

# Envelope approximation using equivalent static wind loads

N. Blaise<sup>1</sup> & V. Denoël<sup>1</sup>

<sup>1</sup>University of Liège (Belgium)

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Advanced COmputational Methods in ENgineering  
Liège, Belgium

# Introduction

## Nodal Dynamic Analysis

## Envelope approximation

## Conclusion

# Analysis of Large Structures under Random Excitations

- ▶ Engineering structures are submitted to random excitations
- ▶ Solve the equation of motion :

$$\mathbf{M}\ddot{\mathbf{x}}(t) + \mathbf{C}\dot{\mathbf{x}}(t) + \mathbf{K}\mathbf{x}(t) = \mathbf{p}(t)$$

- ▶ Responses of the structure (bending moment, stresses, etc) :

$$\mathbf{z}(t) = \mathbf{O}\mathbf{x}(t)$$

- ▶ Design of the structure using extreme values :

$$\mathbf{z}_{i,max} = \mu_{z_i} + g\sigma_{z_i}$$

## Equivalent analysis

- ▶ Next to that, we can compute **equivalent static wind loads (eswl)**,  $\mathbf{p}^e$ , for each extreme responses and...
- ▶ by a series of static analyses under these eswl we can recover each extreme responses :

$$\mathbf{z} = \mathbf{A}\mathbf{p}_i^e \text{ with } z_i = z_{i,max}$$

- ▶ Load-Response Correlation method
- ▶ Displacement-Response Correlation method

## Load-Response Correlation (LRC) method

- **Kasperski** (1991) solves the problem for a **static analysis**
- Covariance matrix between **z** and **p** is calculated by :

$$\mathbf{C}_{zp} = \mathbf{A}\mathbf{C}_p$$

- Following **Bayes' theorem**, the most probable wind load associated to a specific extreme response,  $z_{i,max}$ , is :

$$\mu_{p/z} = \frac{z\sigma_{zp}}{(\sigma_z)^2} \xrightarrow{z=g\sigma_z} \mathbf{p}_{z_{max}} = \mathbf{g}\rho_{zp}\sigma_p$$

$$\mathbf{p}_{z_{i,max}} = \mathbf{g}_i\rho_{p z_i}\sigma_p = \mathbf{p}_i^e$$

## Extension to take into account dynamic effects

- Nodal background and modal resonant analysis
  - ▶ Chen & Kareem (2001) : Equivalent static wind loads for buffeting response of bridges
  - ▶ Holmes (2002) : Effective static load distributions in wind engineering
- Full modal analysis
  - ▶ Fu (2007) : Equivalent Static Wind Loads on Long-Span Roof Structures ;

## Displacement-Response Correlation (DRC) method

- We solve the problem for a **full nodal dynamic analysis**
- Covariance matrix between  $\mathbf{z}$  and  $\mathbf{x}$  is calculated by :

$$\mathbf{C}_{\mathbf{z}\mathbf{x}} = \mathbf{O}\mathbf{C}_{\mathbf{x}}$$

- Following **Bayes' theorem**, the most probable displacements associated to a  $i$ th specific extreme response,  $z_{max,i}$ , is :

$$\mathbf{x}_i^e = g_i \rho_{\mathbf{x} z_i} \sigma_{\mathbf{x}}$$

- The **eswl** is simply computed as :

$$\mathbf{p}_i^e = \mathbf{K}\mathbf{x}_i^e$$

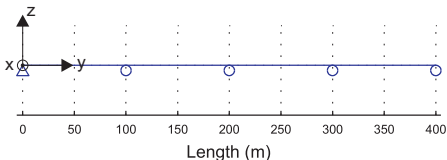
- Design of a structure is simplified with eswl → largely used in **design offices**
- Combination with other static loads is possible
- The **codes** are based on this concept



## Envelope approximation using eswl

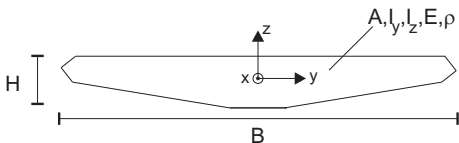
- ▶ When considering all these **static analyses** under each eswl **the envelope diagram is iteratively reconstructed...**
- ▶ Objective : use of a **minimum number of load cases**
  - ▶ which maybe can simultaneously targets several extreme responses
  - ▶ and in order to achieve an accepted underestimation level of the envelope diagram (fixed to **15%**)
- ▶ **Worked example** : Aerodynamic loading on a bridge.

- Four span bridge, simply supported



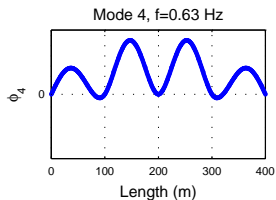
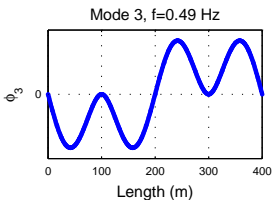
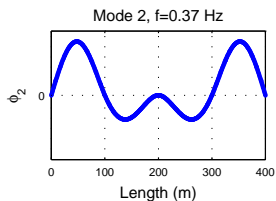
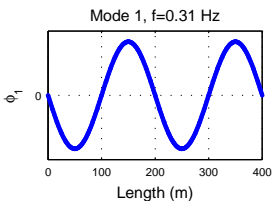
**Finite Element model** : classical beam elements ; 3DOF per node ; 12 elements by span ; 147 DOF.

- Characteristics of the deck



$B = 30 \text{ m}$  (width) ;  $H = 4 \text{ m}$  ; (height) ;  $\Omega = 1 \text{ m}^2$  (section) ;  $I_y = 10 \text{ m}^4$  (inertia) ;  $E = 1\text{e}9 \text{ N/mm}^2$  (Young's modulus) ;  $\rho = 2500 \text{ kg/m}^3$  (density)

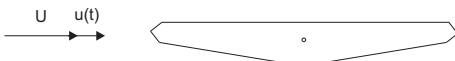
- Modal shapes :



- Rayleigh damping with  $\xi = 1\%$  for the 1st and the 4th modes.

## Aerodynamic loading on the deck

- **One-dimensionnal Gaussian velocity field** (quasi-steady) :



with  $U = 30$  m/s (mean velocity) and  $\sigma_u = 5$  m/s (standard deviation).

- **Ornstein-Uhlenbeck** for  $S_u(\omega)$
- **Coherence function** : decreasing exponential
- Only the **lift aerodynamic force** is considered as :

$$f_{tot}(t) \simeq \underbrace{\frac{1}{2} \rho C_L B I U^2}_{\mu_f} + \underbrace{\rho C_L B I U u(t)}_{f(t)}$$

Introduction

Nodal Dynamic Analysis

Envelope approximation

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## Nodal Dynamic Analysis

- ▶ **Mean contribution** by a **static linear analysis** :

$$\mu_x = \mathbf{K}^{-1} \mu_f$$

where  $\mathbf{K}$  is the stiffness matrix.

- ▶ **Variable contribution** by a **nodal stochastic analysis** :

$$\mathbf{S}_x(\omega) = \mathbf{H}(\omega) \mathbf{S}_f(\omega) \overline{\mathbf{H}}^T(\omega)$$

where  $\mathbf{H}(\omega)$  is the nodal transfer matrix and  $\mathbf{S}_{...}(\omega)$  symbolizes a PSD matrix

## Design of the structure

- ▶ **Spectral moment matrix** of the response :

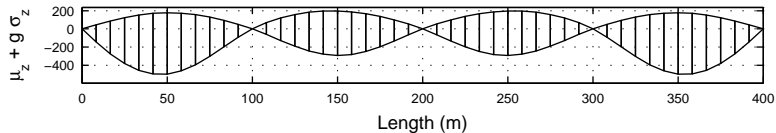
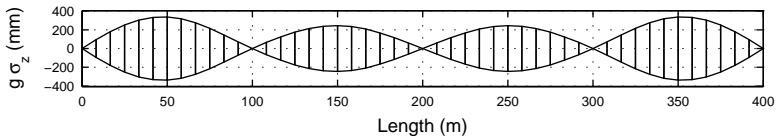
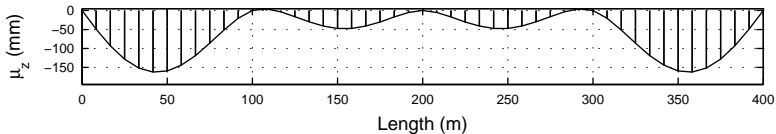
$$\mathbf{S}_z(\omega) = \mathbf{O} \mathbf{S}_x(\omega) \overline{\mathbf{O}}^T \int_{-\infty}^{+\infty} \dots |\omega|^i d\omega \rightarrow m_z^{(i)}$$

- ▶ **Mean extreme value** (using peak factor) for each response using the theory of extrema :

$$z_{max} = \mu_z + g \sigma_z$$

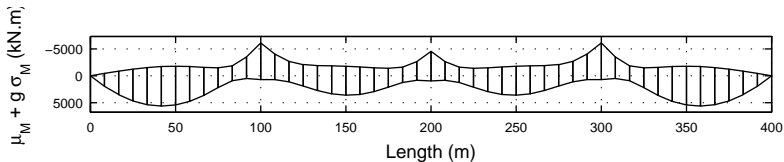
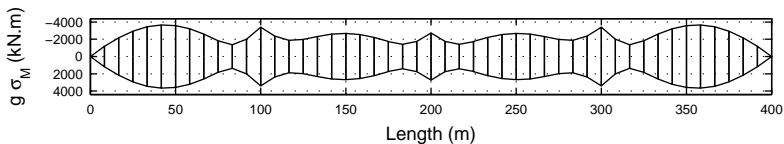
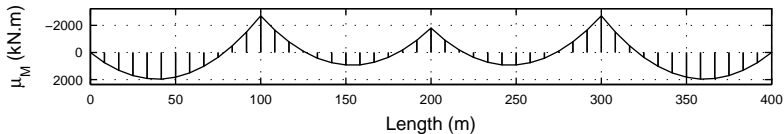
where  $g$  and  $\sigma_z$  are obtained from different orders of the spectral moment matrix

## Diagram of nodal displacements





## Diagram of bending moment



Introduction

Nodal Dynamic Analysis

**Envelope approximation**

Conclusion

## Option 0 : Universal eswl

- ▶ Possible to compute **one eswl** to recover all of the extreme bending moments but...



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Universal wind load distribution simultaneously  
reproducing largest load effects in all subject  
members on large-span cantilevered roof

A. Katsumura<sup>a</sup>, Y. Tamura<sup>b</sup>, O. Nakamura<sup>a</sup>

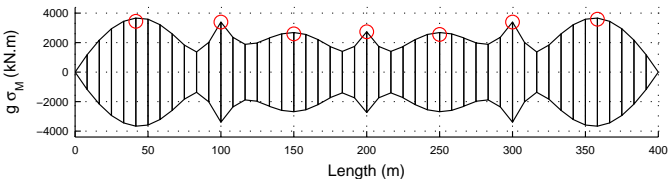
<sup>a</sup>Wind Engineering Institute, Japan

<sup>b</sup>Tokyo Polytechnic University, Japan

we want to keep **physical interpretations, meanings** of the static wind loads!

## Option 1 : Engineering approach

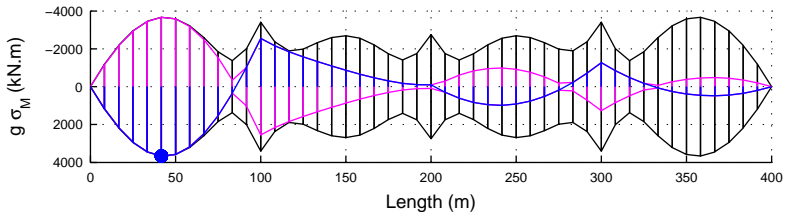
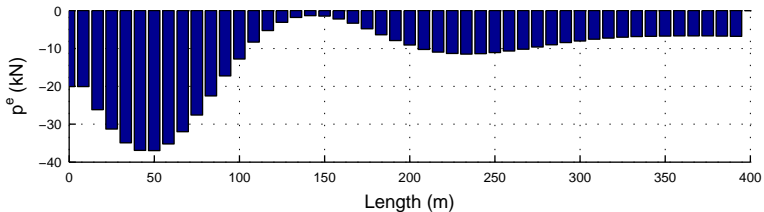
- ▶ Selection of **critical sections** based on the engineering judgement



mid-spans and supports ( $N^* = 7$ )

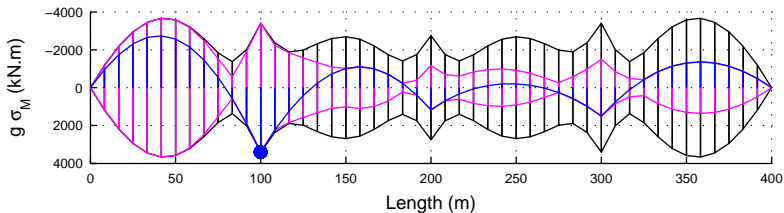
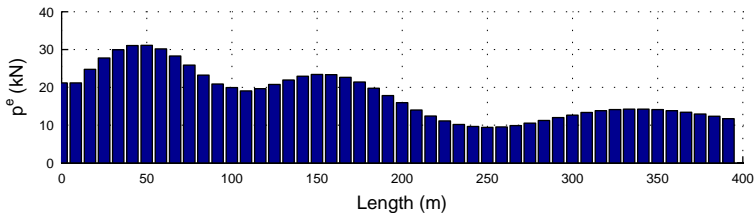
## Option 1 : Engineering approach

Eswl n°1



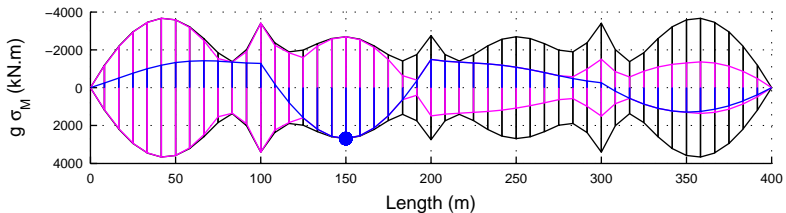
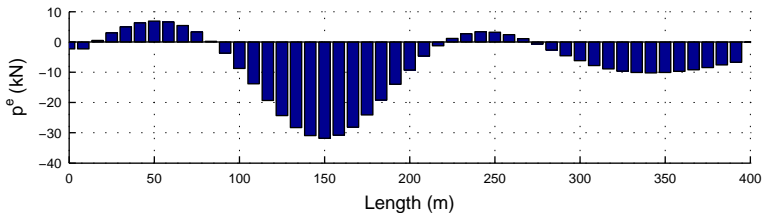
## Option 1 : Engineering approach

Eswl n°2



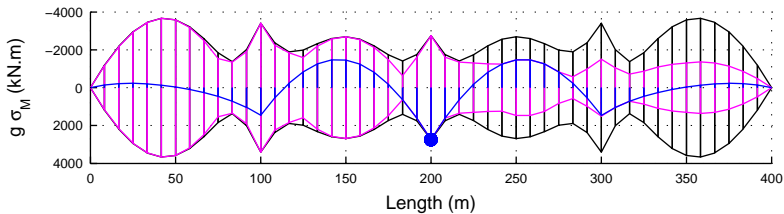
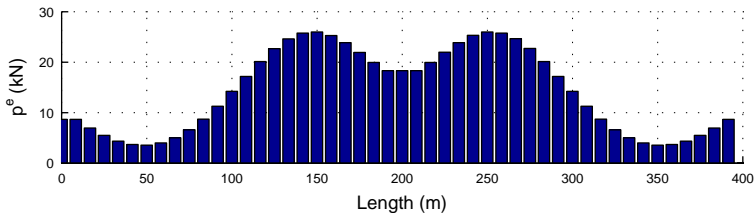
## Option 1 : Engineering approach

Eswl n°3



## Option 1 : Engineering approach

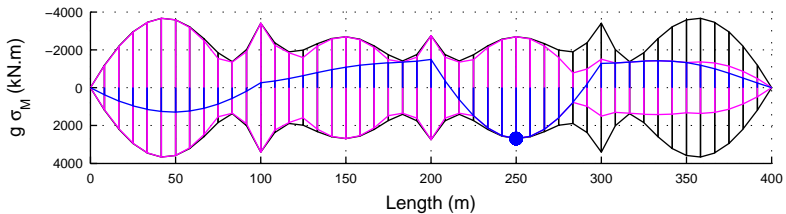
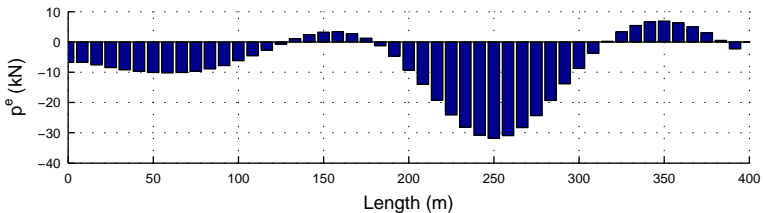
Eswl n°4





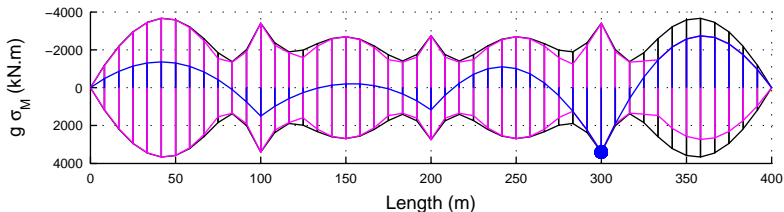
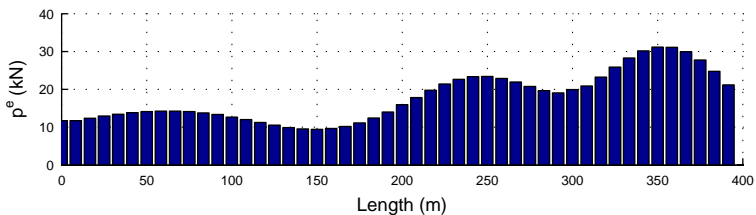
## Option 1 : Engineering approach

Eswl n°5



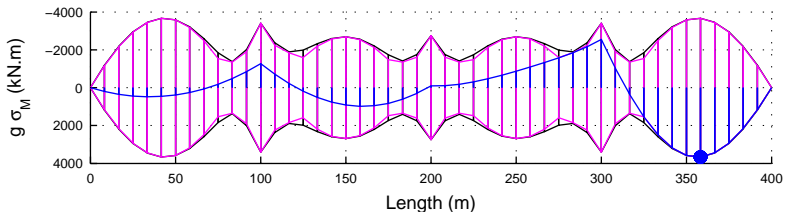
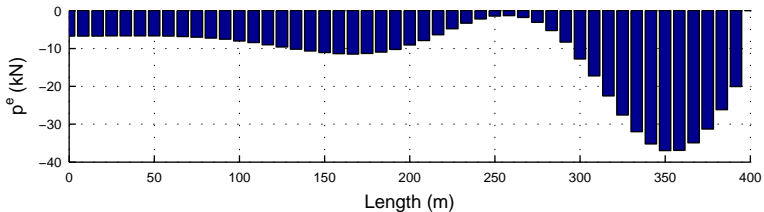
## Option 1 : Engineering approach

Eswl n°6



## Option 1 : Engineering approach

Eswl n°7

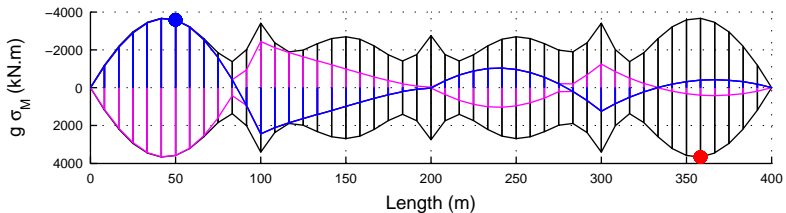
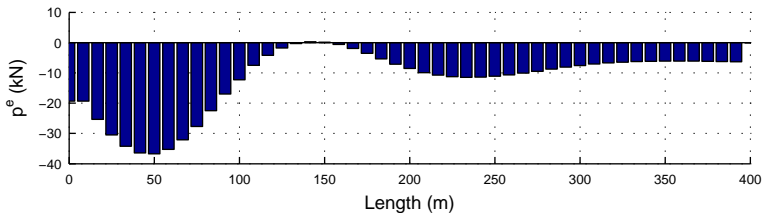


## Option 2 : Iterative process

- ▶ Compute all of the eswl  $\mathbf{p}_j^e$  ( $\mathbf{N}=47$ )
- ▶ Selection of the eswl by an **iterative process which minimizes with the difference** to the target envelope **at each iteration.**

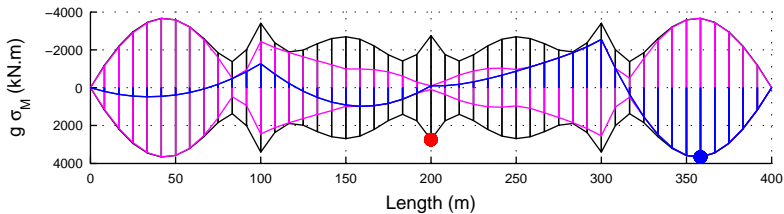
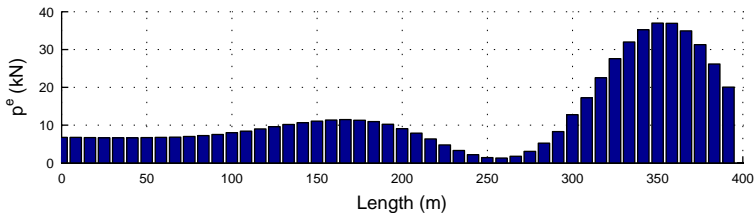
## Option 2 : Iterative process

## Iteration 1



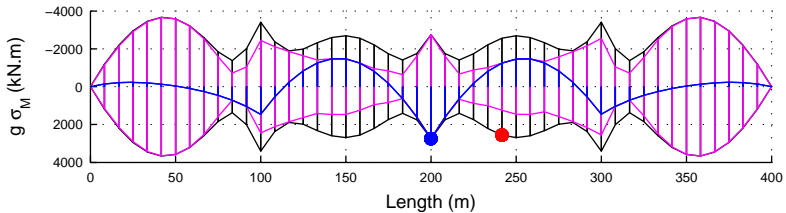
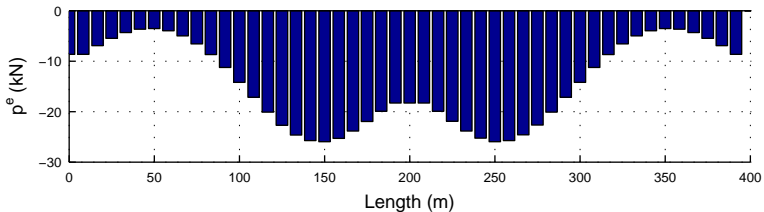
## Option 2 : Iterative process

## Iteration 2



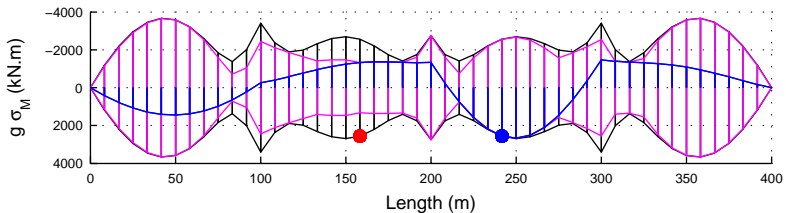
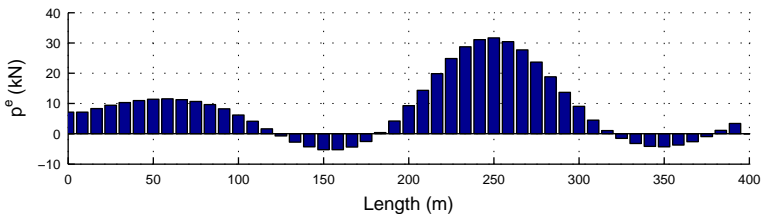
## Option 2 : Iterative process

## Iteration 3



## Option 2 : Iterative process

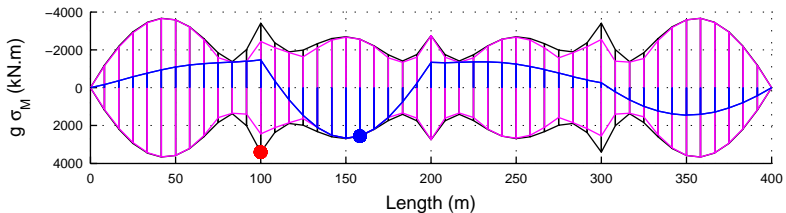
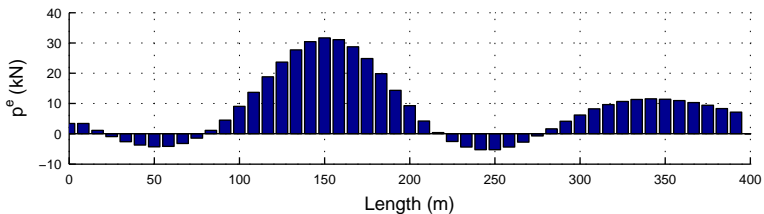
## Iteration 4





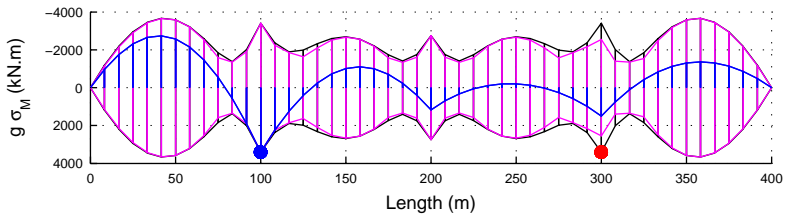
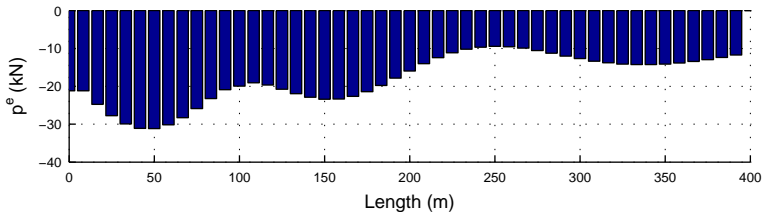
## Option 2 : Iterative process

## Iteration 5



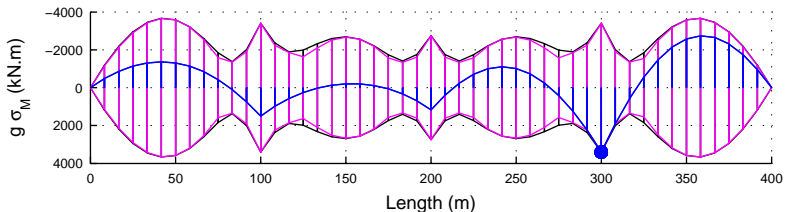
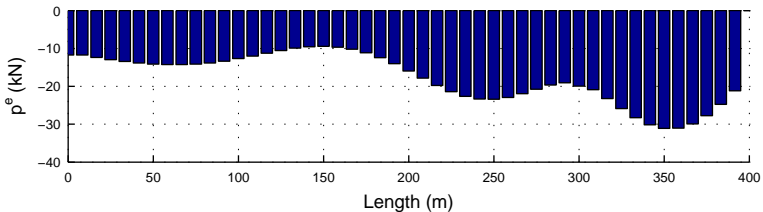
## Option 2 : Iterative process

## Iteration 6



## Option 2 : Iterative process

## Iteration 7





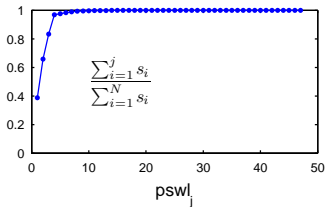
## Singular Value Decomposition of the eswl matrix

- ▶ Singular value decomposition of the **eswl matrix**  $\mathbf{p}^e$  ( $N = 47$ )

$$\begin{array}{ccc}
 \mathbf{p}^e & & \mathbf{p}^P & & \mathbf{s} & & \mathbf{v}' \\
 \left( \begin{array}{ccc} p_{12}^e & \cdots & p_{1n}^e \\ \vdots & \ddots & \vdots \\ p_{m1}^e & & p_{mn}^e \end{array} \right) & = & \left( \begin{array}{ccc} p_{11}^P & \cdots & p_{1r}^P \\ \vdots & \ddots & \vdots \\ p_{m1}^P & & p_{mr}^P \end{array} \right) & \left( \begin{array}{cc} s_{11} & 0 \\ & \ddots \\ 0 & \\ & & s_{rr} \end{array} \right) & \left( \begin{array}{ccc} v_{11} & \cdots & v_{1n} \\ \vdots & \ddots & \vdots \\ v_{r1} & & v_{rn} \end{array} \right) \\
 m \times N & & m \times N^* & & N^* \times N^* & & N^* \times N
 \end{array}$$

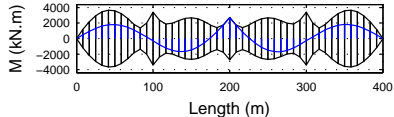
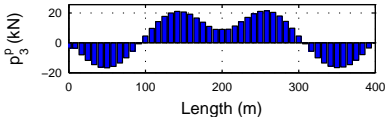
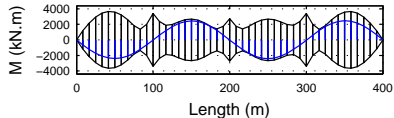
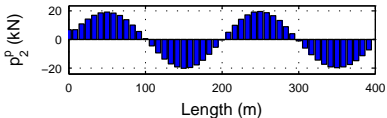
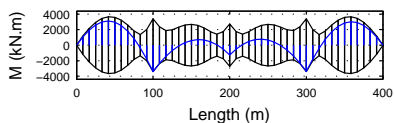
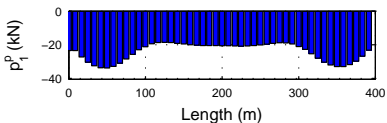
where  $\mathbf{p}^P$  collects **principal static wind loads (pswl)**.

- ▶ Convergence of the decomposition :



**Truncation to the first three modes** ( $N^* = 3$ )

# Singular Value Decomposition of the eswl matrix



- ▶ **principal swl** targets simultaneously several extreme responses
- ▶ **no need of selection** between eswl.

## How to combine ?

# Application Research of Constrained Least-Squares Method in Computing Equivalent Static Wind Loads

Xuanyi Zhou<sup>a</sup>, Ming Gu<sup>b</sup>, Gang Li<sup>c</sup>

**Abstract:** This study proposes a constrained least-squares method to compute the equivalent static wind loads (ESWL) distribution for large-span roofs, which simultaneously targets several peak responses. The loading distribution is regarded as a linear combination of basic load distributions, which is based on the modified LRC method. To obtain ESWL with a reasonable magnitude range, the participation factor was limited to serve as a constraint, yielding an ESWL distribution that is a least-squares solution to a system of constrained linear algebraic equations. To verify its computational accuracy, the method is applied to a real large-span roof structure, and detailed comparisons between results from different basic load distributions were performed.

13th International Conference on Wind Engineering, July 10-15, 2011  
(Amsterdam)

## Option 3. Combination between pswl to obtain gswl

- ▶ **Combination** between the **principal swl** ( $N^* = 3$ ) to obtain **global swl (gswl)** :

$$\mathbf{p}_i^g = \sum_{j=1}^{N^*} \mathbf{p}_j^p \mathbf{q}_{j,i} \quad i = 1 : 3^{N^*} - 1$$

where  $\mathbf{q}$  collects combination  $(-1; 1; 0)$  between the  $N^*$  pswl  $\mathbf{p}_j^p$

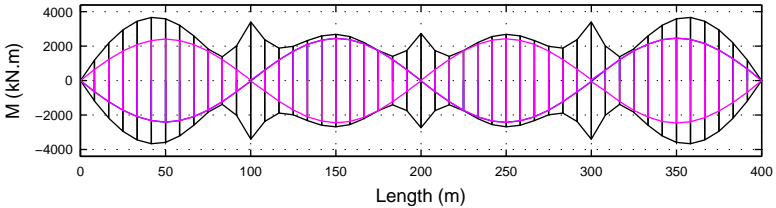
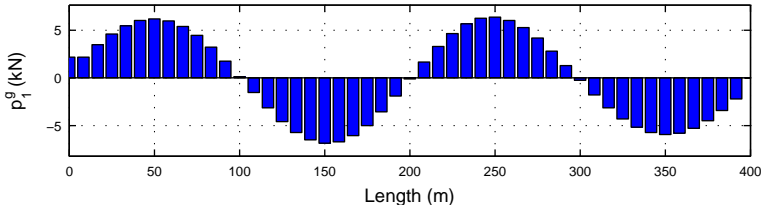
- ▶ The choice of the gswl to apply succesively is realized using the same iterative approach as before



# Option 3. Combination between pswl to obtain gswl

## Combination 1

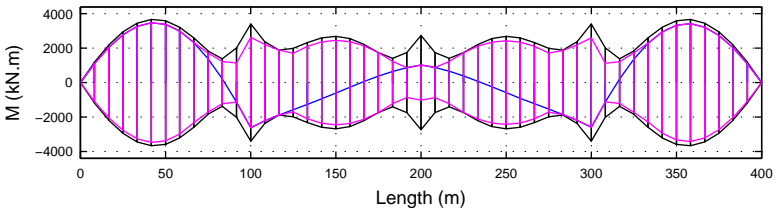
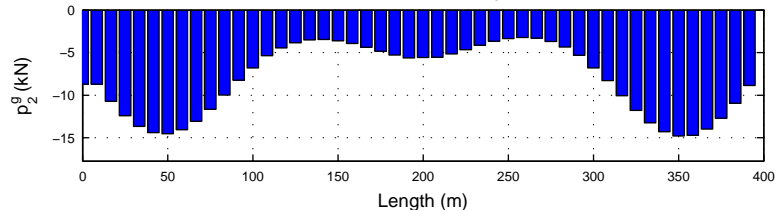
$$\alpha_1 (+0 \phi_1 + 1 \phi_2 + 0 \phi_3)$$



# Option 3. Combination between pswl to obtain gswl

## Combination 2

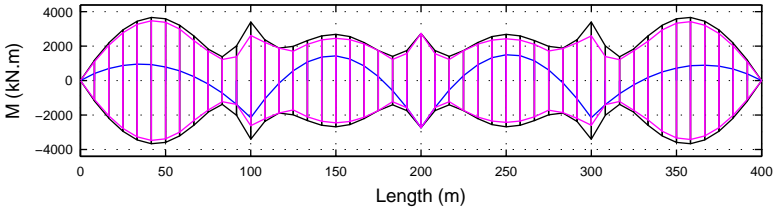
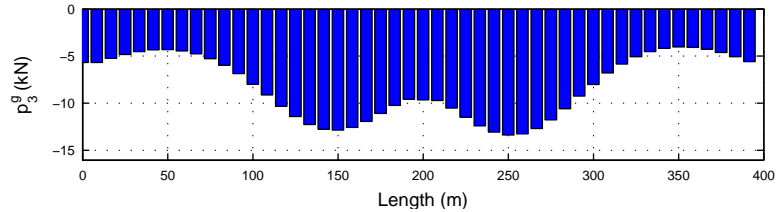
$$\alpha_2 (+1 \phi_1 + 0 \phi_2 + 1 \phi_3)$$



# Option 3. Combination between pswl to obtain gswl

## Combination 3

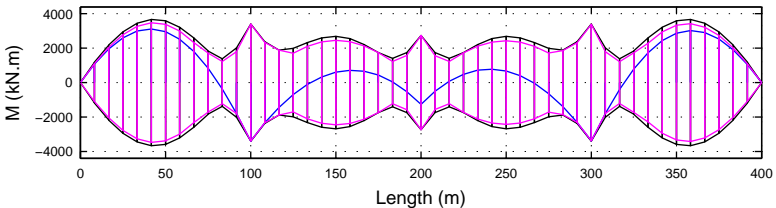
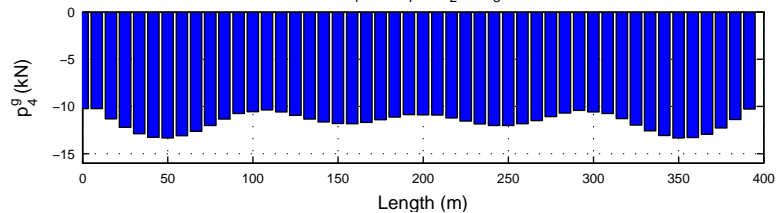
$$\alpha_3 (+1 \phi_1 + 0 \phi_2 - 1 \phi_3)$$



# Option 3. Combination between pswl to obtain gswl

## Combination 4

$$\alpha_4 (+1 \phi_1 + 0 \phi_2 + 0 \phi_3)$$



Introduction

Nodal Dynamic Analysis

Envelope approximation

Conclusion



Objective : use of a **minimum number of load cases** to achieve an accepted underestimation level of the envelope diagram

- ▶ Option 1 : Engineering approach. Choice of the eswl easy for a simple structure but quite complicated when considering large structures. Convergence to the target envelope is not sure.
- ▶ Option 2 : Iterative process. Works quite well but does not allow to target several extreme responses.
- ▶ Option 3 : Combination of pswl to obtain gswl
  - ▶ Computation of a **minimum number** of principal swl using SVD
  - ▶ **Combination** to obtain global swl is **straightforward**
  - ▶ These global swl targets several extreme responses and so
  - ▶ **the convergence** is increased and a minimum number of global swl is obtained





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- ▶ Possible **comparison** of these global swl with others computed for similar structures → **codification**
- ▶ Application to large structures



Thank you for your attention.

Questions ?

Read out more about me on : [www.orbi.ulg.ac.be](http://www.orbi.ulg.ac.be)

Contact me at : [N.Blaise@ulg.ac.be](mailto:N.Blaise@ulg.ac.be)