

Equivalent Static Wind Loads for structures with non-proportional damping

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Introduction

Illustrative example

Equivalent static wind loads

Conclusion

Analysis of structures under random excitations

Structures



View from the sky, Cape town

are subjected to wind excitations



Wembley Square, Cape town (2009)



Vista High school, Cape town (2009)

Dynamic analysis of large structures

■ Modal basis

$$\begin{array}{ccc}
 \begin{array}{c} \nearrow \text{Normal modes of vibration} \\ \Phi^T \mathbf{M} \Phi = \mathbf{I} \end{array} &
 \begin{array}{c} \nearrow \text{Modal damping matrix} \\ \Phi^T \mathbf{C} \Phi = \mathbf{D} \end{array} &
 \begin{array}{c} \nearrow \text{Modal stiffness matrix (diagonal)} \\ \Phi^T \mathbf{K} \Phi = \mathbf{\Omega} \\ \searrow \end{array}
 \end{array}$$

□ Rayleigh Damping

$$\mathbf{C} = \alpha \mathbf{K} + \beta \mathbf{M} \quad \longrightarrow \quad \mathbf{D} = \mathbf{D}_d \quad (\text{diagonal})$$

Dynamic analysis of large structures

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■ Sources of non-proportionality

- damping devices (TMD, TLCD), aerodynamic damping and...

D is **not** diagonal

Dynamic analysis of large structures

■ Modal basis

$$\Phi^T \mathbf{M} \Phi = \mathbf{I} \quad \Phi^T \mathbf{C} \Phi = \mathbf{D} \quad \Phi^T \mathbf{K} \Phi = \mathbf{\Omega}$$

Normal modes of vibration Modal damping matrix
 Modal stiffness matrix (diagonal)

□ Rayleigh Damping

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■ Sources of non-proportionality

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D is **not** diagonal

■ Coupled system of equation of motion

$$\ddot{\mathbf{q}} + \mathbf{D} \dot{\mathbf{q}} + \mathbf{\Omega} \mathbf{q} = \mathbf{g}$$

Modal amplitudes Generalized forces

Dynamic analysis of large structures

■ Split damping matrix

$$\begin{array}{c} \mathbf{D} \\ \left(\begin{array}{ccc} \square & \square & \square \\ \square & \square & \square \\ \square & \square & \square \end{array} \right) \end{array} = \begin{array}{c} \mathbf{D}_d \\ \left(\begin{array}{ccc} \square & & \\ & \square & \\ & & \square \end{array} \right) \end{array} + \begin{array}{c} \mathbf{D}_o \\ \left(\begin{array}{ccc} & \square & \square \\ \square & & \square \\ \square & \square & \square \end{array} \right) \end{array}$$

Diagonal elements
Off-diagonal elements

¹Rayleigh. (1877). The Theory of Sound.Vol. 1. New-York : Dover Publication

Dynamic analysis of large structures

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Diagonal elements Off-diagonal elements

■ Decoupling approximation¹

$$\mathbf{H}_d = (-\mathbf{I}\omega^2 + j\omega\mathbf{D}_d + \mathbf{\Omega})^{-1} \longrightarrow \begin{array}{l} \text{Inversion of a diagonal matrix only} \\ \text{Decoupled system} \end{array}$$

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Dynamic analysis of large structures

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$$\begin{pmatrix} \square & \square & \square \\ \square & \square & \square \\ \square & \square & \square \end{pmatrix}^{\mathbf{D}} = \begin{pmatrix} \square & & \\ & \square & \\ & & \square \end{pmatrix}^{\mathbf{D}_d} + \begin{pmatrix} & \square & \square \\ \square & & \square \\ \square & \square & \square \end{pmatrix}^{\mathbf{D}_o}$$

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■ Full matrix inversion

$$\mathbf{H} = (-\mathbf{I}\omega^2 + j\omega\mathbf{D} + \mathbf{\Omega})^{-1} \rightarrow \begin{array}{l} \text{Full matrix inversion} \\ \text{Coupled system} \end{array}$$

$$\mathbf{H} = (\mathbf{I} + j\omega\mathbf{H}_d\mathbf{D}_o)^{-1} \mathbf{H}_d$$

¹Rayleigh. (1877). The Theory of Sound.Vol. 1. New-York : Dover Publication

Asymptotic expansion method

■ Key-idea¹

$$\mathbf{H} = (\mathbf{I} + j\omega \mathbf{H}_d \mathbf{D}_o)^{-1} \mathbf{H}_d$$

$$\downarrow (\mathbf{I} + \mathbf{X})^{-1} \simeq \mathbf{I} - \mathbf{X} + \mathbf{X}^2 - \dots = \mathbf{I} + \sum_{i=1}^k (-\mathbf{X})^i$$

Condition: $r(\mathbf{X}) = \|\boldsymbol{\lambda}\|_{\infty} < 1$


↖ Eigenvalues of \mathbf{X}

¹Denoël and Degée. (2009). Asymptotic expansion of slightly coupled modal dynamic transfer functions non-proportional damping. *Journal of Sound and Vibration* 328, 1-2, 1-8

Asymptotic expansion method

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$$\mathbf{H} = (\mathbf{I} + j\omega \mathbf{H}_d \mathbf{D}_o)^{-1} \mathbf{H}_d$$



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Condition: $r(\mathbf{X}) = \|\boldsymbol{\lambda}\|_{\infty} < 1$

Eigenvalues of \mathbf{X}

■ Approximation of \mathbf{H}

Decoupling approximation (diagonal)

$$\mathbf{H}_k = \mathbf{H}_d + \underbrace{\sum_{i=1}^k (-j\omega)^i (\mathbf{H}_d \mathbf{D}_o)^i \mathbf{H}_d}_{\text{Corrections terms (non-diagonal)}}$$

No full matrix inversion

Inversion of a diagonal matrix only



Approximate the coupled system

¹Denoël and Degée. (2009). Asymptotic expansion of slightly coupled modal dynamic transfer functions non-proportional damping. *Journal of Sound and Vibration* 328, 1-2, 1-8

Stochastic modal analysis

■ Exact solution

$$\mathbf{S}^{(q)} = \mathbf{H} \mathbf{S}^{(g)} \mathbf{H}^*$$

↗ PSD matrix of generalized forces
 ↘ PSD matrix of modal displacements

¹Canor, Blaise and Denoël. (2012). Efficient uncoupled stochastic analysis with non-proportional damping. *Journal of Sound and Vibration* 331, 24, 5283-5291

Stochastic modal analysis

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■ Stochastic modal analysis¹

$$\mathbf{S}^{(q_k)} = \mathbf{H}_k\mathbf{S}^{(g)}\mathbf{H}_k^*$$

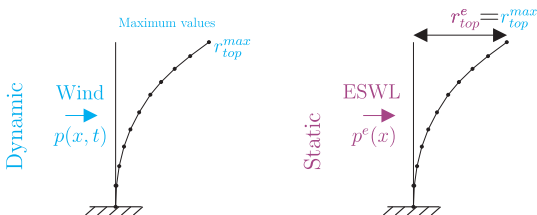
↙

$$\mathbf{S}^{(q_k)} = \mathbf{S}^{(q_d)} + \underbrace{\sum_{i=1}^k \Delta\mathbf{S}^{(q_i)}}_{\text{Corrections terms due to non-proportionality damping}}$$

↖ Solution in the uncoupled system

¹Canor, Blaise and Denoël. (2012). Efficient uncoupled stochastic analysis with non-proportional damping. *Journal of Sound and Vibration* 331, 24, 5283-5291

Equivalent static wind loads



- Chen & Kareem formulation¹

$$\mathbf{p}_j^e = g_j \sum_{m=1}^M W_{jm} \boldsymbol{\psi}_m$$

- Objective :

Approximate formulation $\mathbf{p}_j^{e,k}$ in case of non-proportional damping

¹Chen, and Kareem. (2009). Equivalent static wind loads for buffeting response of bridges by mass and liquid dampers. *Journal of Structural Engineering-Asce* 127, 12, 1467-1475

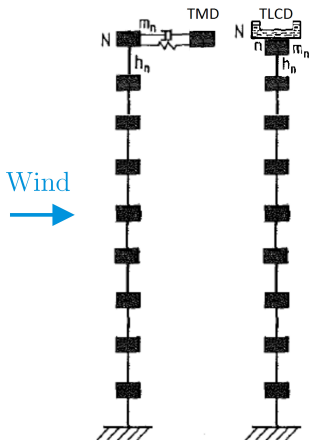
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Conclusion

306 m Tall building



- 10-lumped-mass cantilever beam model
- Random excitation : **wind**
 - 1-D Gaussian velocity field
- Structural and aerodynamic data from¹
- Two studied cases :
 - Tuned Mass Damper
 - Tuned Liquid Column Damper

¹Xu, Samali, and Kwok. (2009). Control of along-wind response of structures by mass and liquid dampers. *Journal of Engineering Mechanics* 118, 1, 20-39

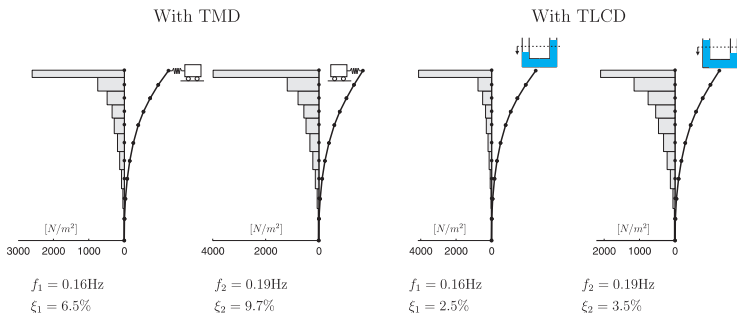
- Inertial forces per unit surface

$$\Psi_m = \mathbf{K}\Phi_m$$

m^{th} inertial force

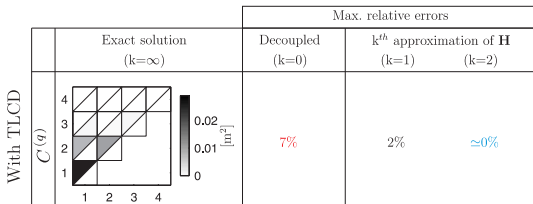
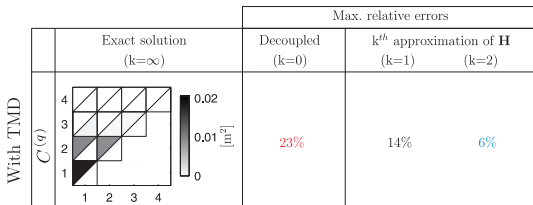
m^{th} modal shape

- First two modes (five modes considered for the analysis)



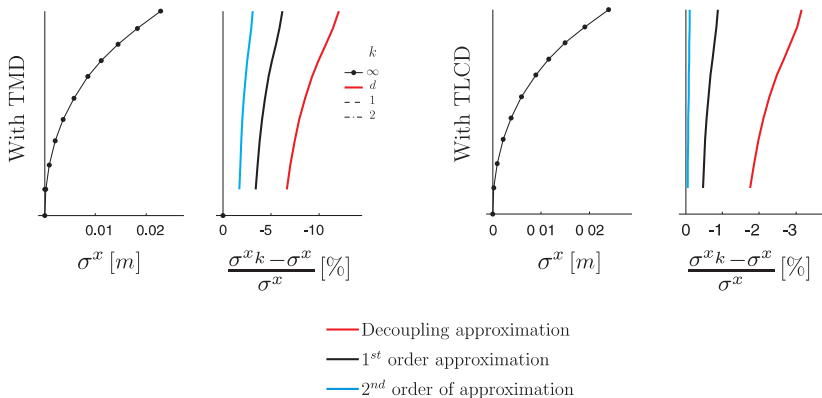
■ Covariance matrix of modal displacements

$$\int_{-\infty}^{+\infty} \mathbf{S}^{(q_k)} d\omega = \mathbf{C}^{(q_k)} + \sum_{i=1}^k \Delta \mathbf{C}^{(q_i)}$$



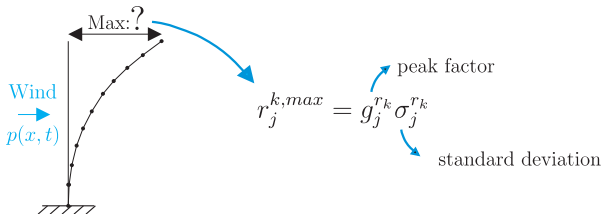
Structural analysis

■ Standard deviations of nodal displacements



■ Envelope values (min and max) of the structural responses

- Extreme value theory



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Asymptotic expansion method

■ Weighted combinations of the inertial forces

$$\mathbf{p}_j^{e,k} = g_j^k \sum_m^M W_{jm}^k \psi_m$$

k^{th} approximation of the weighting coefficients

$$W_{jm}^k = \alpha_j^k W_{jm}^d + \sum_{i=1}^k \Delta W_{jm}^i$$

Asymptotic expansion method

- Weighted combinations of the inertial forces

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- Definition of the ESWL

k^{th} approximation of the ESWL

$$\mathbf{p}_j^{e,k} = \alpha_j^k \mathbf{p}_j^{e,d} + \Delta \mathbf{p}_j^{e,k}$$

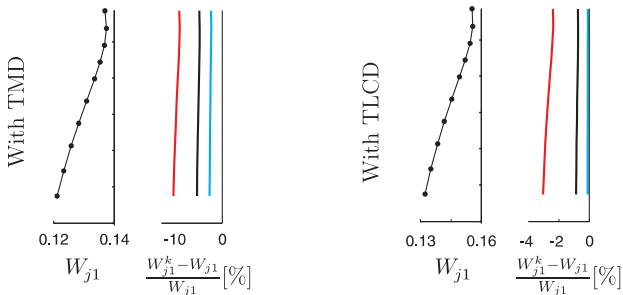
ESWL for the uncoupled system

scaled coefficients

correction resulting from the non-proportionality of damping

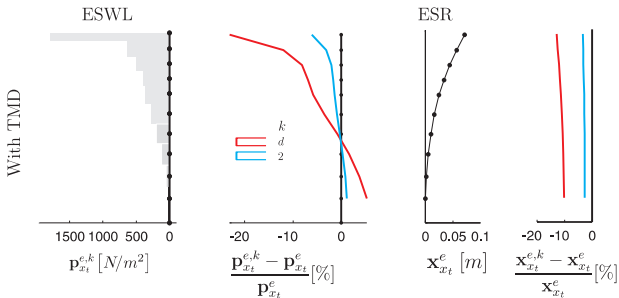
Weighting coefficients

■ First inertial force

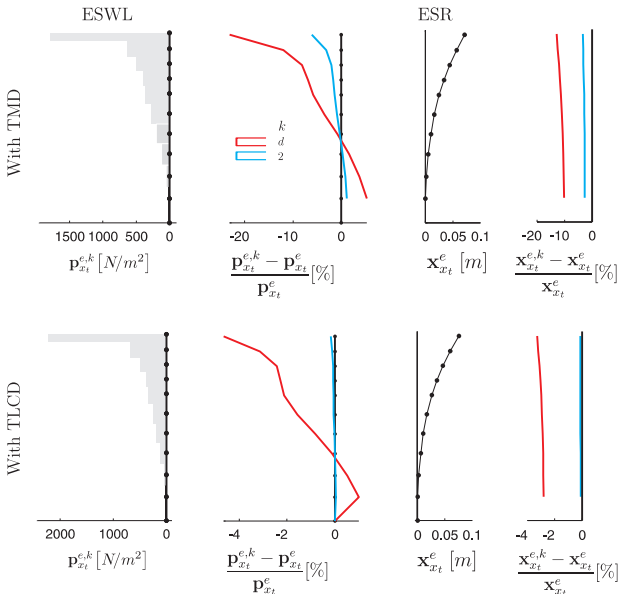


- Decoupling approximation
- 1st order approximation
- 2nd order of approximation

ESWL-Horizontal displacement at the top



ESWL-Horizontal displacement at the top



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- Asymptotic expansion of the modal transfer matrix enables to avoid full transfer matrix inversion

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- **New method** for the establishment of ESWL for structures with non-proportional damping analysed in the modal basis

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- Studied case : 306 m Tall building
 - Second order approximation of \mathbf{H} is sufficient
 - ESWL obtained with the new method correctly fit the real ones

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- Applications
 - Equivalent static design
 - Structural optimization using ESWL

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- Perspective
 - Dynamic system with non-linear terms

The team...

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...thanks you for your kind attention

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Questions ?

