematic hardening



After Elgamal (2003)

Implementation in LAGAMINE code published in Cerfontaine et al. (2014)
NUMGE2014 Proceedings

Prevost model for cohesionless soils - Volumetric behaviour



Non-associated plastic volumetric behaviour

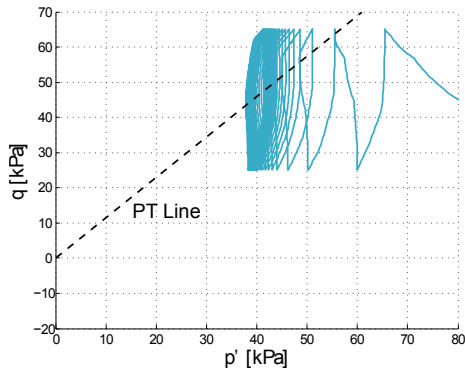
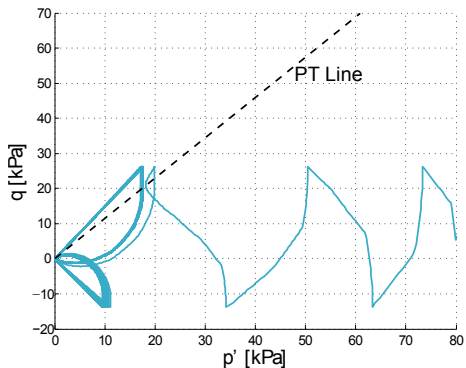
$$\dot{\epsilon}_v^p = \frac{1}{3} \cdot \frac{\eta^2 - \bar{\eta}^2}{\eta^2 + \bar{\eta}^2} \cdot \dot{\lambda}$$

- $\eta = q/p'$
- $\dot{\lambda}$ continuous plastic multiplier
- $\bar{\eta}$ phase transformation ratio, Ishihara (1975)

Very simple (only 1 param.)
 \Rightarrow satisfactory to a 1st approx.

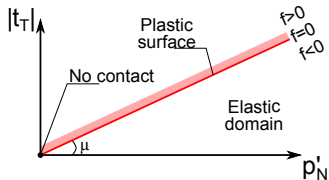
Cyclic triaxial tests (Lund Sand, $D_r = 90\%$, Ibsen & Jakobsen (1996))

Two distinct behaviours from two initial deviatoric stress invariants

Full calibration process published in Cerfontaine (2014), *PhD thesis*

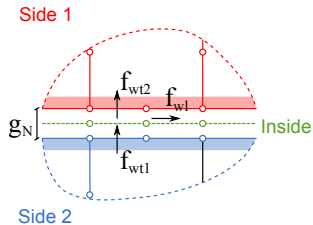
Hydro-mechanically coupled interface element

Mechanical behaviour



+ Penalty method

Flow behaviour



Couplings

■ Effective stress

■ Storage

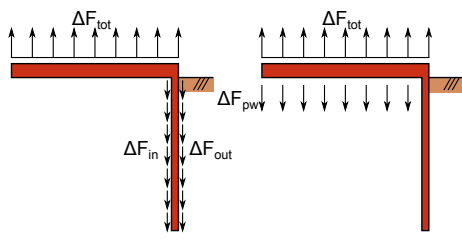
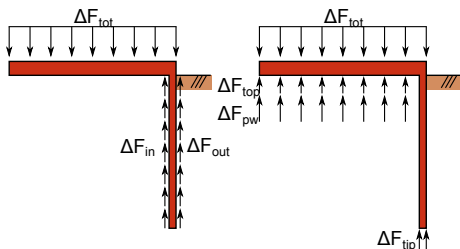
■ Permeability

Published in Cerfontaine et al. (2015) *Computers and Geotechnics*

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Reaction of the caisson to applied vertical load



Resistance to compressive load

ΔF_{tot}

- ΔF_{in} , inner friction ;
- ΔF_{out} , outer friction ;
- ΔF_{pw} , pore water pressure (> 0) ;
- ΔF_{top} , top effective stress ;
- ΔF_{tip} , tip effective stress.

Resistance to extension load ΔF_{tot}

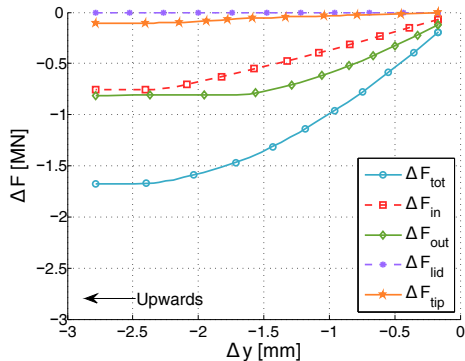
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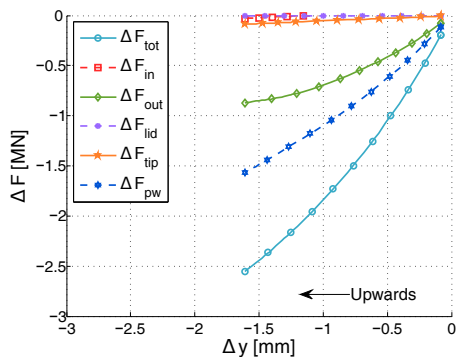
Monotonic extension simulations (load controlled)

Drained



- ΔF_{in} and ΔF_{out} bounded
- $\Delta F_{in} < \Delta F_{out}$ (unloading)

Partially drained (8kPa/s)



- ΔF_{in} and ΔF_{out} bounded
- $\Delta F_{in} < \Delta F_{out}$ (unloading)
- ΔF_{pw} increasing

Pore water pressure generation during extension

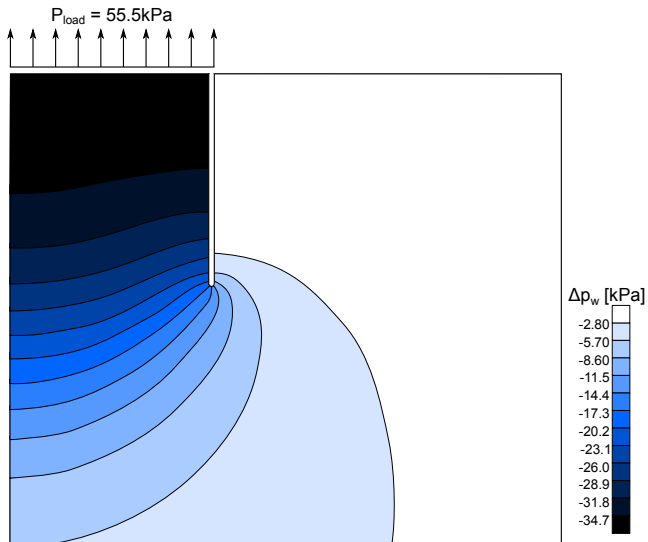
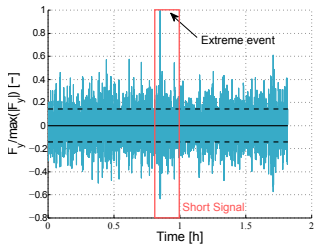


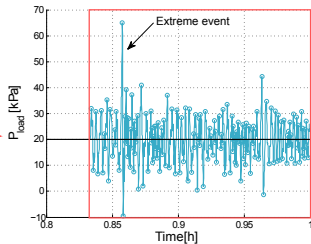
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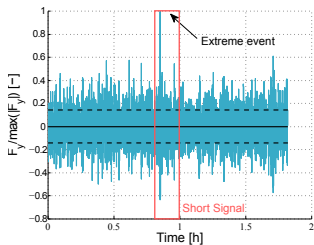
Pseudo-random and equivalent loadings



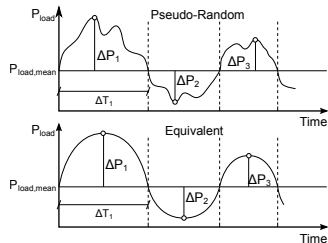
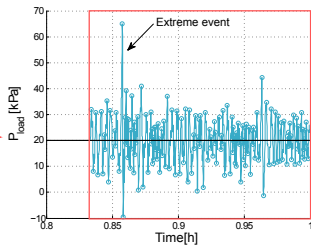
$$\Delta P_{\text{load}} = 45 \text{ kPa}$$
$$P_{\text{load,av}} = 20 \text{ kPa}$$



Pseudo-random and equivalent loadings

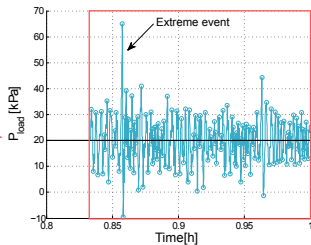
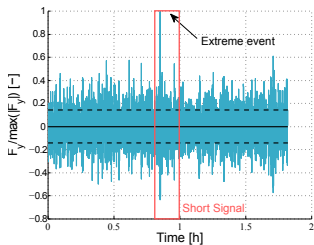


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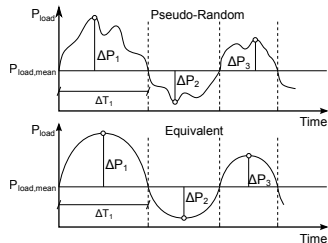


Half-cycle analysis

Pseudo-random and equivalent loadings

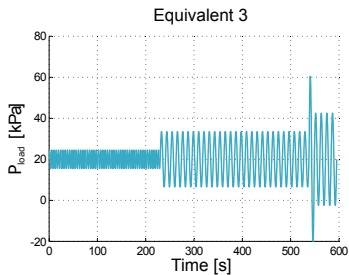
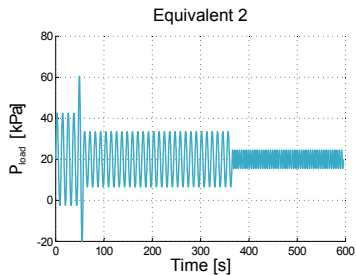
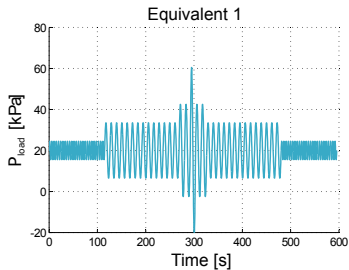
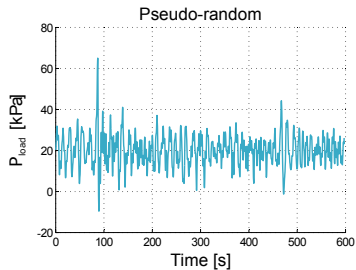


	Batch 1	Batch 2	Batch 3	Batch 4
Nb. cycles [-]	50	28	4	1
T [s]	4.6	11	11.6	11.1
ΔP [kPa]	4.5	13.5	22.5	40.5

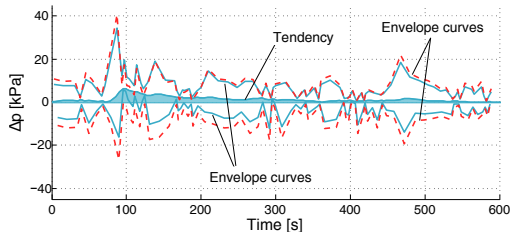
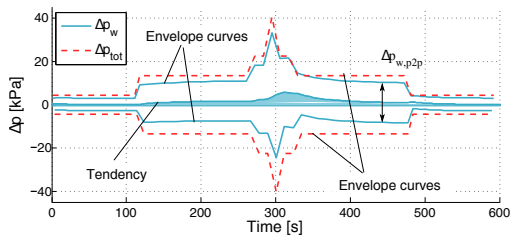


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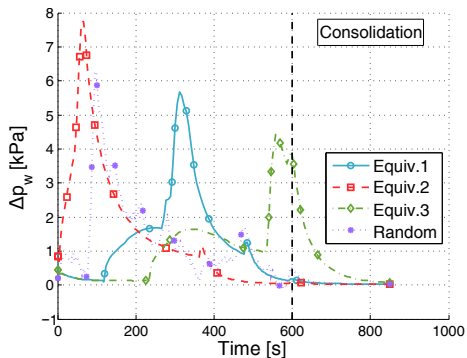


Cyclic partially drained behaviour

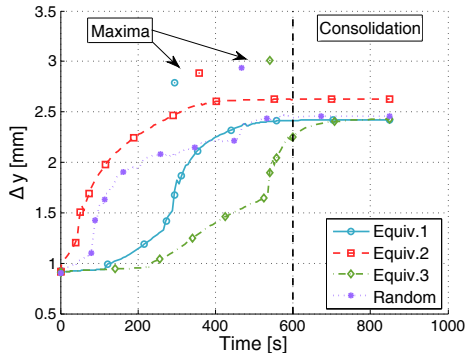


- Envelope and tendency curves : permanent and transient
- Loading mainly sustained by pore water pressure (PWP)
- Accumulation of PWP (max 5kPa)
- Highest accumulation during extreme event

Cyclic partially drained behaviour : displacement and PWP accumulation

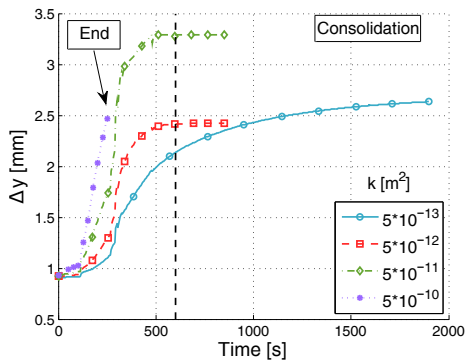


- Max PWP (extreme event sooner)
- Lowest PWP (random)
- Almost no effect of small cycles

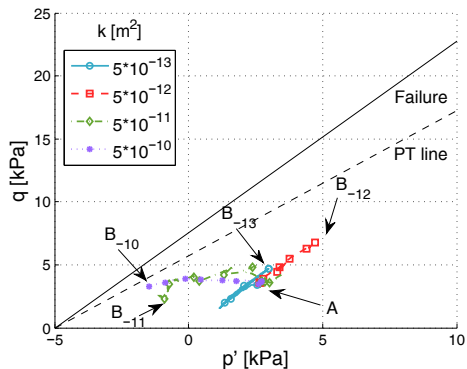


- Linear and non-linear parts
- High accumulation for extreme event
- All displacements converge

Cyclic partially drained behaviour : influence of permeability



- No linear trend with permeability evolution
- Local failure for the highest permeability (high effective stress variations)



- Different stress paths under the lid centre with permeability
- Decrease of p' due to PWP increase or contractancy

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- Coupled modelling of a suction caisson upon monotonic and cyclic loading
- Importance of the partially drained behaviour (both monotonic and cyclic)
- Identification of different modes of resistance not activated all at the same time
- Complex behaviour and accumulation of settlement during a short-time storm event

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- Perspectives
 - Calibration procedure and validation of the model
 - Elaboration of a macro-element
 - 3D simulations including lateral loading

References

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