

Equivalent Static Wind Loads for structures with non-proportional damping

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Introduction

Illustrative example

Equivalent static wind loads

Conclusion

Analysis of structures under random excitations

Structures



are subjected to wind excitations



Wembley Square, Cape town (2009)



Vista High school, Cape town (2009)

Dynamic analysis of large structures

■ Modal basis

$$\Phi^T \mathbf{M} \Phi = \mathbf{I} \quad \Phi^T \mathbf{C} \Phi = \mathbf{D} \quad \Phi^T \mathbf{K} \Phi = \boldsymbol{\Omega}$$

Normal modes of vibration
Modal damping matrix
Modal stiffness matrix (diagonal)

□ Rayleigh Damping

$$\mathbf{C} = \alpha \mathbf{K} + \beta \mathbf{M} \rightarrow \mathbf{D} = \mathbf{D}_d \text{ (diagonal)}$$

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■ Sources of non-proportionality

□ damping devices (TMD, TLCD), aerodynamic damping and...

D is **not** diagonal

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■ Coupled system of equation of motion

$$\ddot{\mathbf{q}} + \mathbf{D}\dot{\mathbf{q}} + \mathbf{\Omega}\mathbf{q} = \mathbf{g}$$

Modal amplitudes
Generalized forces

Dynamic analysis of large structures

■ Split damping matrix

$$\mathbf{D} = \mathbf{D}_d + \mathbf{D}_o$$
$$\begin{pmatrix} \square & \square & \square \\ \square & \square & \square \\ \square & \square & \square \end{pmatrix} = \begin{pmatrix} \square & & \\ & \square & \\ & & \square \end{pmatrix} + \begin{pmatrix} \square & \square & \square \\ \square & \square & \square \\ \square & \square & \square \end{pmatrix}$$

Diagonal elements Off-diagonal elements

¹ Rayleigh. (1877). The Theory of Sound. Vol. 1. New-York : Dover Publication

Dynamic analysis of large structures

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Diagonal elements Off-diagonal elements

■ Decoupling approximation¹

$$\mathbf{H}_d = (-\mathbf{I}\omega^2 + j\omega\mathbf{D}_d + \mathbf{\Omega})^{-1} \rightarrow \begin{array}{l} \text{Inversion of a diagonal matrix only} \\ \text{Decoupled system} \end{array}$$

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■ Full matrix inversion

$$\mathbf{H} = (-\mathbf{I}\omega^2 + j\omega\mathbf{D} + \mathbf{\Omega})^{-1} \rightarrow \begin{array}{l} \text{Full matrix inversion} \\ \text{Coupled system} \end{array}$$

$$\mathbf{H} = (\mathbf{I} + j\omega\mathbf{H}_d\mathbf{D}_o)^{-1} \mathbf{H}_d$$

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Asymptotic expansion method

■ Key-idea¹

$$\mathbf{H} = (\mathbf{I} + j\omega \mathbf{H}_d \mathbf{D}_o)^{-1} \mathbf{H}_d$$



$$(\mathbf{I} + \mathbf{X})^{-1} \simeq \mathbf{I} - \mathbf{X} + \mathbf{X}^2 - \dots = \mathbf{I} + \sum_{i=1}^k (-\mathbf{X})^i$$

Condition: $r(\mathbf{X}) = \|\boldsymbol{\lambda}\|_\infty < 1$

↑
Eigenvalues of \mathbf{X}

Asymptotic expansion method

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Condition: $r(\mathbf{X}) = \|\boldsymbol{\lambda}\|_\infty < 1$

↑
Eigenvalues of \mathbf{X}

■ Approximation of \mathbf{H}

$$\mathbf{H}_k = \mathbf{H}_d + \underbrace{\sum_{i=1}^k (-j\omega)^i (\mathbf{H}_d \mathbf{D}_o)^i \mathbf{H}_d}_{\text{Corrections terms (non-diagonal)}}$$

No full matrix inversion
Inversion of a diagonal matrix only



Approximate the coupled system

¹ Denoël and Degée. (2009). Asymptotic expansion of slightly coupled modal dynamic transfer functions non-proportional damping. *Journal of Sound and Vibration* 328, 1-2, 1-8

Stochastic modal analysis

■ Exact solution

$$\mathbf{S}^{(q)} = \mathbf{H} \mathbf{S}^{(g)} \mathbf{H}^*$$

↗ PSD matrix of generalized forces
↘ PSD matrix of modal displacements

¹ Canor, Blaise and Denoël. (2012). Efficient uncoupled stochastic analysis with non-proportional damping. *Journal of Sound and Vibration* 331, 24, 5283-5291

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■ Stochastic modal analysis¹

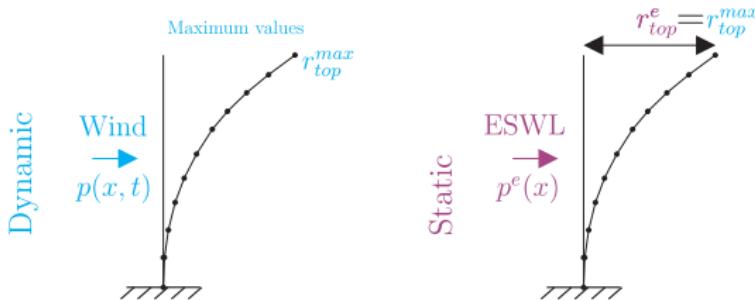
$$\mathbf{S}^{(q_k)} = \mathbf{H}_k \mathbf{S}^{(g)} \mathbf{H}_k^*$$

↙

$$\mathbf{S}^{(q_k)} = \mathbf{S}^{(q_d)} + \underbrace{\sum_{i=1}^k \Delta \mathbf{S}^{(q_i)}}_{\text{ Corrections terms due to non-proportionality damping}}$$

↗ Solution in the uncoupled system

Equivalent static wind loads



■ Chen & Kareem formulation¹

$$\mathbf{p}_j^e = g_j \sum_{m=1}^M W_{jm} \boldsymbol{\psi}_m$$

■ Objective :

Approximate formulation $\mathbf{p}_j^{e,k}$ in case of non-proportional damping

¹ Chen, and Kareem. (2009). Equivalent static wind loads for buffeting response of bridges by mass and liquid dampers. *Journal of Structural Engineering-Asce* 127, 12, 1467-1475

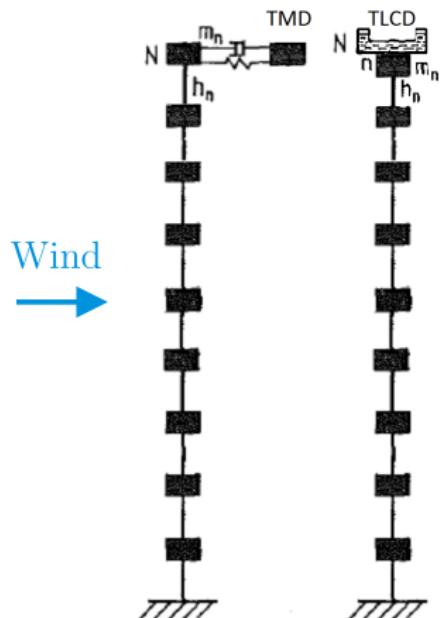
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306 m Tall building



- 10-lumped-mass cantilever beam model
- Random excitation : **wind**
 - 1-D Gaussian velocity field
- Structural and aerodynamic data from¹
- Two studied cases :
 - Tuned Mass Damper
 - Tuned Liquid Column Damper

¹ Xu, Samali, and Kwok. (2009). Control of along-wind response of structures by mass and liquid dampers. *Journal of Engineering Mechanics* 118, 1, 20-39

Modal properties

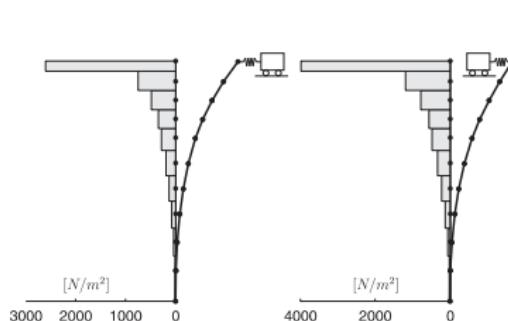
■ Inertial forces per unit surface

$$\Psi_m = \mathbf{K} \Phi_m$$

\curvearrowleft m^{th} inertial force
 \curvearrowright m^{th} modal shape

■ First two modes (five modes considered for the analysis)

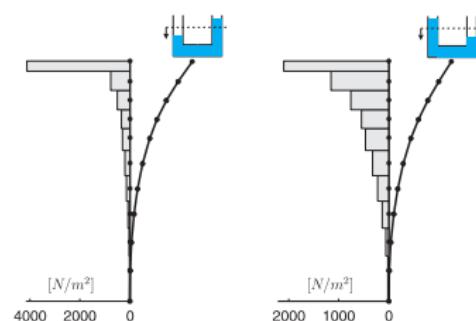
With TMD



$$f_1 = 0.16 \text{Hz}$$
$$\xi_1 = 6.5\%$$

$$f_2 = 0.19 \text{Hz}$$
$$\xi_2 = 9.7\%$$

With TLCD



$$f_1 = 0.16 \text{Hz}$$
$$\xi_1 = 2.5\%$$

$$f_2 = 0.19 \text{Hz}$$
$$\xi_2 = 3.5\%$$

Structural analysis

■ Covariance matrix of modal displacements

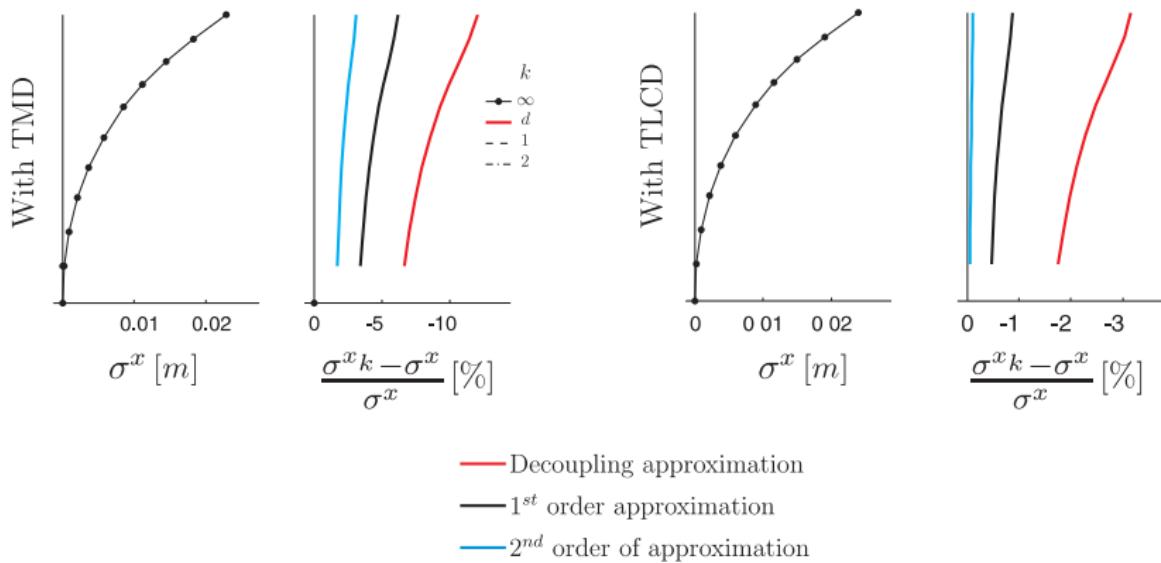
$$\int_{-\infty}^{+\infty} \mathbf{S}^{(q_k)} d\omega = \mathbf{C}^{(q_d)} + \sum_{i=1}^k \Delta \mathbf{C}^{(q_i)}$$

	Exact solution (k=∞)	Max. relative errors		
		Decoupled (k=0)	k th approximation of H (k=1)	(k=2)
With TMD	$C^{(q)}$ 	0.02 0.01 [m ²] 0	23%	14% 6%

	Exact solution (k=∞)	Max. relative errors		
		Decoupled (k=0)	k th approximation of H (k=1)	(k=2)
With TLCD	$C^{(q)}$ 	0.02 0.01 [m ²] 0	7%	2% ≈0%

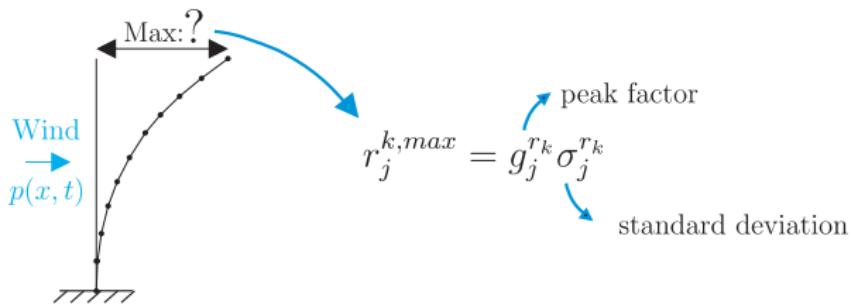
Structural analysis

■ Standard deviations of nodal displacements



Structural design

- Envelope values (min and max) of the structural responses
 - Extreme value theory



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Asymptotic expansion method

■ Weighted combinations of the inertial forces

$$\mathbf{p}_j^{e,k} = g_j^k \sum_m^M W_{jm}^k \psi_m$$

→ k^{th} approximation of the weighting coefficients

$$W_{jm}^k = \alpha_j^k W_{jm}^d + \sum_{i=1}^k \Delta W_{jm}^i$$

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■ Definition of the ESWL

→ k^{th} approximation of the ESWL

$$\mathbf{p}_j^{e,k} = \alpha_j^k \mathbf{p}_j^{e,d} + \Delta \mathbf{p}_j^{e,k}$$

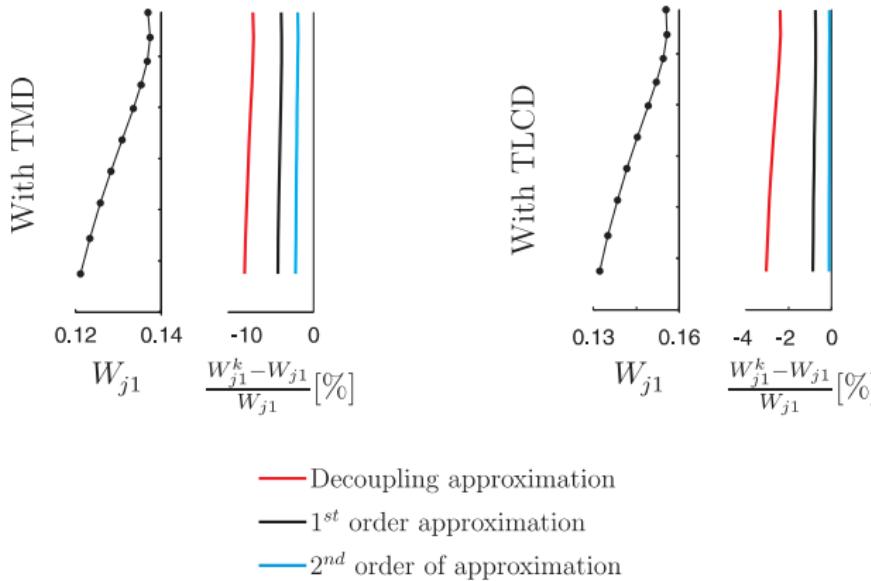
→ correction resulting from the
non-proportionality of damping

→ ESWL for the uncoupled system

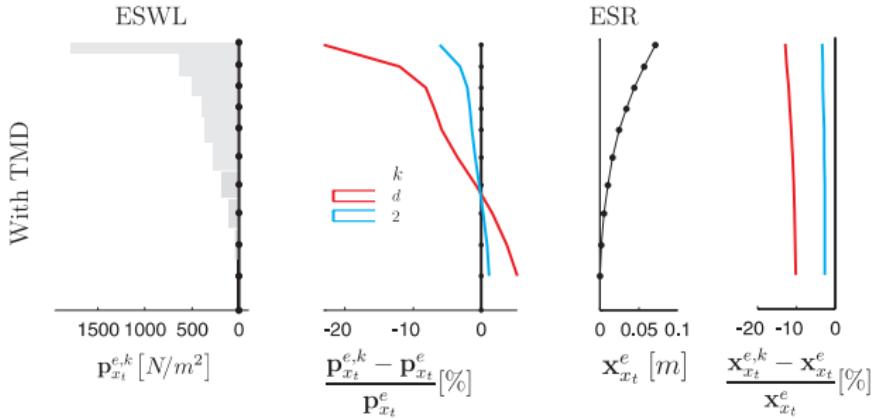
→ scaled coefficients

Weighting coefficients

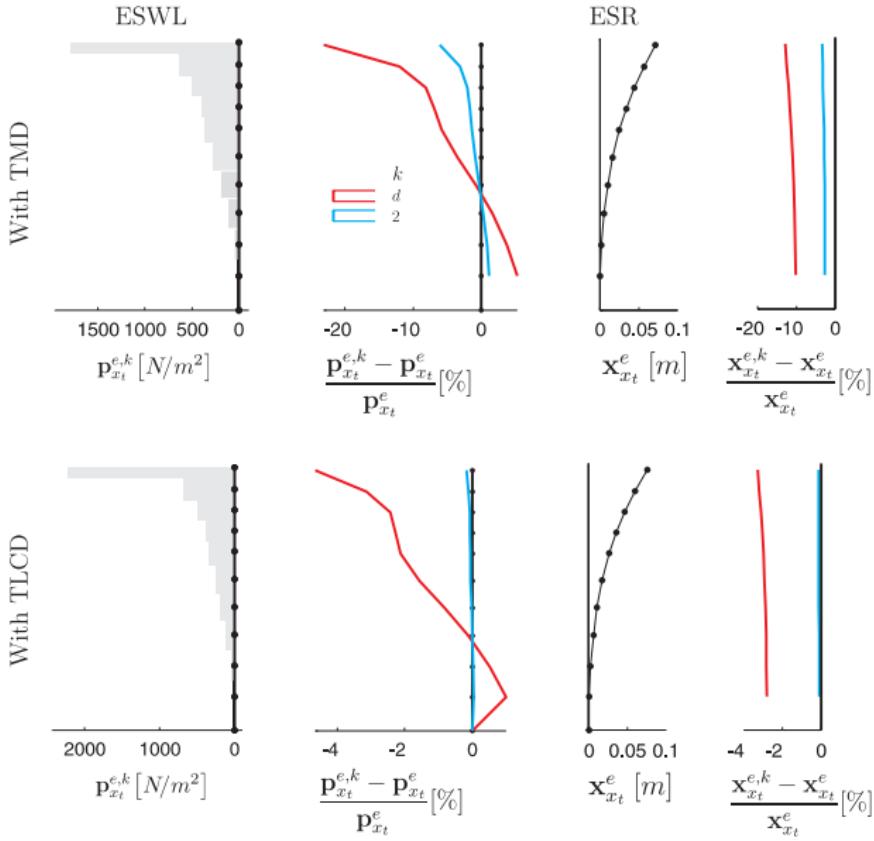
■ First inertial force



ESWL-Horizontal displacement at the top



ESWL-Horizontal displacement at the top



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ooooo

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ooo

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- New method for the establishment of ESWL for structures with non-proportional damping analysed in the modal basis

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 - Second order approximation of \mathbf{H} is sufficient
 - ESWL obtained with the new method correctly fit the real ones

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 - Equivalent static design
 - Structural optimization using ESWL

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 - Equivalent static design
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- Perspective
 - Dynamic system with non-linear terms

The team...

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...thanks you for your kind attention

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Questions ?

